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COLLOCATION FLUTTER ANALYSIS STUDY

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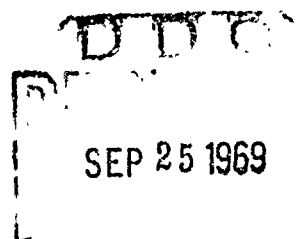
VOLUME II.

**FLUENC - COMPUTER PROGRAM TO CALCULATE
STRUCTURAL INFLUENCE COEFFICIENTS**

APRIL 1969



MISSILE SYSTEMS DIVISION



106

C O F A
C O L L O C A T I O N F L U T T E R A N A L Y S I S
S T U D Y

VOLUME II

FLUENC - Computer Program to Calculate
Structural Influence Coefficients

Prepared by the Dynamics and Environment
Section Personnel, Hughes Aircraft Company
Under Contract No. 0019-68-C-0247

April 1969

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ABSTRACT

A displacement solution for the calculation of structural influence coefficients (SIC's) is presented. The formulation utilizes the lumped parameter approach that is consistent with collocation flutter solutions. The structure is synthesized as concentrated mass elements connected by massless elastic plates and/or beams. There are two methods of generating the mass matrix; they are: 1) lumped concentrated mass points, 2) consistent mass matrices. Along with the calculation of the SIC's, the natural vibration modes and frequencies are calculated. There are two options for punching out the flexibility matrix for use in subsequent COFA computer programs. Option 1, punches out the full flexibility matrix; Option 2, punches out the reduced flexibility matrix eliminating the rows and columns pertaining to structural attach points.

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1.0 INTRODUCTION

In order to determine the aeroelastic behavior of a wing or control surface, it is necessary to know the aerodynamics, elastic properties and mass distributions of the structure. The overall aeroelastic analysis is usually divided into four separate parts as shown in Figure 1.

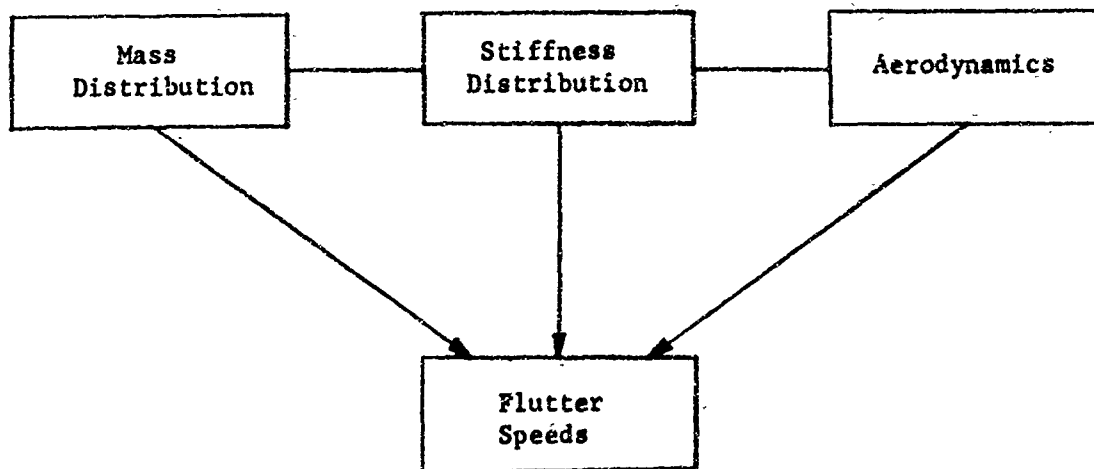


Figure 1. Analysis Procedure

This portion of the report describes the computation of the mass and stiffness distribution. The geometry of a wing or tail surface is too complex for the successful use of closed form analytical techniques. Therefore, a numerical type of analysis must be used. The end product of this analysis is the generation of overall influence coefficient and mass matrices referred to a set of node points arbitrarily picked on the surface of the structure. The finite element method (see Refs. 2 and 3) was used to form the required matrices for a planar structure. This technique is especially suited to solve complex structures and as used in the analysis is general enough to handle the following:

1. Combinations of beam and plate elements
2. Arbitrary boundary conditions
3. Lumped or distributed stiffnesses and masses

A discussion of the theory and computer program which calculates the influence coefficient and the mass matrix as well as the structural modes and frequencies is given in the following sections.

2.0 NOMENCLATURE

C	=	Unknown Boundary Constants
D	=	Plate Rigidity Constant
E	=	Modulus of Elasticity
F	=	Force
K	=	Stiffness Coefficients
M	=	Bending/Torsional Moment
p	=	Pressure
T	=	Coordinate Transformation
t	=	Thickness
w	=	Linear Displacement in z direction
x, y, z	=	Coordinate Axes
s	=	Linear Displacement
$\frac{d^2(s)}{d(x)^2}$	=	Curvature
ρ	=	Density
σ	=	Stress
ν	=	Poisson's Ratio
$\frac{\partial(\)}{\partial(x)}$	=	Partial Derivative
[]	=	Square Matrix
{ }	=	Column Matrix
[]	=	Row Matrix

3.0 TECHNICAL DISCUSSION

3.1 Influence Coefficients

The stiffness method approach is first used to obtain an overall stiffness matrix of the structure. This matrix is reduced by partitioning and then inverted to obtain the influence coefficients at any desired set of control points. The number of control points are denoted by N . At each node, three degrees of freedom are specified: two rotations and the normal displacement. Therefore, a stiffness matrix of approximately $3N$ degrees of freedom is first formed by superimposing individual plate and plane grid beam element global coordinate matrices. The matrix will be somewhat smaller than $3N$ degrees of freedom since boundary restraint conditions will reduce the size of the matrix. To illustrate the matrix condensation method used in the computer program, we will assume that we have N control point normal displacements and M displacements which must be eliminated. The overall stiffness matrix is given as

$$[K] = \begin{bmatrix} K_{NN} & K_{NM} \\ K_{MN} & K_{MM} \end{bmatrix} \quad (1)$$

The structural equilibrium matrix equation can be written as

$$\begin{bmatrix} K_{NN} & K_{NM} \\ K_{MN} & K_{MM} \end{bmatrix} \begin{Bmatrix} \delta_N \\ \delta_M \end{Bmatrix} = \begin{Bmatrix} F_N \\ F_M \end{Bmatrix} \quad (2)$$

We now assume that forces at the points to be eliminated are small and can be neglected. Therefore,

$$\begin{bmatrix} K_{NN} & K_{NM} \\ K_{MN} & K_{MM} \end{bmatrix} \begin{Bmatrix} \delta_N \\ \delta_M \end{Bmatrix} = \begin{Bmatrix} F_N \\ 0 \end{Bmatrix} \quad (3)$$

or

$$[K_{NN}]\{\delta_N\} + [K_{NM}]\{\delta_M\} = \{F_N\}$$

and

$$[K_{MN}]\{\delta_N\} + [K_{MM}]\{\delta_M\} = \{0\}$$

Therefore

$$\{\delta_M\} = -[K_{MM}]^{-1} [K_{MN}]\{\delta_N\} \quad (3a)$$

and

$$\left([K_{NN}] - [K_{NM}][K_{MM}]^{-1}[K_{MN}] \right) \{\delta_N\} = \{F_N\}$$

and since

$$[K_{MN}]^T = [K_{NM}]$$

we have

$$\{\delta_N\} = \left([K_{NN}] - [K_{MN}]^T [K_{MM}]^{-1} [K_{MN}] \right)^{-1} \{F_N\} \quad (4)$$

If we now let

$$[f_{NN}] = \left([K_{NN}] - [K_{MN}]^T [K_{MM}]^{-1} [K_{MN}] \right)^{-1}$$

then Equation (4) can be written as

$$\{\delta_N\} = [f_{NN}] \{F_N\} \quad (5)$$

The matrix $[f_{NN}]$ is called the structural influence coefficient matrix. The application of loads at the control points yield displacements at the control points by carrying out the matrix multiplication indicated in Equation (5).

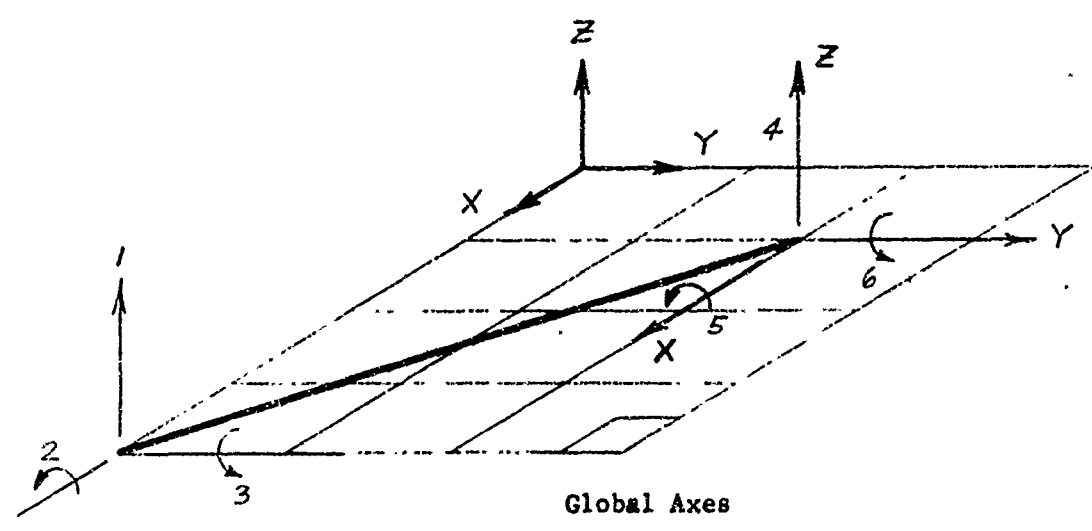
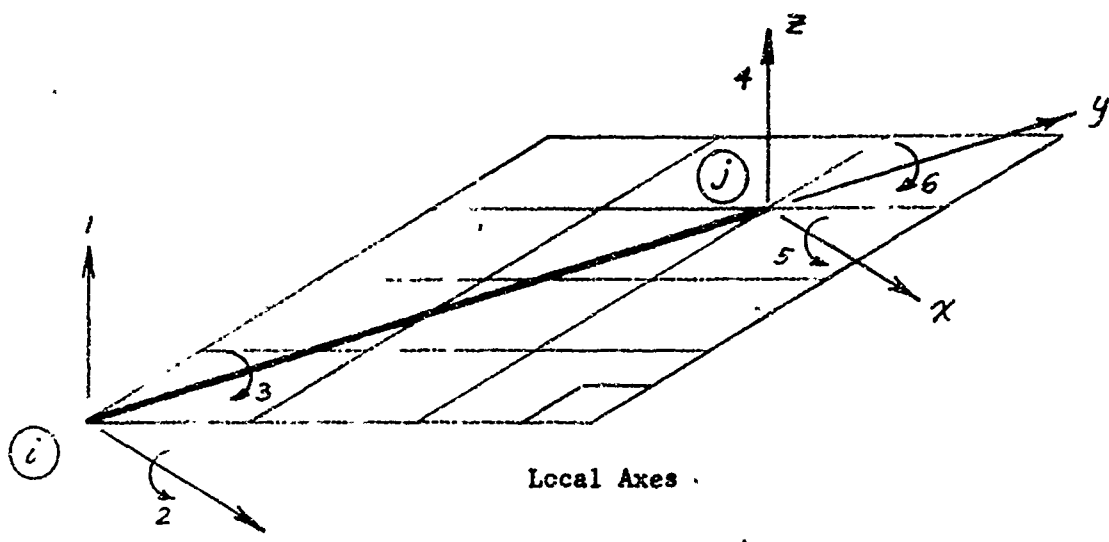


Figure 2. Plane Grid Beam Local and Global Coordinate System

The computer program FLUENC carries out the required operations to obtain the influence coefficient matrix $[f_{NN}]$. A detailed description of the program can be found in Section 4.0. The program is written to form a 50 x 50 influence coefficient matrix. The influence coefficient matrix is punched out on cards in a format compatible with the Collocation Flutter Program.

The plane grid beam global coordinate stiffness matrix used in the program was obtained from Reference 1 and is given in Table 1. The local and global coordinate systems are shown in Figure 2. The figure also contains the sign convention for the six degrees of freedom for each element.

The triangular plate stiffness matrix given in Reference 2 was used in the computer program. The plate element can be materially or geometrically orthotropic as treated in Reference 3. Stiffened plates can be considered to be geometrically orthotropic. The sign convention and nodal degrees of freedom are shown in Figure 3.

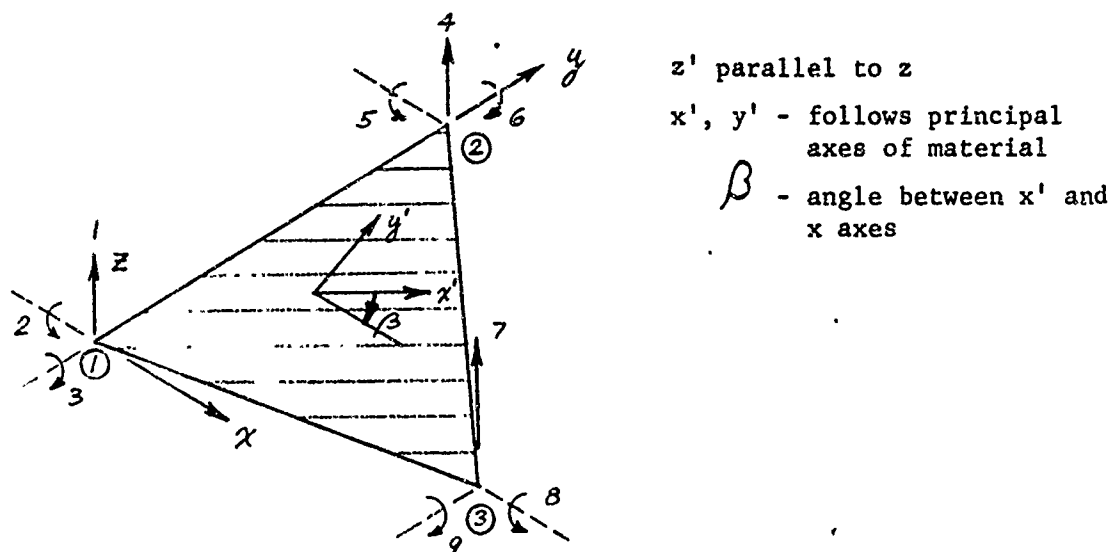


Figure 3. Orthotropic Triangular Element

Following the analysis given in Reference 2, the deflection shape of the plate element is assumed to be of the form

$$w = C_1 + C_2 x + C_3 y + C_4 x^2 + C_5 xy + C_6 y^2 + C_7 x^3 + C_8 (xy^2 + x^2y) + C_9 y^3$$

or

$$w = [N] \{C\} \quad (6)$$

The unknown constants C_1, C_2, \dots, C_9 can be written in terms of the nodal displacements $\delta_1, \delta_2, \dots, \delta_9$ by using the boundary conditions

$$\begin{aligned} \text{at } x=0, y=0 & \quad \begin{cases} w = \delta_1 \\ \partial w / \partial y = \delta_2 \\ \partial w / \partial x = -\delta_3 \end{cases} \\ \text{at } x=0, y=y_2 & \quad \begin{cases} w = \delta_4 \\ \partial w / \partial y = \delta_5 \\ \partial w / \partial x = -\delta_6 \end{cases} \\ \text{at } x=x_3, y=y_3 & \quad \begin{cases} w = \delta_7 \\ \partial w / \partial y = \delta_8 \\ \partial w / \partial x = -\delta_9 \end{cases} \end{aligned} \quad (7)$$

Using Equation (6) in conjunction with the boundary conditions given by Equation (7) yields

$$\begin{Bmatrix} \delta_1 \\ \delta_2 \\ \delta_3 \\ \delta_4 \\ \delta_5 \\ \delta_6 \\ \delta_7 \\ \delta_8 \\ \delta_9 \end{Bmatrix} = [N] \begin{Bmatrix} C_1 \\ C_2 \\ C_3 \\ C_4 \\ C_5 \\ C_6 \\ C_7 \\ C_8 \\ C_9 \end{Bmatrix} \quad (8)$$

where $C =$

$$\begin{bmatrix} 1 & 0 & 0 & 0 & 0 & 0 & 0 & 0 & 0 \\ 0 & 0 & 1 & 0 & 0 & 0 & 0 & 0 & 0 \\ 0 & -1 & 0 & 0 & 0 & 0 & 0 & 0 & 0 \\ 1 & 0 & y_2 & 0 & 0 & y_2^2 & 0 & 0 & y_2^3 \\ 0 & 0 & 1 & 0 & 0 & 2y_2 & 0 & 0 & 3y_2^2 \\ 0 & -1 & 0 & 0 & -y_2 & 0 & 0 & -y_2^2 & 0 \\ 1 & x_3 & y_3 & x_3^2 & x_3 y_3 & y_3^2 & x_3^3 & x_3 y_3^2 + x_3^2 y_3 & y_3^3 \\ 0 & 0 & 1 & 0 & x_3 & 2y_3 & 0 & 2x_3 y_3 + x_3^2 & 3y_3^2 \\ 0 & -1 & 0 & -2x_3 & -y_3 & 0 & -3x_3^2 & -(y_3^2 + 2x_3 y_3) & 0 \end{bmatrix}$$

The constant vector $\{c\}$ can be obtained in terms of the nodal displacements by inverting the matrix $[C]$. Therefore,

$$\{c\} = [C]^{-1} \{s\} \quad (9)$$

The curvatures for a flat plate element are given by

$$\{\epsilon\} = \begin{Bmatrix} \epsilon_x \\ \epsilon_y \\ \epsilon_{xy} \end{Bmatrix} = - \begin{Bmatrix} \partial^2 w / \partial x^2 \\ \partial^2 w / \partial y^2 \\ 2 \partial^2 w / \partial x \partial y \end{Bmatrix} \quad (10)$$

Substituting Equation (6) into Equation (10) yields

$$\{\epsilon\} = [Q] \{c\} \quad (11)$$

where

$$[Q] = \begin{bmatrix} 0 & 0 & 0 & -2 & 0 & 0 & -4x & -2y & 0 \\ 0 & 0 & 0 & 0 & 0 & -2 & 0 & -2x & -6y \\ 0 & 0 & 0 & 0 & -2 & 0 & 0 & -(4x+4y) & 0 \end{bmatrix}$$

Substituting Equation (9) into Equation (11) yields

$$\{\epsilon\} = [Q] [C]^{-1} \{s\} = [B] \{s\} \quad (12)$$

If initial strains are neglected then the moment-curvature relationships can be written in the form

$$\{\sigma\} = \begin{Bmatrix} M_x \\ M_y \\ M_{xy} \end{Bmatrix} = [D] \{\epsilon\} \quad (13)$$

where

$$[D] = \begin{bmatrix} D_x & D_1 & 0 \\ \nu_1 & D_y & 0 \\ 0 & 0 & D_{xy} \end{bmatrix} \quad (14)$$

for a materially or geometrically orthotropic plate. For an isotropic plate Equation (14) reduces to

$$[D] = \frac{E t^3}{12(1-\nu^2)} \begin{bmatrix} 1 & \nu & 0 \\ \nu & 1 & 0 \\ 0 & 0 & \frac{1-\nu}{2} \end{bmatrix} \quad (15)$$

The $[D]$ matrix must undergo a transformation if the principal axes of the material do not coincide with the local coordinate axes. The components of strain in one coordinate axes system are related to the components of strain in another coordinate axes system by the matrix equation

$$\{\epsilon^{\prime}\} = [T]^T \{\epsilon\} \quad (16)$$

(The prime refers to the components of strain referred to the x^{\prime} - y^{\prime} axes in Figure 3)

where

$$[T]^T = \begin{bmatrix} \cos^2 \beta & \sin^2 \beta & -2 \sin \beta \cos \beta \\ \sin^2 \beta & \cos^2 \beta & 2 \sin \beta \cos \beta \\ \sin \beta \cos \beta & -\sin \beta \cos \beta & \cos^2 \beta - \sin^2 \beta \end{bmatrix} \quad (17)$$

Since the internal work is constant no matter which coordinate system is used

$$\{\sigma'\}^T \{\epsilon'\} = \{\sigma\}^T \{\epsilon\} \quad (18)$$

or by Equation (13)

$$\{\epsilon'\}^T [D'] \{\epsilon'\} = \{\epsilon\}^T [D] \{\epsilon\}$$

and by using Equation (16)

$$\{\epsilon\}^T [T] [D'] [T]^T \{\epsilon\} = \{\epsilon\}^T [D] \{\epsilon\}$$

Therefore

$$[D] = [T] [D'] [T]^T \quad (19)$$

The stiffness matrix for a typical element ① ② ③ is given by

$$[K] = \iint_A [B]^T [D] [B] dx dy \quad (20)$$

or by Equations (12) and (19)

$$[K] = [C^{-1}]^T \left(\iint_A [Q]^T [T] [D'] [T]^T [Q] dx dy \right) [C]^{-1} \quad (21)$$

Now let

$$[\bar{D}] = \iint_A [Q]^T [T] [D'] [T]^T [Q] dx dy$$

and carrying out the indicated matrix multiplications yields

$$[\bar{D}] = \iint_A (\text{see Table 1}) dx dy \quad (22)$$

In order to simplify the integration required for evaluating the matrix in Equation (22), it is suggested in Reference 2 that the independent variables be changed as shown in Figure 4.

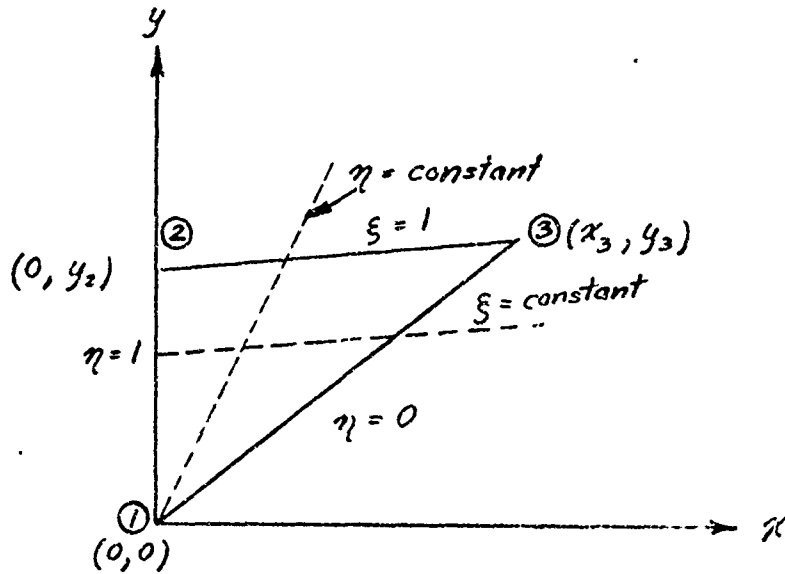


Figure 4
Coordinate Transformation

The relationships

$$\begin{aligned} x &= \xi (1 - \eta) x_3 \\ y &= \xi [(1 - \eta) y_3 + \eta y_2] \end{aligned} \quad (23)$$

are used for the change of variables. The terms in Equation (22) can now be evaluated by using the relationship

$$I(x^m, y^n) = \iint x^m y^n dx dy$$

or

$$I(x^m, y^n) = \iint x^m y^n |J(x, y)| d\xi d\eta \quad (24)$$

where

$$J(x, y) = \begin{vmatrix} \frac{\partial x}{\partial \xi} & \frac{\partial x}{\partial \eta} \\ \frac{\partial y}{\partial \xi} & \frac{\partial y}{\partial \eta} \end{vmatrix} \quad (25)$$

Substituting Equation (23) into Equation (25) yields

$$J(x, y) = \xi x_3 y_2 \quad (26)$$

Substituting Equations (23) and (26) into Equation (24) yields

$$I(x^m, y^n) = \int_0^1 \int_0^1 \xi^{m+n+1} (1-\eta)^m [(1-\eta)y_3 + \eta y_2]^n x_3^{m+1} y_2 d\xi d\eta \quad (27)$$

which can easily be evaluated for any m and n.

3.2 Mass Matrix

D'Alembert's principle can be used for the formulation of the mass matrix. If masses are attached to the nodes of the structure, then the nodal dynamic forces are

$$\{P\} = -[M] \frac{d^2\{\delta\}}{dt^2} \quad (28)$$

where

$$[M] = \begin{bmatrix} M_1 & & & 0 \\ & M_2 & & \\ & & \dots & \\ 0 & & & M_n \end{bmatrix} \quad (29)$$

is a diagonal matrix. The mass of beam and plate elements are usually distributed throughout the structure. Therefore, the distributed pressure loading can be written in the form

$$p = -\rho \frac{d^2 w}{dt^2} \quad (30)$$

Substituting Equations (6) and (9) into Equation (30) yields

$$p = -\rho [N][C]^{-1} \{\ddot{\delta}\}$$

or

$$p = -\rho [R] \{\ddot{\delta}\} \quad (31)$$

where

$$[R] = [N][C]^{-1}$$

Since the equivalent element nodal forces can be computed from the equation

$$\{P\}^e = - \int_V [R]^T p dV \quad (32)$$

then

$$\{P\}^e = \left\{ \int_V [R]^T [R] \rho dV \right\} \{\ddot{\delta}\} \quad (33)$$

Therefore the elemental consistent mass matrix is given by

$$[m]^e = \int [R]^T [R] \rho dV \quad (34)$$

The consistent mass matrices given in Reference 2 (see Tables 3 and 4) are used in the computer program.

Once the elemental consistent mass and/or lumped mass matrices are computed, then the overall matrix is obtained by following the same technique as used in assembling the overall stiffness matrix.

The overall mass matrix is reduced by using Equation (3a). We again assume that we have N control point normal displacements and M displacements which must be eliminated. The overall mass matrix can be written in the form

$$[M] = \begin{bmatrix} M_{NN} & M_{NM} \\ M_{MN} & M_{MM} \end{bmatrix} \quad (35)$$

and the displacements

$$\{\delta\} = \begin{Bmatrix} \delta_N \\ \delta_M \end{Bmatrix} \quad (36)$$

From Equation (3a) we have

$$\{\delta_M\} = -[K_{MM}]^{-1} [K_{MN}] \{\delta_N\} \quad (3a)$$

Since the virtual work of the reduced mass system must equal the virtual work of the true mass system

$$-\{\Delta\delta_N\}^T [M_r] \{\ddot{\delta}_N\} = -\{\Delta\delta\}^T [M] \{\ddot{\delta}\} \quad (37)$$

where

$\{\Delta\delta_N\}$ = virtual displacements of control points

$\{\Delta\delta\}$ = virtual displacements of complete system

$[M_r]$ = overall reduced mass matrix

Equation (37) can be rewritten in the form

$$\{\Delta\delta_N\}^T [M_r] \{\ddot{\delta}_N\} = [\Delta\delta_N^T \quad \Delta\delta_M^T] [M] \begin{Bmatrix} \delta_N \\ \delta_M \end{Bmatrix} \quad (38)$$

Substituting Equation (3a) into Equation (38) yields

$$\{\Delta\delta_N\}^T [M_r] \{\ddot{\delta}_N\} = \{\Delta\delta_N\}^T \left[I \quad -[K_{NM}][K_{MM}]^{-1} \right] [M] \begin{bmatrix} I \\ -[K_{MM}]^{-1}[K_{MN}] \end{bmatrix} \{\ddot{\delta}_N\}$$

which yields the result

$$[M_r] = \left[I \quad -[K_{NM}][K_{MM}]^{-1} \right] [M] \begin{bmatrix} I \\ [K_{MM}]^{-1}[K_{MN}] \end{bmatrix} \quad (39)$$

The reduced mass matrix given by Equation (39) is calculated in the computer program.

3.3 Modes and Frequencies

Since the design engineer may find it useful to know the mode shapes and natural frequencies of the structure, this information can be obtained by using the NMØDE option in the computer program. If no external forces are present then the reduced mass and influence coefficient matrices are related to one another by the relationship

$$[f_{NN}]^{-1} \{ \delta_N \} = - [M_r] \{ \ddot{\delta}_N \} \quad (40)$$

For determining natural frequencies, the deflections $\{ \delta_N \}$ can be written as

$$\{ \delta_N \} = \{ \delta_0 \} \sin \omega t \quad (41)$$

Substituting Equation (41) into Equation (40) yields

$$[f_{NN}]^{-1} \{ \delta_0 \} = \omega^2 [M_r] \{ \delta_0 \} \quad (42)$$

The solution of Equation (42) yields the natural frequencies, ω , and the mode shapes $\{ \delta_0 \}$. Since $[f_{NN}]^{-1}$ and $[M_r]$ are both symmetrical matrices, the mass matrix $[M_r]$ can be triangularized

$$[M_r] = [L][L]^T \quad (43)$$

where

$$[L] = \begin{bmatrix} l_{11} & 0 & 0 & \dots & 0 \\ l_{21} & l_{22} & 0 & \dots & 0 \\ \vdots & \vdots & l_{33} & \dots & 0 \\ l_{n1} & \dots & \dots & \dots & l_{nn} \end{bmatrix}$$

Substituting Equation (43) into Equation (42) yields

$$[f_{NN}]^{-1} \{ \delta_0 \} = \omega^2 [L][L]^T \{ \delta_0 \}$$

$$[L]^{-1} [f_{NN}]^{-1} \{\delta_o\} = \omega^2 [L]^T \{\delta_o\} \quad (44)$$

Since

$$[L^T]^{-1} [L^T] = [I]$$

Equation (44) may be written

$$[L]^{-1} [f_{NN}]^{-1} [L^T]^{-1} [L]^T \{\delta_o\} = \omega^2 [L]^T \{\delta_o\} \quad (44a)$$

or

$$[A] \{\bar{\delta}_o\} = \omega^2 \{\delta_o\} \quad (45)$$

where

$$[A] = [L]^{-1} [f_{NN}]^{-1} [L^T]^{-1}$$

$$\{\bar{\delta}_o\} = [L]^T \{\delta_o\}$$

An eigenvalue subroutine using the Givens method was used in the computer program package to solve Equation (45). The Givens method is fully described in Reference 4.

Note that the dynamical matrix $[A]$ in the form described above is real and symmetric which is required by the Givens method. Conveniently, $[L]$ and $[L^T]$ are in triangular form which is used in the computer program package to save core storage space.

4.0 PROGRAM DESCRIPTION

Computer program FLUENC written in FORTRAN IV carries out the operations set forth in Section 3.0 for generating the structural influence coefficients and mass matrices required by the Collocation Flutter Program. Briefly, the structure is assumed to be representable by a planar network of beams and triangular plate elements connected at discrete joints. At each joint, if there are no restraints, the program assumes three degrees of freedom; that is, one displacement normal to the plane of the structure and two rotations. The program first synthesizes the stiffness and mass matrices for the entire structure including all degrees of freedom from the data input for the beam and triangular plate elements and from the restraint information input for the joints. It then reduces the stiffness and mass matrices by eliminating all the rotational degrees of freedom and leaving only the normal displacements. As a final step, the program inverts the reduced stiffness matrix to obtain the influence coefficients.

Other features of the program include the option to compute lumped masses or to compute the consistent mass matrices for the beam and triangular plate elements or both. Also, the triangular plate elements may have either isotropic or orthotropic properties. There is an additional option to expand the reduced frequency matrix to include the degrees of freedom representing the restraint joint (one joint on a movable surface; two joints on a fixed component). This is accomplished by adding one or two zero rows and columns to the reduced flexibility matrix corresponding to the mass numbers of the attach points involved.

In the sections that follow detailed instructions are given for the preparation of input data and a description is given of the output illustrated with several sample problems. Also included are listings and flow charts of the program and a discussion of the processing requirements.

4.1 Description of Program Input

The following instructions describe the input data, their physical units, and the FORTRAN format they must be punched with. The input quantities' names, all in capitals, are their FORTRAN names and, for reference, their equivalent names in Section 3.0 are listed in Appendix D.

4.1.1 Title Card, format (12A6)

Two cards; any alphanumeric statement in columns 1 to 72.

4.1.2 Problem Size and Control Information, format (7I5)

Column	1 - 5	6 - 10	11 - 15	16 - 20	21 - 25	26 - 30	31 -
Name	NJTS	NR	NBE	NPE	NMØDE	MKEY	NLUMP

- NJTS = number of joints in structure (50 maximum)
 NR = number of joints with one or more restraints
 NBE = number of beam elements in structure
 NPE = number of triangular plate elements in structure
 NMØDE = number of eigenvalues and eigenvectors desired (9 maximum)
 MKEY = 1. do not compute consistent mass terms for beam and/or triangular plate elements
 = 2. compute consistent mass terms for beam and/or triangular plate elements
 NLUMP = number of lumped masses input. Only lumped masses corresponding to the normal displacement at each joint may be input.

4.1.3 Material Properties

(a) Number of Materials, format (I5)

Column	1 - 5
Name	NMAT

NMAT = number of materials for which properties are input (10 max.)

(b) Properties, format (4E10.3)

Input NMAT number of cards, one for each material.

Column	1 - 10	11 - 20	21 - 30	31 - 40
Name	YM(1)	PR(1)	GE(1)	DENS(1)

- YM(i) = Young's modulus of elasticity divided by 10^6 ; psi
 PR(i) = Poisson's ratio
 GE(i) = modulus of rigidity; psi. If input as 0, it will be computed from the following formula:

$$GE(i) = \frac{YM(i)}{2 [1 + PR(i)]}$$

DENS(i) = material density; lb/in³. Not required if MKEY = 1

4.1.4 Joint Coordinate Cards, format (10X, 2E10.3)

Input NJTS number of cards, one for each joint. Also, the structure is assumed to lie in the x-y plane.

Column	1 - 10	11 - 20	21 - 30
Name	m	X(m)	Y(m)

m = joint number (must be input consecutively starting with 1).
May be placed anywhere between columns 1 and 10

X(m) = x coordinate of joint m; inches

Y(m) = y coordinate of joint m; inches

4.1.5 Joint Restraint Information, format (4I5)

Input NR number of cards, one for each joint with one or more restraints.

Column	1 - 5	6 - 10	11 - 15	16 - 20
Name	JT	M1	M2	M3

JT = number of joint having one or more restraints

M1 = 0 free in the z direction

= 1 fixed in the z direction

M2 = 0 free to rotate about the x axis

= 1 fixed about the x axis

M3 = 0 free to rotate about the y axis

= 1 fixed about the y axis

4.1.6 Lumped Masses, format (I5, 5X, E10.3)

Input NLUMP number of cards, one for each lumped mass.

Column	1 - 5	6 - 10	11 - 20
Name	JMASS	blank	RMASS

JMASS = number of joint for which lumped mass is input

RMASS = lumped mass, lb.

If more than one lumped mass is input for a particular joint, the program will sum the masses.

4.1.7 Beam Element Properties, format (3E10.3, 3I5)

Input NBE number of cards, one for each beam element.

Column	1 - 10	11 - 20	21 - 30	31 - 45	36 - 40	41 - 45
Name	AR	XI	YJ	MAT	JTNR	JTFR

AR = area of beam cross section, in²

XI = moment of inertia of area, in⁴

YJ = effective torsional moment of inertia, in⁴

MAT = material code corresponding to one of the materials input under paragraph 4.1.3.

JTNR, JTFR = joint numbers at the ends of the beam element

4.1.8 Triangular Plate Element Properties, format (E10.3, 5I5)

Input NPE number of cards, one for each triangular plate element.

Column	1 - 10	11 - 15	16 - 20	21 - 25	26 - 30	31 - 35
Name	PTH	MAT	JT1	JT2	JT3	NDX

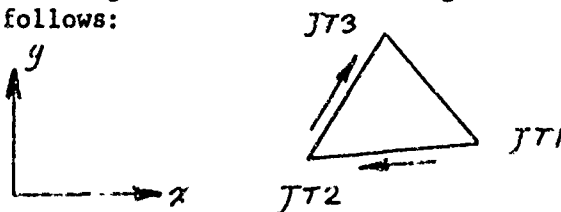
PTH = plate thickness, in.

MAT = material code corresponding to one of the materials input under paragraph 4.1.3

JT1, JT2, JT3 = joint numbers at the three corners of the triangular plate

Restrictions:

- a) The order of the joint numbers must be given in a clockwise manner as follows:



- b) The angle formed by the edges of the triangular plate at JT1 must not be 90°.

NDX = 0 the plate has isotropic properties and the flexural rigidity terms are computed from

$$DX = DY = \frac{YM(MAT) \times PTH^3}{12 \{ 1 - [PR(MAT)]^2 \}}$$

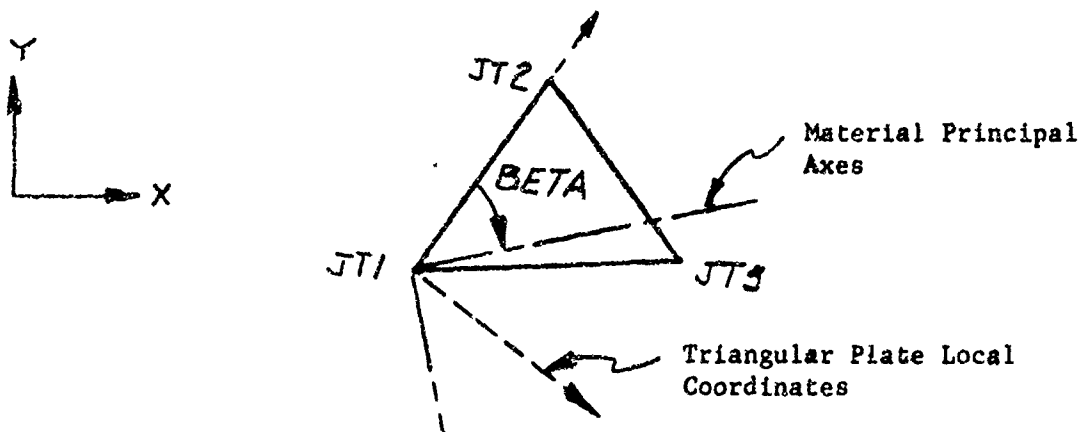
$$D1 = [PR(MAT)] \times DX$$

= 1 the plate has orthotropic properties and the flexural rigidity terms are input by the next card [format (4E10.3)]

Column	1 - 10	11 - 20	21 - 30	31 - 40	41 - 50
Name	DX	DY	D1	DKY	BETA

DX, DY, D1, DKY = flexural rigidity terms, in.lb.

BETA = angle between material principal axes and the triangular plate local coordinates as shown below



4.1.9 Option to Expand Reduced Flexibility Matrix

Note: The following card (NCOD) is always required at the end of all input data for any one particular case, whether or NOT the option is to be executed.

	FORMAT (116)	
Column	1-6	
Name	NCOD	
Item	(1)	

NCOD = 0 Option not executed

= 1 Option executed

IF NCOD = 1, the following card is required

	FORMAT (318)		
Column	1-8	9-16	17-24
Name	NR	NNE	NWO
Item	(1)	(2)	(3)

NR = Number of boundary points used (1 or 2)

NNE = Mass number of first attach point

NWO = Mass number of second attach point, if NR = 2

NWO = 0 or left blank if NR = 1

To input more than one problem, the user need only repeat the cards in paragraph 4.1.1 through 4.1.8 for each additional problem.

4.2 Description of Program Output

The program prints out all the input data for every problem followed by the solution consisting of the reduced upper right triangular stiffness (lb/in), flexibility (in/lb) and weight (lb) matrices as well as the modes and frequencies when these are requested on the card in paragraph 4.1.2. The stiffness, flexibility, and mass matrices that are printed/punched out only contain terms that are associated with the normal displacement "z". This is done so that when the flexibility matrix is used in subsequent collocation flutter analyses only the essential degrees of freedom are included in the flutter analyses. Also, the matrices are reduced to eliminate control points associated with fixed points (boundaries). If it is desirable to include the boundary points, it is only necessary to intersperse rows and columns of zero's at the proper place in the matrices. Immediately following the joint restraint information in the output, the program prints out the coordinate numbers assigned by the program to the normal displacements at each unrestrained joint. The elements in all the reduced output matrices are ordered according to these coordinate numbers.

In addition, the program punches out the entire flexibility and weight matrices row by row with the format (1P6E12.5) which is compatible with the input requirements of the Collocation Flutter Program. Each punched matrix is identified by a little card as the first card.

4.3 Sample Problems

To illustrate the use of program FLUENCE, three sample problems are included in Appendix A. Each sample problem starts with a problem statement and is followed by a listing of the input data and the output of the program. The first sample problem is a simply supported uniform beam composed of five beam segments. The second is a uniform cantilever plate divided into 72 triangular plate elements, and the third is a lumped mass and beam network simulating a missile control surface.

4.4 Processing Requirements

Program FLUENCE has been run on the GE-635 computer and it required about 31,000 cells of core storage. It is expected that the program storage requirement will be about the same on other digital computers. In addition to using the input and output files, 05 and 06, which are standard for the GE-635 computer, the program requires six other peripheral files, five of which are designated in the program by the numeric codes 07, 08, 19, 10 and 11, and the sixth is the card punch file.

There is no general formula for determining the run time required for a problem, but if a GE-635 computer is used, an estimate may be made from the times required for the three sample problems in Appendix A, which are as follows:

Sample Problem No.	No. of Joints	No. of Beam Elements	No. of Plate Elements	Consistent Masses Computed	Lumped Masses Input	No. of Modes & Freqs Computed	Run Time Hr.
1	6	5	0	Yes	No	4	0.0015
2	50	0	72	Yes	No	9	0.0691
3	29	45	0	No	Yes	9	0.0161

4.5 Program Listing and Flow Chart

In the event future changes are needed in the program, a listing of the program is included in Appendix B. The program consists of a MAIN deck, 24 subroutines and one function subprogram. MAIN has the function of reading in data, numbering the coordinates (subroutine COORDN), generating the codes for assembling the stiffness and weight matrices and calling the subroutines which develop the stiffness and mass terms for the beam and triangular plate elements. When the entire stiffness and weight matrices have been established for the whole structure, the MAIN program calls a subroutine which reduces these matrices as discussed before and determines the modes and frequencies as well.

The 24 subroutines and one function subprogram can be divided conveniently into five groups according to their function. The first group consists of those routines that develop the beam stiffness terms; these are TRANS and BEAMK. The second group consists of the routines which determine the beam mass terms; these are TRANS and BEAMM. The third group develops the triangular plate stiffness terms and these are PLATEK, CMAT, MINV, DINMAT, MATMPY, DMAT, DBLINT and PLYMP. The fourth group determines the triangular plate mass terms and these consist of PLATEM, CMAT, MINV, DINMTM, MATMPY, DBLINT and PLYMP. The fifth group of subroutines reduces the stiffness and

and mass matrices, finds the eigenvalues and eigenvectors and outputs the solution. This group is comprised of EIGEN, VIVID, ZRØMAK, ZRØMAM, SYMINV, EIGMAT, BIGMAT, LOOP1, LOOP2, LOOP3 and LOOP4.

Since the program listing is annotated extensively with comment statements, no further explanatory remarks are given here for the program. However, to facilitate the understanding of the interrelationships among the many subroutines, a flow chart of the entire FLUENC program is included in Appendix C.

$\frac{12EI}{L^3}$					
$\frac{6EI}{L^2} m$	$\frac{4EI}{L} m^2 + \frac{GJ}{L} l^2$				
$-\frac{6EI}{L^2} l$	$-\frac{4EI}{L} lm + \frac{GJ}{L} lm$	$\frac{4EI}{L} l^2 + \frac{GJ}{L} m^2$			
$-\frac{12EI}{L^3} m$	$-\frac{6EI}{L^2} m$	$\frac{6EI}{L^2} l$			
$\frac{6EI}{L^2} m$	$\frac{2EI}{L} m^2 - \frac{GJ}{L} l^2$	$-\frac{2EI}{L} lm - \frac{GJ}{L} lm$	$-\frac{6EI}{L^2} m$	$\frac{4EI}{L} m^2 + \frac{GJ}{L} l^2$	
$-\frac{6EI}{L^2} l$	$-\frac{2EI}{L} lm - \frac{GJ}{L} lm$	$\frac{2EI}{L} l^2 - \frac{GJ}{L} m^2$	$\frac{6EI}{L^2} l$	$-\frac{4EI}{L} lm + \frac{GJ}{L} lm$	$\frac{4EI}{L} l^2 + \frac{GJ}{L} m^2$

$$l = \frac{X_j - X_i}{L}$$

$$m = \frac{Y_j - Y_i}{L}$$

X_i, Y_i, X_j, Y_j are the global end coordinates of the beam in Figure 2

Table 1. Plane Grid Beam Stiffness Matrix in Global Coordinates

0	0	0	0	0	0	0	0	0	0	0
0	0	0	0	0	0	0	0	0	0	0
0	0	0	0	0	0	0	0	0	0	0
0	0	0	4D ₁₁	4D ₂₂	4D ₃₃	4D ₄₄	12D ₁₂ x	12D ₁₃ x	4[D ₁₂ y + D ₂₂ x + 2D ₃₃ (x+y)]	12D ₁₂ y
0	0	0	4D ₃₁	4D ₃₂	4D ₃₃	4D ₃₄	4D ₃₅	4D ₃₆	4[D ₁₂ y + D ₂₂ x + 2D ₃₃ (x+y)]	12D ₁₂ y
0	0	0	4D ₄₁	4D ₄₂	4D ₄₃	4D ₄₄	4D ₄₅	4D ₄₆	4[D ₁₂ y + D ₂₂ x + 2D ₃₃ (x+y)]	12D ₁₂ y
0	0	0	12D ₂₁ x	12D ₂₂ x	12D ₂₃ x	12D ₂₄ x	36D ₂₅ x ²	12x[D ₁₂ y + D ₂₂ x + 2D ₃₃ (x+y)]	12x[D ₁₂ y + D ₂₂ x + 2D ₃₃ (x+y)]	36D ₂₅ xy
0	0	0	4(D ₁₂ y + D ₂₂ x) + 8D ₃₃ (x+y)	4(D ₁₂ y + D ₂₂ x) + 8D ₃₃ (x+y)	4(D ₁₂ y + D ₂₂ x) + 8D ₃₃ (x+y)	4(D ₁₂ y + D ₂₂ x) + 8D ₃₃ (x+y)	12(D ₁₂ xy + D ₂₂ x ²) + 24D ₃₃ x(x+y)	4y[D ₁₂ y + D ₂₂ x + 2D ₃₃ (x+y)] + 4x[D ₁₂ y + D ₂₂ x + 2D ₃₃ (x+y)] + 8(x+y)[D ₁₂ y + D ₂₂ x + 2D ₃₃ (x+y)]	12[D ₁₂ y ² + D ₂₂ xy + 2D ₃₃ (xy + y ²)]	
0	0	0	12D ₂₁ y	12D ₂₂ y	12D ₂₃ y	12D ₂₄ y	36D ₂₅ xy	12y[D ₁₂ y + D ₂₂ x + 2D ₃₃ (x+y)]	12y[D ₁₂ y + D ₂₂ x + 2D ₃₃ (x+y)]	36D ₂₅ y ²

where $[D] = \begin{bmatrix} D_{11} & D_{12} & D_{13} \\ D_{21} & D_{22} & D_{23} \\ D_{31} & D_{32} & D_{33} \end{bmatrix}$

Table 2. Integrand Appearing in Equation 22. Triangular Plate Element

$\frac{13}{35} + \frac{6I}{5AL^2}$						
$\frac{11L}{210} + \frac{I}{10AL}$	$\frac{L^2}{105} + \frac{2I}{15A}$	<i>Symmetric</i>				
0	0		$\frac{J}{3A}$			
$\frac{9}{70} - \frac{6I}{5AL^2}$	$\frac{13L}{420} - \frac{I}{10AL}$		0			
$-\frac{13L}{420} + \frac{I}{10AL}$	$-\frac{L^2}{140} - \frac{I}{30A}$		0		$\frac{L^2}{105} + \frac{2I}{15A}$	
0	0		$\frac{J}{6A}$	0	0	$\frac{J}{3A}$

$$[m] = \rho AL$$

Table 3. Consistent Mass Matrix for Beam in Local Coordinates

$$[m] = \rho t [c]^T \iint$$

$$dx dy [c]^{-1}$$

1													
x	x ²												
y	xy	y ²											
x ²	x ³	x ² y	x ⁴										
xy	x ² y	xy ²	x ³ y	x ² y ²									
y ²	xy ²	y ³	x ² y ²	xy ³	y ⁴								
x ³	x ⁴	x ³ y	x ⁵	x ⁴ y	x ³ y ²	x ⁶							
xy ² + x ² y	xy ³ + x ² y ²	xy ³ + x ² y ²	x ² y ² + x ³ y	x ² y ³ + x ³ y ²	xy ⁴ + x ² y ³	x ⁴ y ² + x ⁵ y	x ⁴ y ² + x ⁵ y	(xy ² + x ² y) ²					
y ³	xy ³	y ⁴	x ² y ³	xy ⁴	y ⁵	x ² y ³	x ² y ³	xy ⁵ + xy ⁴					

Table 4. Consistent Mass Matrix for Triangular Plate Element in Local Coordinates

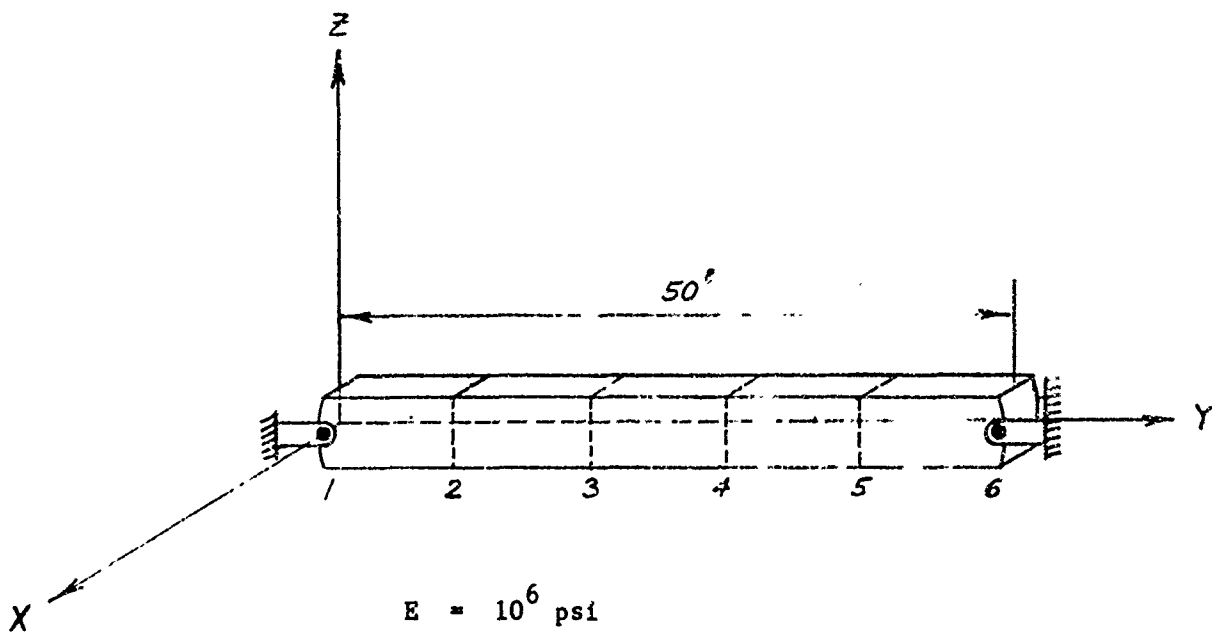
References

1. Tezcan, S. S., "Computer Analysis of Plane and Space Structures", Journal of the Structural Division, ASCE, April 1966
2. Przemieniecki, J. S., "Theory of Matrix Structural Analysis", McGraw-Hill Book Co., New York, 1968
3. Zienkiewicz, O. C., "The Finite Element Method in Structural and Continuum Mechanics", McGraw-Hill Publishing Company Limited, London, 1967
4. Bishop, R. E.D, Gladwell, G. M. L., and Michaelson, S., "The Matrix Analysis of Vibration", Cambridge University Press, London, 1965

APPENDIX A

Three Sample Problems - Input and Output

Sample Problem No. 1
Simply Supported Beam



$$E = 10^6 \text{ psi}$$

$$\nu = 0.33$$

$$\rho = 0.012 \text{ lb/in}^3$$

$$A = 100 \text{ in}^2$$

$$I = 2 \text{ in}^4$$

$$J = 4 \text{ in}^4$$

Calculate first five vibration modes and frequencies using the consistent mass matrix option.

Listing of Input Data Cards

SIMPLY SUPPORTED BEAM WITH 4 JOINTS
AUGUST 1968

	6	7	8	9	4	2
1.	0.	0.11	0.	0.	0.010	
1	0.		0.			
2	0.		10.			
3	0.		20.			
4	0.		30.			
5	0.		40.			
6	0.		50.			
1	1	0	1			
2	1	0	1			
100.	2.		4.		1	2
100.	2.		4.		1	3
100.	2.		4.		1	4
100.	2.		4.		1	5
100.	2.		4.		1	6

NOT REPROGRAMMED

Program Output

~SIMPLY-SUPPORTED BEAM WITH 6 JOINTS
AUGUST 1968

NJTS = 6 NR = 2 NBE = 5 NPE = 0 NMODE = 4 NKEY = 2 NLUMP = 0

MATERIAL PROPERTIES
 NO. YOUNG'S MODULUS POISSON RATIO MODULUS OF RIGIDITY DENSITY
 1 0.10000E 07 0.33000 0.37594E 06 0.12000E-01

JOINT COORDINATES
 JOINT NO. X COORD. Y COORD.
 1 0. 0.
 2 10.00000
 3 0. 20.00000
 4 0. 30.00000
 5 0. 40.00000
 6 0. 50.00000

JOINT RESTRAINT CORE
 JOINT NO. Z DISPLACEMENT ROTATION ABOUT X ROTATION ABOUT Y
 1 1 0 1
 6 1 0 1

COORDINATE NUMBERS FOR EACH Z DISPLACEMENT AT EACH UNRESTRAINED JOINT
 JOINT NO. COORD. NO.
 2 1
 3 2
 4 3
 5 4

BEAM ELEMENT PROPERTIES
 ELEMENT NO. A I J MAT JOINT 1 JOINT 2
 1 100.0000 2.0000 4.0000 1 1 2
 2 100.0000 2.0000 4.0000 1 2 3
 3 100.0000 2.0000 4.0000 1 3 4
 4 100.0000 2.0000 4.0000 1 4 5
 5 100.0000 2.0000 4.0000 1 5 6

REDUCED UPPER TRIANGULAR STIFFNESS MATRIX

ROW 1 0.19751E 05 -0.19005E 05 0.92679E 04 0.20670E 04

ROW 2 0.28019E 05 -0.21072E 05 0.62679E 04

ROW 3 0.28019E 05 -0.19005E 05
ROW 4 0.19751E 05

R E D U C E D U P P E R T R I A N G U L A R F L E X I B I L I T Y M A T R I X

ROW 1 0.53333E-03 0.75000E-03 0.66667E-03 8.38333E-03
ROW 2 0.12000E-02 0.11333E-02 0.66667E-03
ROW 3 0.12000E-02 0.75000E-03
ROW 4 0.53333E-03

R E D U C E D U P P E R T R I A N G U L A R W E I G H T M A T R I X

ROW 1 0.11172E 02 0.93900E 00 -0.56295E 00 0.21468E 00
ROW 2 0.10609E 02 0.11537E 01 -0.56295E 00
ROW 3 0.10609E 02 0.93900E 00
ROW 4 0.11172E 02

HERE ARE THE EIGENVALUES AND EIGENVECTORS

EIGENVECTOR NUMBER 1
CORRESPONDING TO 1.0030593E 04
6.1803364E-01 9.9999962E-01 1.0000000E 00 6.1803416E-01

EIGENVECTOR NUMBER 2
CORRESPONDING TO 1.6120593E 05
1.0000000E 00 6.1803416E-01 -6.1803363E-01 -9.9999960E-01

EIGENVECTOR NUMBER 3
CORRESPONDING TO 8.4178930E 05
1.0000000E 00 -6.1803399E-01 -6.1803401E-01 1.0000000E 00

EIGENVECTOR NUMBER 4
CORRESPONDING TO 2.9858634E 06
-6.1803397E-01 1.0000000E 00 -9.9999993E-01 6.1803399E-01

HERE ARE THE NATURAL FREQUENCIES

THE NATURAL FREQUENCY NUMBER 1 IS 19,948 CPS
THE NATURAL FREQUENCY NUMBER 2 IS 69,981 CPS
THE NATURAL FREQUENCY NUMBER 3 IS 149,963 CPS
THE NATURAL FREQUENCY NUMBER 4 IS 279,924 CPS

SAMPLE PROBLEM NO. 1a

Simply Supported Beam

Identical to Sample Problem 1 with the addition of lumped mass input at joint 3 and 4.

Program Output

SIMPLY SUPPORTED BEAM WITH 6 JOINTS - USING BOTH CONSISTENT MASS MATRIX
 OPTION AND LUMPED MASS INPUT AT JOINTS 3 AND 4.

JNTS = 6 NR = 2 NRE = 5 NPE = 0 NMODE = 4 MKEY = 2 NLUMP = 2

MATERIAL PROPERTIES
 NO. YOUNG'S MODULUS 0.1000E 07 POISSON RATIO 0.33000 MODULUS OF RIGIDITY 0.37594E 06 DENSITY 0.12000E-01

JOINT COORDINATES
 JOINT NO. X COORD. Y COORD.
 1 0. 0.
 2 10.00000
 3 20.00000
 4 30.00000
 5 40.00000
 6 50.00000

JOINT RESTRAINT CODE
 JOINT NO. Z DISPLACEMENT ROTATION ABOUT X ROTATION ABOUT Y
 1 1 0 0
 6 1 0 1

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COORDINATE NUMBERS FOR EACH Z DISPLACEMENT AT EACH UNRESTRAINED JOINT

JOINT NO. COORD. NO.
 1 1
 2 2
 3 3
 4 4
 5 4

LUMPED WEIGHTS
 JOINT NO. WEIGHT
 3 20.0000
 4 30.0000

BEAM ELEMENT PROPERTIES
 ELEMENT NO. AREA I J HAI JOINT 1 JOINT 2
 1 100.0000 2.0000 4.0000 1 2
 2 100.0000 2.0000 4.0000 1 2
 3 100.0000 2.0000 4.0000 1 3
 4 100.0000 2.0000 4.0000 1 4
 5 100.0000 2.0000 4.0000 1 5
 6 100.0000 2.0000 4.0000 1 6

REDUCED SUPPLY TRIANGULAR STIFFNESS MATRIX

ROW 1 0.19751E 05 -0.19005E 05 0.62679E 04 -0.20670E 04

ROW 2 0.28019E 05 -0.21072E 05 0.82679E 04

ROW 3 0.28019E 05 -0.19005E 05

ROW 4 0.19751E 05

P F D U C E D U P P E R T R I A N G U L A R F L E X I B I L I T Y M A T R I X

ROW 1 0.53333E-03 0.75000E-03 0.66667E-03 0.38333E-03

ROW 2 0.12000E-02 0.11333E-02 0.66667E-03

ROW 3 0.12000E-02 0.75000E-03

ROW 4 0.53333E-03

R E D U C E D U P P E R T R I A N G U L A R F L E X I B I L I T Y M A T R I X

ROW 1 0.11172E 02 0.93900E 00 -0.56295E 00 0.21468E 00

ROW 2 0.30609E 02 0.11537E 01 -0.56295E 00

ROW 3 0.40609E 02 0.93900E 00

ROW 4 0.11172E 02

HERE ARE THE EIGENVALUES AND EIGENVECTORS

EIGENVECTOR NUMBER 1
CORRESPONDING TO 1.0004330E 03
6.0476160E-01 9.9286114E-01 1.0000000E 00 6.1375684E-01

EIGENVECTOR NUMBER 2
CORRESPONDING TO 1.0007129E 05
1.0000000E 00 7.2539523E-01 -5.6665892E-01 -8.8326765E-01

EIGENVECTOR NUMBER 3
CORRESPONDING TO 6.7652134E 05
8.9313311E-01 -1.9308592E-01 -1.8372997E-01 1.0000000E 00

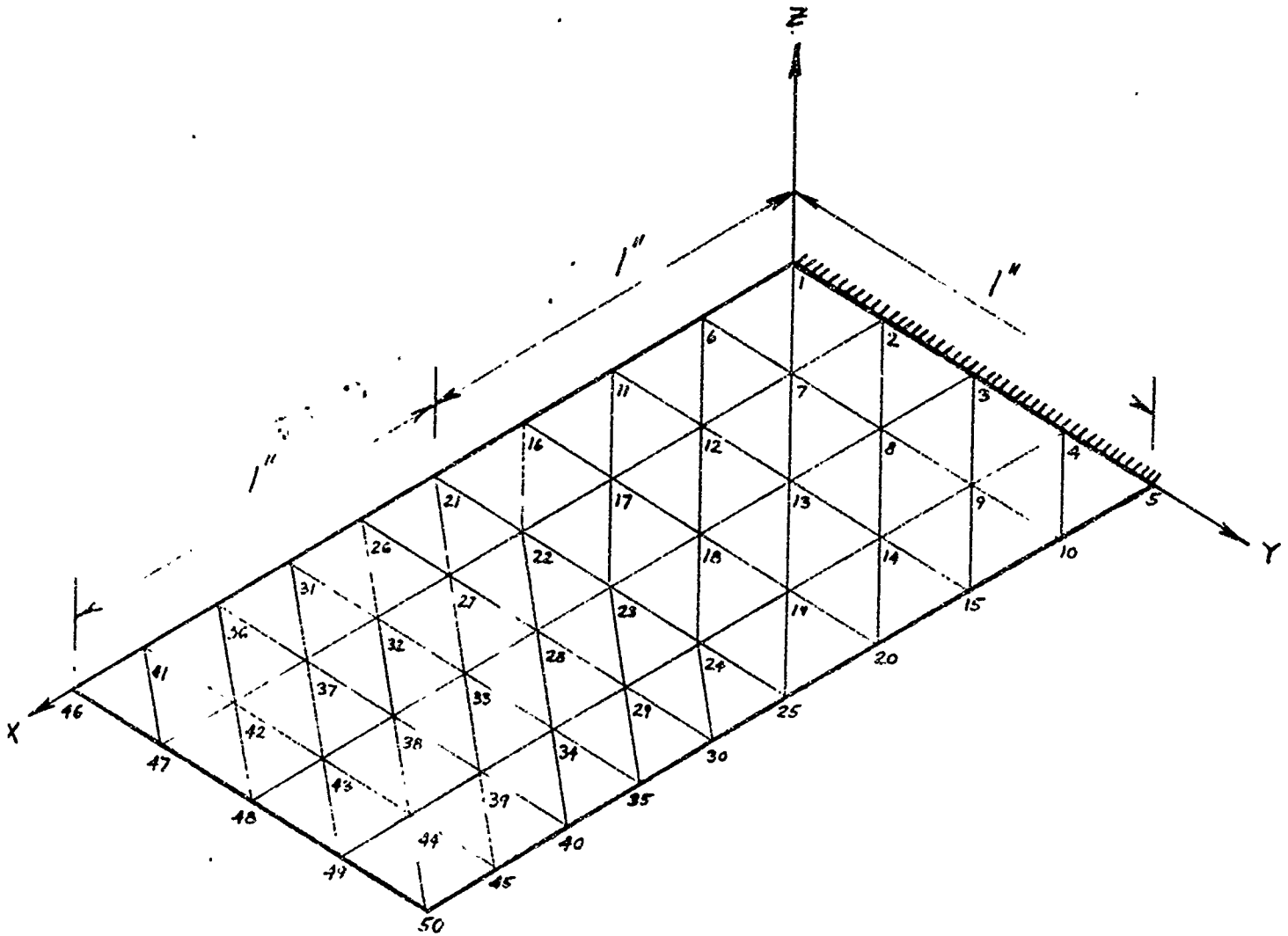
EIGENVECTOR NUMBER 4
CORRESPONDING TO 1.3253830E 06
1.0000000E 00 -5.3894772E-01 3.9625669E-01 -9.1981987E-01

HERE ARE THE NATURAL FREQUENCIES

THE NATURAL FREQUENCY NUMBER	1	1S	10.054	CPS
THE NATURAL FREQUENCY NUMBER	2	1S	50.347	CPS
THE NATURAL FREQUENCY NUMBER	3	1S	130.916	CPS
THE NATURAL FREQUENCY NUMBER	4	1S	183.228	CPS

Sample Problem No. 2

Cantilever Plate



$$E = 3 \times 10^7 \text{ psi}$$

$$\nu = 0.3$$

$$\rho = 0.283 \text{ lb/in}^3$$

$$t = 0.1 \text{ in.}$$

Listing of Input Data Cards

CANTILEVER PLATE WITH 50 JOINTS
AUGUST 1968

	50	0	72	9	2	0
1						
30.	0.3		0.			0.285
1	0.		0.			
2	0.		.25			
3	0.		.5			
4	0.		.75			
5	0.		1.			
6	.25		0.			
7	.25		.25			
8	.25		.5			
9	.25		.75			
10	.25		1.			
11	.5		0.			
12	.5		.25			
13	.5		.5			
14	.5		.75			
15	.5		1.			
16	.75		0.			
17	.75		.25			
18	.75		.5			
19	.75		.75			
20	.75		1.			
21	1.		0.			
22	1.		.25			
23	1.		.5			
24	1.		.75			
25	1.		1.			
26	1.2		0.			
27	1.2		.25			
28	1.2		.5			
29	1.2		.75			
30	1.2		1.			
31	1.4		0.			
32	1.4		.25			
33	1.4		.5			
34	1.4		.75			
35	1.4		1.			
36	1.6		0.			
37	1.6		.25			
38	1.6		.5			
39	1.6		.75			
40	1.6		1.			
41	1.8		0.			
42	1.8		.25			
43	1.8		.5			
44	1.8		.75			
45	1.8		1.			
46	2.0		0.			
47	2.0		.25			
48	2.0		.5			

NOT REPRODUCIBLE

40	2.0	.75		
50	2.0	1.		
1	1	1	1	
2	1	1	1	
3	1	1	1	
4	1	1	1	
5	1	1	1	
0.1	1	1	1	7
0.1	1	1	1	2
0.1	1	1	2	8
0.1	1	1	2	3
0.1	1	1	3	9
0.1	1	1	3	4
0.1	1	1	4	10
0.1	1	1	4	5
0.1	1	1	6	12
0.1	1	1	6	7
0.1	1	1	7	13
0.1	1	1	7	8
0.1	1	1	8	14
0.1	1	1	8	9
0.1	1	1	9	15
0.1	1	1	9	10
0.1	1	1	9	11
0.1	1	1	11	17
0.1	1	1	11	12
0.1	1	1	12	18
0.1	1	1	12	13
0.1	1	1	13	19
0.1	1	1	13	14
0.1	1	1	14	20
0.1	1	1	14	15
0.1	1	1	16	22
0.1	1	1	16	17
0.1	1	1	17	23
0.1	1	1	17	18
0.1	1	1	18	24
0.1	1	1	18	19
0.1	1	1	19	25
0.1	1	1	19	20
0.1	1	1	21	27
0.1	1	1	21	22
0.1	1	1	22	28
0.1	1	1	22	23
0.1	1	1	23	29
0.1	1	1	23	24
0.1	1	1	24	30
0.1	1	1	24	25
0.1	1	1	26	32
0.1	1	1	26	27
0.1	1	1	27	33
0.1	1	1	27	28
0.1	1	1	28	34
0.1	1	1	28	33
0.1	1	1	28	34

NOT REPRODUCIBLE

0.1	1	28	29	34
0.1	1	29	35	34
0.1	1	29	38	35
0.1	1	31	37	36
0.1	1	31	32	7
0.1	1	32	38	37
0.1	1	32	33	38
0.1	1	33	39	38
0.1	1	33	34	39
0.1	1	34	40	39
0.1	1	34	35	40
0.1	1	36	42	41
0.1	1	36	37	42
0.1	1	37	43	42
0.1	1	37	38	43
0.1	1	38	44	43
0.1	1	38	39	44
0.1	1	39	45	44
0.1	1	39	40	45
0.1	1	41	47	46
0.1	1	41	42	47
0.1	1	42	48	47
0.1	1	42	43	48
0.1	1	43	49	48
0.1	1	43	44	49
0.1	1	44	50	49
0.1	1	44	45	50

ELIPTICAL FORM

Program Output

CANTILVER PLATE WITH 50 JOINTS
AUGUST 1968

NJTS = 50 MH = 5 MRE = 0 MPL = .72 MMOD = 9 MKEY = 2 NLUMP = 0

M.A.T.E.R.I.A.L P.R.O.P.E.R.T.I.E.S
 NO. YOUNG'S MODULUS POISSON RATIO MODULUS OF RIGIDITY DENSITY
 1 0.30000E+08 0.30000 0.1150E+08 0.2000E-01

J.O.I.N.T C.O.O.R.D.I.N.A.T.E.S
 JOINT NO. X COORD. Y COORD. I

1	0.	0.	1
2	0.25000	0.	
3	0.50000	0.	
4	0.75000	0.	
5	1.00000	0.	
6	0.25000	0.	
7	0.50000	0.	
8	0.75000	0.	
9	1.00000	0.	
10	0.25000	0.	
11	0.50000	0.	
12	0.75000	0.	
13	1.00000	0.	
14	0.25000	0.	
15	0.50000	0.	
16	0.75000	0.	
17	1.00000	0.	
18	0.25000	0.	
19	0.50000	0.	
20	0.75000	0.	
21	1.00000	0.	
22	0.25000	0.	
23	0.50000	0.	
24	0.75000	0.	
25	1.00000	0.	
26	0.25000	0.	
27	0.50000	0.	
28	0.75000	0.	
29	1.00000	0.	
30	0.25000	0.	
31	0.50000	0.	
32	0.75000	0.	
33	1.00000	0.	
34	0.25000	0.	
35	0.50000	0.	
36	0.75000	0.	
37	1.00000	0.	
38	0.25000	0.	
39	0.50000	0.	
40	0.75000	0.	
41	1.00000	0.	
42	0.25000	0.	
43	0.50000	0.	
44	0.75000	0.	
45	1.00000	0.	

NOT REPRODUCIBLE

46	2.00000	0.20000
47	2.00000	0.50000
48	2.00000	0.70000
49	2.00000	1.00000
50	2.00000	1.00000

JOINT RESTRAINT CODE *****
 JOINT NO. Z DISPLACEMENT ROTATION ABOUT X ROTATION ABOUT Y

1	1	1	1	1
2	1	1	1	1
3	1	1	1	1
4	1	1	1	1
5	1	1	1	1

COORDINATE NUMBERS FOR EACH Z DISPLACEMENT AT EACH UNRESTRAINED JOINT

JOINT NO.	COORD. NO.
6	1
7	2
8	1
9	1
10	5
11	6
12	7
13	8
14	9
15	10
16	11
17	12
18	13
19	14
20	15
21	16
22	17
23	18
24	19
25	20
26	21
27	22
28	23
29	24
30	25
31	26
32	27
33	28
34	29
35	30
36	31
37	32
38	33
39	34
40	35
41	36
42	37
43	38
44	39
45	40
46	41
47	42
48	43
49	44
50	45

43	44	45	46	47	48	49	50	51	52	53	54	55	56	57	58	59	60
0.1000	0.1000	0.1000	0.1000	0.1000	0.1000	0.1000	0.1000	0.1000	0.1000	0.1000	0.1000	0.1000	0.1000	0.1000	0.1000	0.1000	0.1000
0.27473E 04	0.27473E 04	0.27473E 04	0.27473E 04	0.27473E 04	0.27473E 04	0.27473E 04	0.27473E 04	0.27473E 04	0.27473E 04	0.27473E 04	0.27473E 04	0.27473E 04	0.27473E 04	0.27473E 04	0.27473E 04	0.27473E 04	0.27473E 04
0.82418E 03	0.82418E 03	0.82418E 03	0.82418E 03	0.82418E 03	0.82418E 03	0.82418E 03	0.82418E 03	0.82418E 03	0.82418E 03	0.82418E 03	0.82418E 03	0.82418E 03	0.82418E 03	0.82418E 03	0.82418E 03	0.82418E 03	0.82418E 03
0.96154E 03	0.96154E 03	0.96154E 03	0.96154E 03	0.96154E 03	0.96154E 03	0.96154E 03	0.96154E 03	0.96154E 03	0.96154E 03	0.96154E 03	0.96154E 03	0.96154E 03	0.96154E 03	0.96154E 03	0.96154E 03	0.96154E 03	0.96154E 03

RE D U C E D U P P E R T R I A N G U L A R S I E F F N E S S M A T R I X

0.158492E 06	0.958880E 05	-0.22117E 04	0.58055E 04	-0.10982E 00	-0.62329E 05	0.48326E 02	0.83242E 02	0.55807E 01	0.42755E 01
0.56824E 05	0.63880E 05	-0.22117E 04	0.58055E 04	-0.10982E 00	-0.62329E 05	0.48326E 02	0.83242E 02	0.55807E 01	0.42755E 01
-0.13656E 03	-0.23829E 04	0.48208E 04	0.95578E 03	0.15193E 03	0.61424E 02	-0.14328E 04	-0.28584E 04	-0.11340E 03	-0.31340E 03
-0.18435E 04	-0.96810E 02	-0.22885E 02	0.43144E 03	0.85105E 03	0.42876E 03	0.56185E 02	0.94839E 01	0.55807E 01	0.42755E 01
-0.23900E 03	-0.13544E 03	-0.24879E 02	-0.37753E 01	0.18083E 02	0.44329E 02	0.26342E 02	0.55807E 01	0.42755E 01	0.42755E 01

0.13735E 07	0.51840E 06	0.19888E 06	-0.38079E 05	0.29900E 05	-0.48473E 06	-0.69075E 05	0.41854E 05	-0.45124E 05
0.55155E 02	0.14360E 06	0.51281E 05	0.23845E 05	0.12713E 04	-0.94555E 05	-0.34281E 05	-0.21320E 05	-0.34918E 04
-0.65325E 03	0.68250E 03	0.96630E 04	0.84179E 04	0.19618E 04	-0.10710E 03	-0.21515E 05	-0.23586E 04	-0.25243E 04
-0.82498E 03	-0.17917E 03	0.71923E 02	0.29994E 03	0.72275E 03	0.32184E 03	0.68783E 02	-0.19415E 02	-0.16382E 03
-0.10975E 03	-0.10121E 03	-0.28248E 02	-0.28248E 02	0.23162E 02	0.33249E 02	0.10464E 02	0.71997E 01	0.71997E 01

0.15662E 07	0.50503E 06	0.87220E 05	0.15932E 05	0.52338E 04	-0.44967E 06	-0.66763E 05	0.42249E 05	-0.36398E 04
0.12358E 05	0.13379E 06	0.58593E 05	0.34355E 04	0.57668E 03	-0.31149E 04	-0.32033E 05	-0.21294E 05	-0.43986E 04
-0.18881E 03	0.10659E 04	0.88848E 04	0.84574E 04	0.22377E 04	0.45019E 02	-0.27998E 05	-0.21288E 04	-0.24693E 04
-0.91688E 03	-0.12485E 02	0.71883E 02	0.22856E 03	0.69529E 03	0.35446E 03	0.34873E 01	-0.16228E 02	-0.12276E 03
-0.17640E 03	-0.11381E 03	-0.56691E 00	0.16993E 01	0.19918E 02	0.27754E 02	0.23978E 02	0.23978E 02	0.23978E 02

0.13498E 07	0.15588E 06	-0.63592E 04	0.26778E 05	0.41272E 04	-0.48338E 06	-0.52917E 05	0.99684E 03	-0.63327E 04
0.17391E 03	0.14398E 06	0.53943E 05	0.12791E 05	0.67898E 03	-0.38873E 04	-0.34857E 05	-0.22987E 05	-0.68883E 02
-0.22059E 03	0.12534E 04	0.95655E 04	0.89692E 04	-0.10302E 02	-0.55559E 02	-0.39880E 03	-0.21878E 04	-0.26878E 04
0.51469E 01	-0.11783E 02	0.11421E 03	0.28541E 03	0.80802E 03	-0.15287E 01	0.38778E 01	-0.31888E 02	-0.18351E 03
-0.21868E 03	0.38585E 00	-0.18283E 01	0.59783E 01	0.12843E 02	0.28457E 02	0.28457E 02	0.28457E 02	0.28457E 02

0.48833E 06	0.18169E 04	-0.88997E 04	0.23129E 05	0.12883E 05	-0.21731E 06	-0.33881E 03	0.21288E 04	-0.67875E 04
0.93579E 04	0.68639E 05	-0.88498E 01	0.29467E 02	0.18844E 03	-0.35499E 03	-0.15234E 05	0.24882E 02	-0.14538E 03
-0.32516E 03	-0.57621E 03	0.47284E 04	0.42386E 01	0.31177E 02	-0.92873E 02	0.28577E 03	-0.11884E 04	-0.15427E 01
-0.85534E 01	0.26762E 02	-0.63543E 02	0.29887E 03	-0.44343E 00	0.24246E 01	-0.79428E 01	0.19735E 02	-0.18485E 02
-0.18681E 08	-0.59884E 08	0.18758E 01	-0.46714E 01	0.12213E 02	0.12213E 02	0.12213E 02	0.12213E 02	0.12213E 02

0.35117E 06	0.19898E 06	0.99973E 05	-0.35879E 05	0.71678E 04	-0.18485E 06	-0.57658E 05	0.36518E 05	-0.17812E 05
0.38780E 04	0.68126E 05	0.66477E 05	-0.41255E 03	0.26388E 04	-0.45851E 02	-0.71697E 05	-0.38181E 05	-0.48803E 04
-0.13647E 03	-0.22970E 03	0.57388E 04	0.95677E 04	0.24432E 04	0.15446E 02	0.71878E 02	-0.17289E 04	-0.29523E 04
-0.11678E 04	-0.48480E 07	-0.14518E 02	0.45328E 03	0.85374E 03	0.39888E 03	0.35779E 02	0.40649E 01	-0.72577E 02
-0.16199E 03	-0.88438E 02	-0.18345E 02	-0.18076E 01	0.65337E 03	0.27573E 03	0.58824E 02	-0.61988E 01	-0.86181E 02

0.11572E 07	0.58778E 06	0.19554E 06	-0.36196E 05	0.31551E 05	-0.43975E 06	-0.58468E 05	0.48826E 05	-0.13465E 05
0.81272E 03	0.14817E 06	0.21986E 05	0.89733E 03	0.82513E 03	-0.78962E 03	-0.48143E 05	-0.24513E 05	-0.35176E 04
-0.23341E 03	0.48809E 03	0.93280E 04	0.16679E 04	0.12621E 03	0.16313E 03	-0.23588E 04	-0.224361E 04	-0.224361E 04
-0.79484E 03	-0.18868E 03	0.48276E 02	0.54800E 03	0.65337E 03	0.27573E 03	0.58824E 02	-0.61988E 01	-0.86181E 02
-0.11618E 03	-0.52521E 07	-0.15371E 02	0.54800E 03	0.65337E 03	0.27573E 03	0.58824E 02	-0.61988E 01	-0.86181E 02

0.23583E 07	-0.51133E 06	0.18143E 06	0.15176E 05	0.49851E 04	-0.48989E 06	-0.56175E 05	0.26921E 05	-0.33864E 04
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0.13049F 05 0.13089F 06 0.51492E 05 0.62446E 03 0.33046E 04 -0.38530E 05 -0.24077E 05 -0.43752E 04
-0.23088E 03 0.10993E 04 0.08527E 04 0.01891E 04 0.19621E 04 0.52598E 02 -0.27812E 03 -0.21881E 04 -0.24816E 04
-0.90307E 03 -0.15751E 02 0.61999E 02 0.50149E 03 0.63386E 03 0.31198E 03 0.25867E 01 -0.67848E 01 -0.88471E 02
-0.10324E 03 -0.67642E 02

ROW 9
0.31651F 07 -0.19591F 06 0.62174E 04 0.28216E 05 0.45631E 04 -0.44331E 06 -0.36827E 05 0.95424E 03 -0.88818E 04
0.31822E 05 0.14446E 06 0.53382E 05 -0.12568E 03 0.11858E 04 -0.32188E 04 -0.41348E 05 -0.25752E 03 0.71189E 02
-0.38878E 03 0.17832E 04 0.95347E 04 0.08715E 04 -0.12512E 02 0.74619E 02 -0.34256E 03 -0.22324E 04 -0.26898E 04
0.32584E 01 -0.21363F 02 0.96348E 02 0.47031E 03 0.73569E 03 -0.89139E 08 0.48261E 01 -0.17623E 02 -0.52972E 02
-0.13869F 03

ROW 10
0.36880E 00 0.18220E 04 -0.89781E 04 0.22910E 05 0.18003E 05 -0.19105E 06 -0.38717E 03 0.20156E 04 -0.65248E 04
0.73738E 04 0.58928E 05 0.28881E 02 0.28285E 03 -0.17725E 03 0.93778E 03 -0.17984E 05 0.11649E 01 -0.59184E 02
0.39258E 03 -0.39688E 03 0.39534E 03 -0.39534E 03 0.39534E 03 -0.52562E 02 0.31977E 03 -0.11564E 04 0.96718E 08
-0.69746E 01 0.28589E 02 0.68175E 02 0.28809E 03 -0.27491E 08 0.18395E 01 -0.53933E 01 0.16976E 02 -0.47756E 02

ROW 11
0.35248E 06 -0.18835E 06 0.18849E 06 -0.35925E 05 0.71471E 04 -0.28277E 06 -0.21813E 05 0.36681E 03 -0.16723E 05
0.37913E 04 0.78366F 05 0.88819E 05 -0.5393E 04 0.44591E 04 -0.52396E 03 -0.23873E 05 -0.28822E 05 -0.47497E 04
0.37492E 03 -0.26396E 03 0.69733E 04 0.94862E 04 0.27286E 04 -0.17648E 03 5.46339E 02 -0.18418E 04 -0.29129E 04
-0.18530E 04 -0.89578E 01 0.53530E 01 0.29566E 03 0.56742E 03 9.22286E 03 0.98868E 01 0.54648E 08

ROW 12
0.11533E 07 -0.58680F 06 0.19565E 06 -0.36275E 05 0.25794E 05 -0.47528E 06 -0.77388E 05 0.43939E 05 -0.14273E 05
0.2527E 04 0.17805E 06 0.66318E 05 -0.36886E 04 0.18327E 04 -0.55323E 03 -0.45737E 05 -0.21782E 05 -0.48874E 04
0.32838E 03 0.52112E 03 0.18583E 05 0.88438E 04 0.18522E 04 0.17859E 03 -0.12866E 03 -0.24589E 04 -0.22988E 04
-0.66012E 03 -0.18442E 03 0.14685E 02 0.39107E 03 0.41716E 03 0.13917E 03 0.29330E 02

ROW 13
0.13648E 07 -0.58888F 06 0.18133E 06 0.15953E 05 -0.47495E 05 -0.44383E 06 -0.74388E 05 0.29896E 03 -0.56694E 04
0.16471E 05 0.16888E 06 0.65988E 05 0.66766E 03 0.11809E 03 0.14518E 04 -0.44383E 05 -0.22416E 05 -0.48498E 04
-0.43282E 03 0.96850E 03 0.98912E 04 0.80463E 04 0.17208E 04 0.98581E 02 -0.16725E 03 -0.22836E 04 -0.22642E 04
-0.74648F 03 -0.16894E 02 0.93629E 01 0.36483E 03 0.38469E 03 0.17899E 03

ROW 14
0.11623E 07 -0.19689F 06 -0.65344E 04 0.29482E 05 -0.63582E 03 -0.48895E 06 -0.48893E 05 0.17445E 04 -0.92160E 04
0.16953E 05 0.18182E 06 0.63876E 05 -0.21955E 03 0.79995E 03 0.18751E 04 -0.45739E 05 -0.25238E 05 0.14488E 03
-0.44786E 03 0.11684F 04 0.38283E 05 0.88173E 04 -0.23892E 02 0.73612E 02 -0.27899E 03 -0.22799E 04 -0.25778E 04
0.57887E 01 -0.14869E 02 0.48880E 02 0.31016E 03 0.49884E 03

ROW 15
0.36446E 06 0.18134E 04 -0.87778E 04 0.22488E 05 0.20088E 05 -0.28934E 06 -0.59149E 03 0.21659E 04 -0.74188E 04
0.7594E 04 0.76567E 05 -0.26882E 02 0.33247E 03 -0.25252E 03 0.16419E 04 -0.19228E 05 -0.28826E 02 0.79496E 01
-0.85285E 01 -0.88557E 03 0.48738E 04 -0.24177E 08 0.27442E 02 -0.5573E 02 0.25152E 03 -0.11998E 04 0.16899E 01
-0.77578E 01 0.18294E 02 -0.67515E 02 0.28835E 03

ROW 16
0.44138E 06 -0.17758E 06 0.98868E 05 -0.33875E 05 0.67856E 04 -0.38584E 06 -0.75217E 05 0.36485E 03 -0.14779E 05
0.3886E 04 0.18738F 06 0.99396E 05 -0.18739E 05 0.68247E 04 -0.88783E 03 -0.32572E 05 -0.35578E 05 -0.53588E 04
0.49542E 03 -0.35745E 03 0.84581E 04 0.33331E 05 -0.27472E 04 -0.27255E 03 0.77618E 02 -0.13881E 04 -0.23577E 04
-0.65077E 03 0.14396F 02 -0.3886E 01

ROW 17
0.13178E 07 -0.47859E 06 0.17585E 06 -0.32448E 05 0.45997E 05 -0.68852E 06 -0.61988E 05 0.34231E 03 -0.18229E 05
-0.35887E 04 0.27734E 06 0.63832E 05 -0.92299E 04 0.24819E 04 0.16215E 04 -0.57787E 05 -0.27372E 05 -0.41777E 04
0.75374E 03 -0.78641E 02 0.12381E 05 0.94540E 04 0.15463E 04 0.36639E 02 0.78415E 02 -0.19899E 04 -0.17693E 04
-0.36195F 03 -0.68857E 02

ROW 18
0.15178E 07 -0.47894E 06 0.91262E 05 0.17044E 05 0.97879E 04 -0.68886E 06 -0.59298E 05 0.24575E 03 -0.41931E 04
0.1739E 05 0.21683E 06 0.82553E 05 0.42250E 03 0.49399E 03 -0.12517E 04 -0.56935E 05 -0.28268E 05 -0.55362E 04
-0.36818E 03 0.18118E 04 0.11957E 05 0.94316E 04 0.19613E 04 0.35537E 02 -0.66616E 02 -0.19423E 04 -0.16655E 04
-0.55416E 03

0.17585F 07 -0.48728E 06 0.65008E 05 0.15448E 04 0.21303E 05 -0.67639E 06 -0.54348E 05 0.31258E 04 -0.84098E 03
 0.61312E 03 0.15221E 06 0.33543E 05 -0.13831E 04

ROW 34
 0.15746E 07 -0.19745E 06 -0.81659E 04 0.43493E 05 0.19506E 05 -0.71017E 06 -0.26926E 05 -0.14952E 03 -0.59496E 04
 0.17563E 04 0.15235E 06 0.34188E 05

ROW 35
 0.56827E 06 0.24849E 04 -0.13506E 05 0.29938E 05 0.38170E 05 -0.32243E 06 0.52442E 02 0.14828E 04 -0.54193E 04
 0.13498E 03 0.21844E 05

ROW 36
 0.39709E 06 -0.21258E 04 0.77381E 05 -0.29837E 05 0.59883E 04 -0.17768E 06 0.44879E 04 0.48167E 05 -0.23387E 05
 0.59849E 04

ROW 37
 0.13551E 07 -0.54129E 06 0.17814E 06 -0.33274E 05 0.65615E 05 -0.59277E 06 0.38498E 05 0.38182E 05 -0.11616E 05

ROW 38
 0.15786E 07 -0.25175E 06 0.90535E 05 0.54714E 04 0.92647E 05 -0.40788E 06 0.55173E 05 0.16593E 02

ROW 39
 0.13663E 07 -0.24518E 06 -0.33837E 04 0.57444E 04 0.95885E 05 -0.40488E 06 0.44781E 05

ROW 40
 0.48428E 06 0.94742E 03 -0.38154E 04 0.28256E 04 0.83852E 05 -0.18737E 06

ROW 41
 0.18859E 06 -0.11884E 06 0.58978E 05 -0.21853E 05 0.49778E 04

ROW 42
 0.48597E 06 -0.14789E 06 0.14888E 06 -0.32388E 05

ROW 43
 0.68876E 06 -0.3873F 06 0.85332E 05

ROW 44
 0.55298E 06 -0.17597E 06

ROW 45
 0.17383E 06

P F D U C E R U P P E R T R I A N G U L A R F L E X I B I L I T Y M A T R I X

ROW 1
 0.92988E-05 0.37856E-05 0.16145E-05 0.73691E-06 0.55368E-07 0.15767E-04 0.18815E-04 0.58149E-05 0.32322E-02
 0.12937E-05 0.28012E-04 0.14958E-04 0.18435E-04 0.68436E-05 0.38731E-05 0.23989E-04 0.19381E-04 0.14844E-04
 0.19837E-04 0.2237E-05 0.27163E-04 0.1638E-04 0.14103E-04 0.10283E-04 0.38352E-04 0.25917E-04
 0.21567E-04 0.17374E-04 0.33581E-04 0.29181E-04 0.24868E-04 0.28637E-04 0.16493E-04 0.36788E-04
 0.32436E-04 0.29138E-04 0.19786E-04 0.40824E-04 0.35886E-04 0.31885E-04 0.22140E-04 0.22937E-04

ROW 2
 0.33727E-05 0.28889E-05 1.13218E-05 0.51092E-06 0.85699E-05 0.74597E-05 0.58253E-05 0.41237E-05 0.27659E-05
 0.12889E-04 0.11264E-04 0.95479E-05 0.7473E-05 0.68538E-05 0.16738E-04 0.15845E-04 0.13298E-04 0.11489E-04
 0.97889E-05 0.18889E-04 0.18889E-04 0.14586E-04 0.12708E-04 0.22859E-04 0.21899E-04 0.21899E-04 0.19323E-04
 0.17539E-04 0.15735E-04 0.2582E-04 0.22353E-04 0.28564E-04 0.18769E-04 0.28941E-04 0.27167E-04
 0.25386E-04 0.21588E-04 0.21886E-04 0.38283E-04 0.28421E-04 0.26634E-04 0.24843E-04

ROW 3
 0.27446E-05 0.19422E-05 0.18671E-05 0.49852E-05 0.54866E-05 0.58464E-05 0.53489E-05 0.43832E-05 0.85783E-05
 0.8897E-05 0.89978E-05 0.87131E-05 0.12222E-04 0.12328E-04 0.12324E-04 0.12324E-04 0.12125E-04 0.11768E-04
 0.15889E-04 0.15184E-04 0.15853E-04 0.14887E-04 0.14627E-04 0.17931E-04 0.17895E-04 0.17817E-04 0.17678E-04
 0.17468E-04 0.29759E-04 0.28894E-04 0.20601E-04 0.20465E-04 0.28295E-04 0.23577E-04 0.21408E-04 0.23388E-04
 0.2138E-04 0.2313E-04 0.26388E-04 0.2013E-04 0.2013E-04 0.2013E-04 0.2013E-04 0.2013E-04 0.2013E-04

ROW 4
 0.27446E-05 0.19422E-05 0.18671E-05 0.49852E-05 0.54866E-05 0.58464E-05 0.53489E-05 0.43832E-05 0.85783E-05
 0.8897E-05 0.89978E-05 0.87131E-05 0.12222E-04 0.12328E-04 0.12324E-04 0.12324E-04 0.12125E-04 0.11768E-04
 0.15889E-04 0.15184E-04 0.15853E-04 0.14887E-04 0.14627E-04 0.17931E-04 0.17895E-04 0.17817E-04 0.17678E-04
 0.17468E-04 0.29759E-04 0.28894E-04 0.20601E-04 0.20465E-04 0.28295E-04 0.23577E-04 0.21408E-04 0.23388E-04
 0.2138E-04 0.2313E-04 0.26388E-04 0.2013E-04 0.2013E-04 0.2013E-04 0.2013E-04 0.2013E-04 0.2013E-04

Table with 10 columns of numerical data. Rows 1-5 are labeled 'ROW 5' and rows 6-10 are labeled 'ROW 6'.

Table with 10 columns of numerical data. Rows 11-15 are labeled 'ROW 7' and rows 16-20 are labeled 'ROW 8'.

Table with 10 columns of numerical data. Rows 21-25 are labeled 'ROW 9' and rows 26-30 are labeled 'ROW 10'.

Table with 10 columns of numerical data. Rows 31-35 are labeled 'ROW 11' and rows 36-40 are labeled 'ROW 12'.

Table with 10 columns of numerical data. Rows 41-45 are labeled 'ROW 13' and rows 46-50 are labeled 'ROW 14'.

Table with 10 columns of numerical data. Rows 51-55 are labeled 'ROW 15' and rows 56-60 are labeled 'ROW 16'.

Table with 10 columns of numerical data. Rows 61-65 are labeled 'ROW 17' and rows 66-70 are labeled 'ROW 18'.

Table with 10 columns of numerical data. Rows 71-75 are labeled 'ROW 19' and rows 76-80 are labeled 'ROW 20'.

Table with 10 columns of numerical data. Rows 81-85 are labeled 'ROW 21' and rows 86-90 are labeled 'ROW 22'.

Table with 10 columns of numerical data. Rows 91-95 are labeled 'ROW 23' and rows 96-100 are labeled 'ROW 24'.

0.55423E-03	0.49007E-03	0.44853E-03	0.40187E-03	0.35755E-03	0.65858E-03	0.60270E-03	0.54922E-03	0.49420E-03
0.45151E-03	0.75982E-03	0.70428E-03	0.65002E-03	0.59822E-03	0.54848E-03	0.66858E-03	0.89563E-03	0.75842E-03
0.69771E-03	0.64699E-03							
ROW 27								
0.47059E-03	0.44043E-03	0.41047E-03	0.38147E-03	0.35902E-03	0.56983E-03	0.53788E-03	0.50649E-03	0.47583E-03
0.69763E-03	0.66683E-03	0.63530E-03	0.60351E-03	0.57204E-03	0.79570E-03	0.76453E-03	0.73277E-03	0.70175E-03
0.66805E-03								
ROW 28								
0.43166E-03	0.41970E-03	0.40804E-03	0.39624E-03	0.38455E-03	0.25148E-03	0.24013E-03	0.22813E-03	0.21688E-03
0.62966E-03	0.61975E-03	0.60848E-03	0.59674E-03	0.58466E-03	0.72430E-03	0.71422E-03	0.70363E-03	0.69186E-03
0.42859E-03	0.41465E-03	0.40147E-03	0.38911E-03	0.37690E-03	0.52044E-03	0.50285E-03	0.48209E-03	0.45912E-03
0.68289E-03	0.61188E-03	0.62094E-03	0.60707E-03	0.59311E-03	0.69333E-03	0.70331E-03	0.71276E-03	
ROW 29								
0.46212E-03	0.44145E-03	0.42466E-03	0.40947E-03	0.39451E-03	0.55649E-03	0.52711E-03	0.50418E-03	0.48276E-03
0.61336E-03	0.64518E-03	0.61359E-03	0.64179E-03	0.67851E-03	0.70144E-03	0.71111E-03	0.72541E-03	0.74492E-03
ROW 30								
0.80123E-03	0.73474E-03	0.67116E-03	0.61569E-03	0.56156E-03	0.93731E-03	0.86964E-03	0.80475E-03	0.74492E-03
0.68627E-03	0.18786E-02	0.18827E-02	0.91640E-03	0.87354E-03	0.81389E-03			
ROW 31								
0.69915E-03	0.66174E-03	0.62447E-03	0.58817E-03	0.56533E-03	0.82426E-03	0.78969E-03	0.75885E-03	0.71285E-03
0.99477E-03	0.95884E-03	0.91767E-03	0.87880E-03	0.83922E-03				
ROW 32								
0.64943E-03	0.63372E-03	0.61689E-03	0.70883E-03	0.77341E-03	0.75801E-03	0.74118E-03	0.72295E-03	
0.91893E-03	0.89762E-03	0.88238E-03	0.86583E-03					
ROW 33								
0.64252E-03	0.64855E-03	0.73476E-03	0.74428E-03	0.75632E-03	0.76388E-03	0.77176E-03	0.85448E-03	0.86441E-03
0.87489E-03	0.88492E-03	0.89374E-03						
ROW 34								
0.68286E-03	0.67883E-03	0.78271E-03	0.73522E-03	0.76739E-03	0.80347E-03	0.78820E-03	0.81797E-03	0.85015E-03
0.88516E-03	0.92163E-03							
ROW 35								
0.11162E-02	0.18368E-02	0.96266E-03	0.89481E-03	0.82891E-03	0.12907E-02	0.12881E-02	0.11292E-02	0.10563E-02
0.98829E-03								
ROW 36								
0.99355E-03	0.94784E-03	0.98228E-03	0.85799E-03	0.12038E-02	0.11582E-02	0.11106E-02	0.10620E-02	0.10165E-02
ROW 37								
0.93145E-03	0.91148E-03	0.88956E-03	0.11215E-02	0.11067E-02	0.10901E-02	0.10698E-02	0.10477E-02	
ROW 38								
0.91988E-03	0.92498E-03	0.18458E-02	0.18558E-02	0.18656E-02	0.18754E-02	0.18824E-02		
ROW 39								
0.96371E-03	0.97392E-03	0.18046E-02	0.18388E-02	0.18776E-02	0.11189E-02			
ROW 40								
0.13176E-02	0.14162E-02	0.13247E-02	0.12423E-02	0.11643E-02				
ROW 41								
0.13644E-02	0.13873E-02	0.12584E-02	0.11969E-02					
ROW 42								
0.12835E-02	0.12611E-02	0.12324E-02						
ROW 43								

NOT REPRODUCIBLE

ROW 44 0-12787E-02 0-12741E-02

ROW 45 0-13282E-02

R E B U C E D V P P E R I R I A M P U L A R M E I O H T M A T R I X

ROW 1	0-42674E-03	0-22531E-03	-0-08832E-04	-0-11392E-04	0-32178E-04	0-18499E-03	-0-42997E-04	0-21685E-04
	-0-88666E-05	-0-19222E-04	-0-23520E-05	-0-25422E-05	0-53180E-07	0-83799E-05	0-25211E-04	0-64390E-05
	0-57877E-07	0-45331E-06	-0-25948E-05	-0-87592E-05	-0-13930E-06	-0-13665E-06	0-95964E-06	0-28769E-05
	0-15941E-05	0-21784E-06	0-38647E-07	0-38870E-06	-0-59512E-06	-0-12799E-06	-0-32076E-07	0-98121E-07
	0-26547E-06	0-18943E-06	0-54211E-07	0-95300E-06	-0-65204E-07	-0-42977E-07	-0-15310E-07	-0-33498E-08

ROW 2	0-13172E-02	0-58823E-04	-0-59192E-05	0-31915E-04	0-86641E-05	0-24982E-05	-0-14017E-04	0-18266E-04
	-0-84894E-06	-0-62897E-04	-0-51190E-04	-0-16894E-05	-0-74788E-06	0-15830E-05	0-24458E-04	0-58888E-05
	0-48877E-06	-0-64451E-06	-0-75889E-05	-0-85717E-05	-0-38238E-06	-0-38832E-06	0-21324E-05	0-26826E-05
	0-13832E-05	0-25823E-06	-0-85836E-07	0-68235E-06	-0-81242E-06	-0-49493E-06	0-24359E-07	0-15941E-06
	0-22882E-06	0-15831E-06	0-57544E-07	-0-23989E-07	-0-36191E-07	-0-48189E-07	-0-16813E-07	

ROW 3	0-13895E-02	0-52486E-04	-0-78875E-04	-0-16781E-04	-0-44856E-04	0-24359E-04	-0-42743E-04	0-98728E-06
	-0-57516E-05	-0-57274E-04	-0-69882E-04	-0-48788E-05	-0-47727E-06	0-20889E-05	0-21181E-04	0-29188E-05
	0-13895E-06	-0-89597E-04	-0-63272E-05	-0-63488E-05	-0-38816E-05	-0-34889E-07	0-24632E-06	0-17367E-05
	0-14188E-05	0-18298E-07	-0-71950E-07	-0-47879E-06	-0-77281E-06	-0-54330E-06	-0-38232E-08	0-18231E-06
	0-28258E-06	0-17865E-06	0-55859E-09	-0-34371E-08	-0-24781E-07	-0-41914E-07	-0-42411E-07	

ROW 4	0-13359E-02	0-21947E-03	0-66937E-05	0-98966E-05	-0-42888E-04	0-24238E-03	0-95317E-04	0-15786E-06
	-0-33898E-05	-0-68475E-04	-0-53882E-04	0-87878E-07	0-21838E-06	0-21583E-05	0-24832E-04	0-25836E-04
	0-13724E-06	-0-14281E-05	-0-88186E-05	-0-18534E-04	0-18655E-07	-0-27495E-07	0-48229E-06	0-17188E-05
	-0-68283E-08	0-54188E-08	-0-14572E-06	-0-14173E-06	-0-19184E-05	0-22451E-08	-0-34342E-08	0-43758E-07
	0-28888E-04	-0-41344E-09	0-27777E-09	-0-86644E-08	-0-12642E-07	-0-61523E-07		0-89321E-07

ROW 5	0-35888E-03	-0-42519E-05	0-18113E-04	-0-26345E-04	0-31874E-04	0-54589E-04	0-19268E-06	-0-11689E-05
	-0-83151E-05	-0-27452E-04	-0-21594E-07	0-29244E-08	-0-28473E-06	0-13456E-05	0-98441E-05	-0-23441E-07
	-0-22655E-06	0-75144E-07	-0-34131E-05	0-61861E-09	-0-29699E-07	0-74182E-07	-0-76113E-07	0-93444E-06
	0-73243E-08	-0-27457E-07	0-38872E-07	-0-25888E-06	0-38229E-09	-0-26224E-08	0-73580E-08	-0-12829E-07
	-0-18335E-09	0-34557E-09	-0-12833E-08	0-26389E-08	-0-13392E-07			0-65798E-07

ROW 6	0-43884E-03	0-23688E-03	-0-87688E-04	0-58858E-04	-0-14336E-04	0-31334E-04	0-98929E-04	-0-48568E-04
	-0-83183E-05	-0-23446E-04	-0-67156E-04	0-88488E-06	-0-22638E-05	-0-26375E-06	0-82874E-05	0-21397E-04
	-0-58184E-06	0-51257E-06	-0-26978E-05	-0-76272E-05	-0-33288E-05	-0-32824E-07	-0-87718E-07	0-86923E-08
	0-13724E-05	0-14887E-06	0-22891E-07	-0-26889E-06	-0-76787E-06	-0-47248E-06	-0-87734E-07	-0-82854E-08
	0-28444E-06	0-11371E-06	0-31246E-07	0-38323E-08				0-69357E-07

ROW 7	0-41142E-02	0-52578E-04	-0-38856E-05	0-36259E-04	0-78885E-05	0-22896E-03	0-22331E-04	0-16879E-04
	-0-39925E-06	-0-59846E-04	-0-51512E-04	-0-23288E-05	-0-19672E-06	0-69746E-06	0-28545E-04	0-19684E-04
	0-82898E-07	-0-43761E-06	-0-57383E-05	-0-68889E-05	-0-24641E-05	-0-28162E-06	0-14111E-06	0-16485E-05
	0-38478E-05	0-18288E-06	-0-18288E-06	-0-43874E-06	-0-68329E-06	-0-35968E-06	-0-93873E-07	0-11124E-07
	0-13677E-06	0-89833E-07	0-38715E-07					0-97987E-07

ROW 8	0-32841E-02	0-53518E-04	-0-88888E-04	-0-16888E-04	-0-44564E-04	0-22125E-03	0-22246E-04	-0-29588E-04
	-0-88888E-05	-0-55858E-04	-0-58117E-04	-0-45988E-05	-0-35889E-06	0-14385E-05	0-18495E-04	0-20259E-04
	0-14118E-06	-0-65858E-06	-0-58287E-06	-0-88888E-05	-0-25254E-05	-0-26895E-07	0-18412E-06	0-14288E-05
	0-13357E-05	0-12881E-07	-0-44878E-07	-0-36898E-06	-0-58288E-06	-0-48592E-06	-0-19118E-08	0-11432E-07

NCT REPRODUCIBLE

ROW 9

0.14134E-02	0.24690E-03	0.67114E-05	0.85358E-05	-0.42959E-04	0.21874E-03	0.70805E-04	0.21297E-06	0.21754E-03
-0.39298E-05	-0.67958E-04	-0.51714E-04	0.95728E-07	-0.36462E-06	0.28349E-05	0.28314E-04	0.24323E-04	-0.48422E-07
0.29179E-06	-0.93239E-06	-0.53826E-05	-0.427504E-05	0.38223E-06	-0.40238E-07	0.38836E-06	0.14962E-05	0.26807E-03
0.52224E-06	0.17467E-17	-0.99280E-17	-0.30457E-06	-0.79128E-06	0.52211E-09	-0.20034E-08	0.20596E-07	0.66397E-07
0.17797E-06								

ROW 10

0.48956E-03	-0.43598E-05	0.18280E-04	0.25631E-04	0.37002E-04	0.10352E-06	-0.111167E-65	0.39641E-85	
-0.61482E-05	-0.24123E-04	0.86784E-07	-0.12885E-06	-0.10288E-06	0.28014E-06	0.88826E-05	0.12054E-07	0.25849E-07
-0.23426E-07	0.18464E-06	-0.25811E-05	0.70188E-08	-0.18310E-07	0.24837E-07	-0.88592E-07	0.73225E-06	-0.27626E-09
0.57367E-08	-0.13361E-07	0.35997E-07	-0.19203E-06	0.30846E-09	-0.32517E-09	0.17828E-08	-0.67128E-08	0.41384E-07

ROW 11

0.47815E-03	0.23446E-03	-0.88802E-04	0.35848E-04	-0.14242E-04	0.49902E-04	0.11091E-03	-0.42662E-04	0.21787E-04
-0.83494E-05	-0.12427E-04	0.86784E-07	-0.12885E-06	-0.10288E-06	0.28014E-06	0.88826E-05	0.12054E-07	0.25849E-07
-0.58280E-06	0.28082E-06	-0.22635E-05	-0.427504E-05	0.38223E-06	-0.40238E-07	0.38836E-06	0.14962E-05	0.26807E-03
0.18028E-05	0.77838E-07	-0.11761E-07	-0.27522E-06	-0.62283E-06	-0.27222E-06	-0.53285E-07	0.15272E-09	

ROW 12

0.14186E-02	0.25554E-04	-0.48780E-04	0.36499E-04	0.19171E-04	0.24863E-03	0.89566E-04	-0.17369E-04	0.17207E-04
0.16943E-06	-0.48938E-04	-0.43161E-04	0.86431E-06	-0.14698E-05	0.35243E-06	0.18787E-06	0.14857E-04	0.27842E-05
0.13553E-06	-0.48788E-06	-0.49926E-06	-0.53794E-05	-0.17984E-05	-0.20581E-06	0.88448E-07	0.13879E-05	0.16593E-05
0.73728E-06	0.13959E-06	-0.33128E-07	-0.34192E-06	-0.40489E-06	-0.21859E-06	-0.55185E-07		

ROW 13

0.13956E-02	0.52224E-04	-0.88802E-04	0.35848E-04	-0.14242E-04	0.49902E-04	0.11091E-03	-0.42662E-04	0.21787E-04
-0.38748E-05	-0.49881E-04	-0.43161E-04	0.86431E-06	-0.14698E-05	0.35243E-06	0.18787E-06	0.14857E-04	0.27842E-05
0.38881E-06	-0.29273E-06	-0.42128E-05	-0.56237E-05	-0.16281E-05	-0.46808E-07	0.25994E-07	0.15218E-05	0.16593E-05
0.77241E-06	0.16672E-07	-0.13334E-07	-0.26559E-06	-0.41319E-06	-0.23311E-06			

ROW 14

0.14125E-02	0.24870E-03	0.63215E-05	0.86720E-05	-0.40287E-04	0.24134E-03	0.88765E-04	-0.13171E-06	0.29566E-05
-0.24592E-05	-0.54863E-04	-0.49617E-04	0.14799E-06	0.18378E-06	0.31217E-06	0.16047E-04	0.28882E-04	-0.18169E-06
0.15358E-06	-0.61135E-06	-0.45138E-05	-0.71677E-05	0.99322E-08	-0.31274E-07	0.28519E-06	0.18307E-05	0.22198E-05
-0.54829E-08	-0.38837E-08	-0.46889E-07	-0.26357E-06	-0.53788E-06				

ROW 15

0.41440E-03	-0.42558E-05	0.98981E-05	-0.26366E-04	0.25163E-04	0.48458E-04	-0.41558E-07	-0.38875E-06	0.28815E-05
-0.33811E-05	-0.22385E-04	-0.73461E-07	-0.15817E-07	-0.48885E-06	0.23258E-06	0.68183E-05	0.44648E-07	0.22851E-08
0.92489E-07	0.82263E-07	-0.21878E-05	0.25443E-08	-0.19771E-07	0.11254E-07	-0.28232E-07	0.58235E-06	0.28942E-08
0.38889E-08	0.16243E-08	0.13984E-07	-0.83618E-06					

ROW 16

0.38418E-03	0.28592E-03	-0.75278E-04	0.31888E-04	-0.12842E-04	-0.26977E-05	0.88974E-04	-0.33891E-04	0.15421E-04
-0.56418E-05	-0.86336E-05	-0.46841E-04	0.44837E-05	-0.22635E-05	0.47887E-06	0.44468E-05	0.14986E-04	0.35366E-05
-0.81631E-08	0.35424E-06	-0.17482E-05	-0.27281E-05	-0.21828E-05	0.12889E-06	-0.21512E-07	0.76817E-06	0.21845E-05
0.68870E-06	0.18255E-06	-0.16882E-07						

ROW 17

0.13258E-02	0.58867E-04	-0.98887E-06	0.31692E-04	0.43865E-05	0.13138E-03	-0.39588E-05	-0.13953E-04	0.18281E-04
0.71858E-05	-0.14272E-04	-0.38578E-04	-0.79688E-06	0.42895E-06	-0.89639E-06	0.11785E-04	0.18828E-04	0.25974E-05
-0.46458E-06	0.28258E-06	-0.28682E-05	-0.37178E-05	-0.14699E-05	-0.38886E-07	-0.76549E-06	0.92434E-06	0.12817E-05
0.53792E-06	0.69638E-07							

ROW 18

0.12959E-02	0.25412E-04	-0.77898E-04	-0.15345E-04	-0.54588E-04	0.13656E-03	-0.29272E-05	-0.28798E-04	-0.18328E-05
-0.86458E-05	-0.17878E-04	-0.28328E-04	-0.26948E-05	-0.29638E-06	-0.12178E-05	0.11631E-04	0.18947E-04	0.38843E-05
0.11284E-06	-0.39621E-06	-0.38838E-05	-0.39382E-05	-0.14815E-05	-0.75414E-08	0.12583E-06	0.92476E-06	0.13853E-05
0.58124E-06								

ROW 20	0.41879E-03	-0.55991E-05	0.13278E-04	-0.30279E-04	0.23728E-04	-0.70775E-05	-0.03747E-06	0.37029E-06	0.13225E-05
	-0.20135E-05	-0.10432E-04	-0.18134E-06	0.57132E-07	-0.56017E-06	0.34340E-06	0.34980E-05	0.51975E-08	-0.14200E-07
	0.72009E-07	0.17139E-06	-0.13340E-05	-0.11071E-08	0.27410E-08	-0.27900E-07	0.31990E-07	0.44039E-06	
ROW 21	0.37530E-03	0.19010E-03	-0.67950E-04	0.28454E-04	-0.11042E-04	0.30821E-02	0.04799E-04	-0.37031E-04	0.17200E-04
	-0.67745E-05	-0.10206E-04	-0.51176E-04	0.41977E-05	-0.27096E-05	0.20394E-06	0.01000E-05	0.15079E-04	0.32031E-05
	-0.71109E-06	0.01200E-06	-0.25848E-05	-0.07167E-05	-0.11224E-05	-0.13925E-04	0.22061E-07		
ROW 22	0.11340E-02	0.54017E-04	-0.12027E-05	0.27270E-04	0.75739E-05	0.17401E-03	0.14007E-04	-0.13113E-04	0.11230E-04
	0.67000E-05	-0.26010E-04	-0.32007E-04	0.12041E-06	0.30604E-06	-0.76254E-04	0.13635E-04	0.13294E-04	0.23263E-05
	-0.55175E-06	0.76490E-07	-0.37200E-05	-0.01970E-05	-0.13140E-05	-0.93010E-07			
ROW 23	0.11323E-02	0.47261E-04	-0.63570E-04	-0.16430E-04	-0.26000E-04	0.18422E-03	0.12000E-04	-0.21744E-04	-0.20940E-05
	-0.09030E-05	-0.26075E-04	-0.37400E-04	-0.31010E-05	-0.54734E-06	-0.15999E-05	0.13250E-04	0.12235E-04	0.31292E-05
	-0.20990E-07	-0.72746E-06	-0.36490E-05	-0.44774E-05	-0.13050E-05				
ROW 24	0.11490E-02	0.19466E-03	0.02519E-05	0.16060E-04	-0.50619E-04	0.17902E-03	0.55406E-04	0.20024E-05	0.57030E-05
	-0.53725E-05	-0.46754E-04	-0.55600E-04	0.29432E-06	0.62902E-06	-0.23027E-04	0.12640E-04	0.15262E-04	0.44975E-07
	0.14692E-06	-0.30149E-06	-0.44014E-05	-0.49493E-05					
ROW 25	0.35309E-03	-0.57000E-05	0.12011E-04	-0.20000E-04	0.26254E-04	0.21033E-04	-0.11065E-05	-0.02190E-06	-0.19096E-07
	-0.15770E-05	-0.10330E-04	-0.15610E-06	-0.71521E-07	-0.10982E-06	-0.24412E-06	0.55400E-05	-0.040310E-08	-0.10141E-06
	0.21531E-06	-0.20029E-06	-0.10511E-05						
ROW 26	0.37105E-03	0.10006E-03	-0.00270E-04	0.20760E-04	-0.11025E-04	0.25966E-05	0.03999E-04	-0.37550E-04	0.16950E-04
	-0.63190E-05	-0.79201E-05	-0.40330E-04	0.02642E-05	-0.23739E-05	0.32104E-06	0.70149E-05	0.18223E-04	0.10792E-06
	0.65290E-06	0.94997E-07							
ROW 27	0.11204E-02	0.40011E-04	-0.20012E-06	0.27173E-04	0.05072E-05	0.17042E-04	0.14019E-04	-0.12909E-04	0.11360E-04
	0.76900E-05	-0.22764E-04	-0.30004E-04	0.14911E-05	0.24090E-06	-0.10773E-06	0.15100E-04	0.11202E-04	0.17200E-05
ROW 28	0.11293E-02	0.41429E-04	-0.63339E-04	-0.17062E-04	-0.59030E-04	0.10712E-03	0.11920E-04	-0.22230E-04	-0.26500E-05
	-0.77655E-05	-0.23575E-04	-0.33399E-04	-0.16566E-05	-0.13336E-06	-0.46000E-07	0.14702E-04	0.12022E-04	0.25240E-05
ROW 29	0.11377E-02	0.10932E-03	0.09315E-05	0.15659E-04	-0.50254E-04	0.10150E-03	0.50036E-04	0.19400E-05	0.02005E-05
	-0.43230E-05	-0.51000E-04	-0.31047E-04	0.41590E-07	0.10056E-05	-0.56742E-06	0.10550E-04	0.12066E-04	
ROW 30	0.30313E-03	-0.59170E-05	0.10000E-04	-0.20027E-04	0.26056E-04	0.24007E-04	-0.12457E-05	0.10733E-05	-0.45314E-06
	0.12703E-06	-0.17226E-04	-0.14051E-06	0.05431E-07	-0.57764E-06	0.13276E-05	0.59915E-05		
ROW 31	0.37543E-03	0.19092E-03	-0.66000E-04	0.20220E-04	-0.11014E-04	-0.37049E-05	0.70314E-04	-0.42023E-04	0.17006E-04
	-0.65537E-05	-0.19650E-04	-0.56517E-04	0.11374E-04	-0.11374E-04	-0.52279E-05	0.11000E-05		
ROW 32	0.11233E-02	0.51000E-04	0.30063E-06	0.27000E-04	0.33910E-05	0.16633E-03	0.59456E-05	-0.15260E-04	0.12302E-04
	0.33020E-05	-0.35531E-04	-0.30000E-04	0.33000E-04	-0.66187E-07				
ROW 33	0.11262E-02	0.44070E-04	-0.62720E-04	-0.19075E-04	-0.62004E-04	0.17534E-03	0.59915E-06	-0.25004E-04	-0.19756E-05
	-0.21272E-04	-0.33670E-04	-0.37262E-04	-0.23502E-05					

0.277856E-02 0.14777E-03 0.95535E-03 0.15485E-04 -0.63487E-04 0.16349E-03 2.46444E-04 0.12450E-03 8.16970E-02

ROW 35 0.34431E-03 -0.53752E-05 0.12055E-04 -0.26619E-04 0.19498E-04 0.10243E-04 -0.58716E-06 -0.98961E-06 0.35958E-05

ROW 36 0.37798E-03 0.28266E-03 -0.63595E-04 0.26762E-04 -0.12011E-04 0.54121E-04 0.12806E-03 -0.64325E-04 0.32645E-04

ROW 37 0.11658E-02 0.63535E-04 0.42888E-05 0.26868E-04 0.12725E-05 0.21657E-05 0.29224E-04 -0.13342E-04 0.12565E-04

ROW 38 0.11611E-02 0.58084E-04 -0.06278E-04 -0.29401E-05 -0.28376E-04 0.22872E-05 0.23342E-04 -0.16692E-04

ROW 39 0.11925E-02 0.21602E-03 0.13571E-05 0.17752E-04 -0.35855E-04 0.23341E-05 0.43808E-04

ROW 40 0.36888E-03 -0.16282E-05 0.13818E-04 -0.34365E-04 0.54161E-04 0.42861E-04

ROW 41 0.86339E-04 0.59140E-04 -0.34844E-04 0.14564E-04 -0.06648E-05

ROW 42 0.49425E-03 -0.82125E-04 0.26118E-04 0.11511E-04

ROW 43 0.53042E-03 -0.77828E-04 -0.26293E-04

ROW 44 0.47325E-03 0.57210E-04

ROW 45 0.12485E-03

NOT REPRODUCIBLE

HERE ARE THE EIGENVALUES AND EIGENVECTORS

EIGENVECTOR NUMBER	CORRESPONDING ID	EIGENVALUE	EIGENVECTOR
2.7407843E-02	2.1000233E-07	2.6003531E-07	1.9602752E-02
9.0582941E-02	8.0990002E-02	7.3517372E-02	2.8042070E-01
1.9089256E-01	1.0057407E-01	3.4305413E-01	3.3197199E-01
3.2000731E-01	7.9727014E-01	4.6432702E-01	4.3731815E-01
4.8706739E-01	5.9491701E-01	5.6392713E-01	5.4040159E-01
7.2793315E-01	7.1192702E-01	6.9666764E-01	6.3330005E-01
8.4729429E-01	8.3081412E-01	8.1410763E-01	7.0730037E-01
9.6545278E-01	9.4054101E-01	9.3259025E-01	7.9000000E-00

EIGENVECTOR NUMBER	CORRESPONDING ID	EIGENVALUE	EIGENVECTOR
8.5521500E-02	2.7716654E-02	1.1120014E-02	4.3703035E-02
9.2737017E-02	7.0466004E-02	3.4090670E-02	2.4238271E-02
3.4720018E-02	2.4728302E-01	6.1553020E-01	5.5615291E-01
3.4073756E-01	6.1404724E-01	6.7345074E-01	3.8900466E-01
7.3160031E-01	7.7002096E-01	3.7000765E-01	3.2222200E-02
8.0007576E-01	4.2622012E-01	1.7400405E-02	4.5510627E-01
4.2034300E-01	6.4490500E-03	4.5913063E-01	9.1152995E-01
3.2745666E-02	4.5324094E-01	9.2303551E-01	1.0000000E-00

EIGENVECTOR NUMBER	CORRESPONDING ID	EIGENVALUE	EIGENVECTOR
-1.7472901E-01	1.3291904E-01	1.1394600E-01	9.9536630E-02
-4.1720210E-01	3.4911200E-01	3.3101010E-01	2.0972693E-01
-6.0010701E-01	9.5300007E-01	5.1737110E-01	8.2059021E-01
-8.2053332E-01	5.9964417E-01	6.9300430E-01	6.3357042E-01
-5.0000000E-01	4.0529155E-01	3.6210532E-01	3.3827405E-01
-6.6905213E-01	2.6633730E-02	7.7129130E-02	8.2347580E-02
4.0330429E-01	5.0031020E-01	5.2125940E-01	5.3107960E-01
9.7754035E-01	9.9049134E-01	1.0000000E-00	8.4004007E-01

EIGENVECTOR NUMBER	CORRESPONDING ID	EIGENVALUE	EIGENVECTOR
-1.0032000E-01	5.4622611E-02	4.6235010E-02	1.0004000E-01
-1.7357266E-01	1.0157030E-01	3.5000670E-01	5.0200112E-01
9.6596537E-02	4.6010670E-01	7.7424700E-01	5.6752232E-01
3.7150117E-01	6.0224004E-01	9.9364000E-01	7.9537030E-01
4.0003677E-01	6.7553504E-02	9.9757202E-02	7.9257745E-02
3.2000568E-01	1.1050100E-01	7.3000700E-02	2.2000297E-01
3.4000550E-01	1.9957320E-02	3.4300920E-01	5.9400000E-00
6.0700000E-02	4.0634705E-01	7.6601401E-01	1.0000000E-00

EIGENVECTOR NUMBER	CORRESPONDING ID	EIGENVALUE	EIGENVECTOR
4.5020030E-01	3.1571991E-01	7.4041070E-01	1.9732440E-01
7.5200270E-01	6.0713240E-01	4.0753390E-01	3.6472696E-01
5.6379330E-01	4.3000010E-01	3.3485021E-01	1.0734300E-01
-7.1702513E-03	0.5000123E-02	4.7005001E-01	3.0204990E-01
-0.0100243E-01	7.2307370E-01	6.1940000E-01	4.5535474E-01
-5.1330479E-01	4.2600670E-01	3.4152900E-01	3.3033447E-01
1.2400010E-01	2.1300700E-01	2.6302750E-01	7.0071657E-01
9.1727512E-01	9.0300147E-01	1.0000000E-00	7.5522000E-01

EIGENVECTOR NUMBER	CORRESPONDING ID	EIGENVALUE	EIGENVECTOR
2.1000000E-01	1.0720000E-01	1.0000000E-00	1.0000000E-00

NOT REPRODUCIBLE

1.8374410E-01 -1.0971055E-01 -3.2108024E-01 -4.3013440E-01 3.7881074E-01 1.4978124E-01
 7.8648933E-02 -2.1143981E-01 -2.5773532E-01 2.8035019E-02 -4.3289540E-02 -4.4758783E-02
 7.8863181E-02 2.4596607E-01 -2.875452E-01 -2.1214830E-01 -6.8787703E-02 2.3883812E-01
 5.8485234E-01 -1.8234120E-01 -2.6889999E-01 -1.3464532E-01 2.2392664E-01 6.5758324E-01
 1.2955652E-01 -1.5326722E-01 -1.9555979E-01 5.8782436E-02 4.4786194E-01 5.8369401E-01
 8.6926320E-02 -2.1198833E-01 -1.7798899E-01 9.1759127E-02 1.8088800E-01 3.4918185E-01
 -1.8627699E-01 -3.7443474E-01 -2.4227899E-01

EIGENVECTOR NUMBER 7
 CORRESPONDING TO 1.7581530E 10
 -7.980879E-04 -4.5298745E-02 -1.1112433E-02 9.4331717E-02 2.8841379E-01 6.4284345E-02
 -1.836188E-01 -4.8228813E-02 1.7856599E-01 5.1883698E-01 2.7178799E-01 -1.1319853E-01
 -2.1433281E-01 7.4782497E-02 5.5755368E-01 5.7348539E-01 -4.7469770E-02 -3.4759428E-01
 1.3489836E-01 3.8458168E-01 1.7277736E-01 2.5584858E-02 -3.9868222E-01 -2.3877884E-01
 2.953780E-01 8.559599E-01 5.773886E-02 -3.7748155E-01 -1.9577987E-01 3.7627285E-01
 7.293218E-01 8.8137875E-04 -3.5768368E-01 -7.8888485E-02 5.9888255E-01 5.1555558E-01
 -1.4274691E-01 -3.774431E-01 5.1987770E-02 8.4306784E-01 7.7894770E-01 -8.7717438E-01
 -4.4511191E-01 1.8888881E-01 1.8888881E-01

EIGENVECTOR NUMBER 8
 CORRESPONDING TO 2.6717637E 10
 -6.642289E-01 -5.1843482E-01 -4.256898E-01 -3.8272487E-01 -1.4568813E-01 -7.1545954E-01
 -7.768924E-01 -6.788131E-01 -4.6143620E-01 -2.1863632E-01 3.4938472E-01 3.8827935E-03
 -6.1528161E-02 5.3331285E-02 2.6196635E-01 1.8888888E-08 8.422218E-01 4.8331724E-01
 4.9664120E-01 6.4961593E-01 5.7819731E-01 4.2422572E-01 2.6289809E-01 7.3829619E-01
 3.381129E-01 -3.7424884E-01 -2.9432837E-01 -3.2613491E-01 -3.4251430E-01 -3.8312624E-01
 -7.863288E-01 -6.111258E-01 -5.4779873E-01 -5.5821771E-01 -5.8189580E-01 -1.4368387E-01
 -1.8861844E-01 -2.5886722E-02 -1.2723870E-02 -9.9958384E-02 8.5458917E-01 8.7198748E-01
 9.3986379E-01 9.5329414E-01 8.8888888E-01

EIGENVECTOR NUMBER 9
 CORRESPONDING TO 4.1568824E 10
 -4.3837638E-01 -6.6936688E-03 1.6848881E-01 2.2613313E-02 -2.4785980E-01 -8.3947799E-01
 -6.884887E-01 3.9885455E-01 8.787394E-02 -6.1876983E-01 8.262834E-01 8.8238925E-02
 5.3185887E-01 1.8851482E-01 -7.8884996E-01 -5.8873842E-01 1.4678887E-01 4.8888881E-01
 2.7281818E-02 8.8821923E-01 -1.2712886E-02 1.8884444E-01 2.3888318E-01 7.8878392E-03
 -4.3976157E-01 9.877975E-02 -1.8825669E-02 5.9391835E-02 -5.848818E-02 -6.7915445E-02
 4.8838589E-01 -1.1888259E-01 -3.1422586E-01 -6.7874988E-02 3.7188735E-01 7.8895818E-01
 -1.1826194E-01 -4.8883951E-01 -2.758387E-02 7.6248514E-01 9.138382E-01 -7.7588135E-02
 -5.8794386E-01 1.6225758E-02 1.8888888E-08

NOT REPRODUCIBLE

1022

HERE ARE THE NATURAL FREQUENCIES

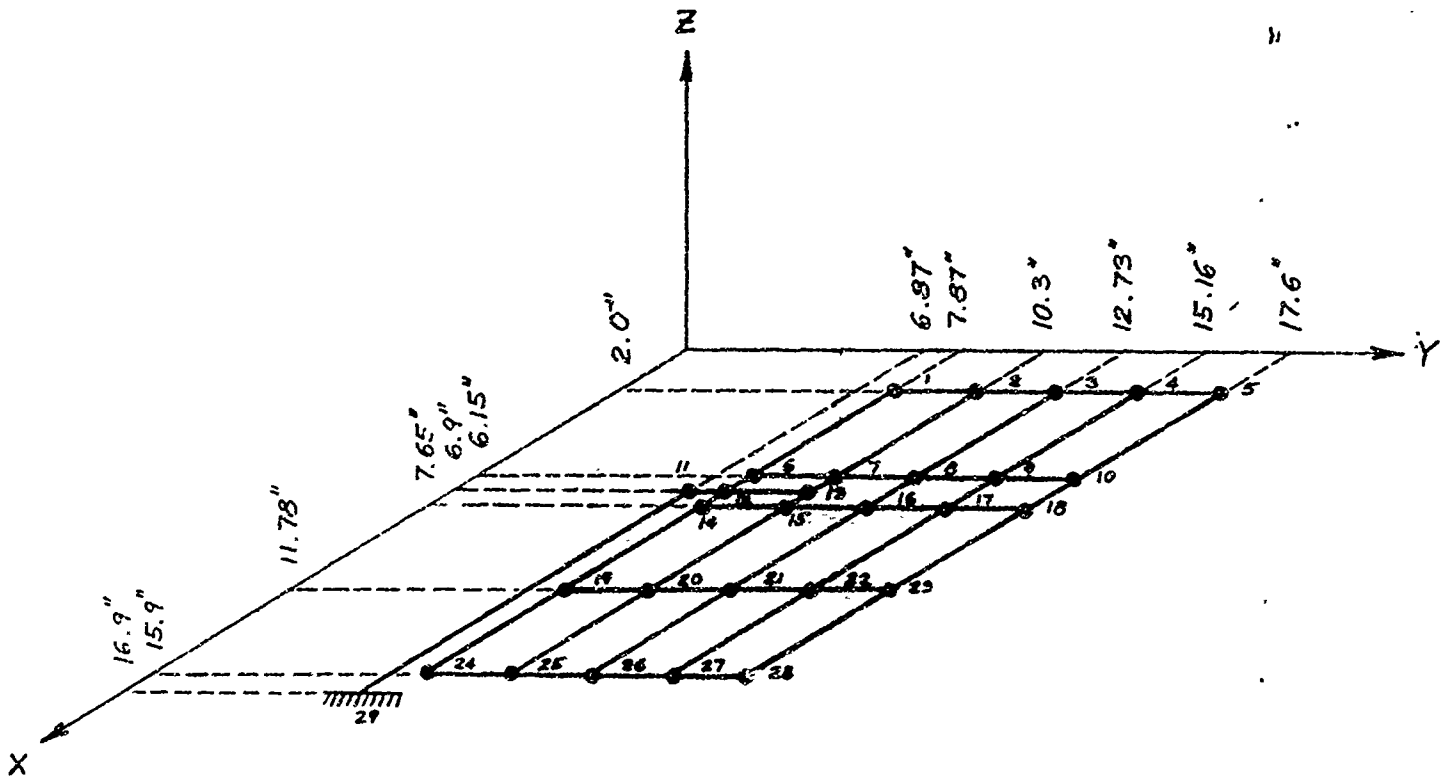
THE NATURAL FREQUENCY NUMBER	1	IS	764.618	CPS
THE NATURAL FREQUENCY NUMBER	2	IS	3438.197	CPS
THE NATURAL FREQUENCY NUMBER	3	IS	4698.889	CPS
THE NATURAL FREQUENCY NUMBER	4	IS	11001.007	CPS
THE NATURAL FREQUENCY NUMBER	5	IS	13177.622	CPS
THE NATURAL FREQUENCY NUMBER	6	IS	20579.985	CPS
THE NATURAL FREQUENCY NUMBER	7	IS	21183.287	CPS
THE NATURAL FREQUENCY NUMBER	8	IS	20814.720	CPS
THE NATURAL FREQUENCY NUMBER	9	IS	20274.123	CPS

EXTRACTED FROM

Sample Problem No. 3

Missile Control Surface Model
(Modeled with beam elements and lumped weights)

Find first five natural modes and frequencies.



Note: Joint 11 is restrained from rotating about y

Lumped Masses

Joint No.	Mass lb.
1	0.050
2	0.110
3	0.115
4	0.125
5	0.196
6	0.155
7	0.305
8	0.305
9	0.305
10	0.165
11	0.060
12	0.165
13	0.005
14	0.183
15	0.325
16	0.310
17	0.280
18	0.140
19	0.062
20	0.078
21	0.078
22	0.078
23	0.080
24	0.033
25	0.051
26	0.051
27	0.051
28	0.042
29	0.050

Beam Element Properties

Member i - j	Moment-of-Inertia Area	Torsional Constant
	inch ⁴	
1-2	0.0009	0.0055
2-3	0.0009	0.0055
3-4	0.0009	0.0055
4-5	0.0018	0.0055
6-7	0.0164	0.0300
7-8	0.0164	0.0300
8-9	0.0164	0.0300
9-12	0.0164	0.0300
12-13	0.0160	0.0300
14-15	0.0147	0.0280
15-16	0.0147	0.0280
16-17	0.0147	0.0280
17-18	0.0147	0.0280
19-20	0.0053	0.0010
20-21	0.0053	0.0010
21-22	0.0053	0.0010
22-23	0.0053	0.0010
24-25	0.0031	0.0006
25-26	0.0031	0.0006
26-27	0.0031	0.0006
27-28	0.0031	0.0006
1-6	0.0013	0.0026
2-7	0.0027	0.0054
3-8	0.0027	0.0054
4-9	0.0027	0.0054
5-10	0.0026	0.0029
6-12	0.0503	0.1000
12-14	0.0503	0.1000
7-13	0.0255	0.0510
13-15	0.0255	0.0510
8-16	0.0380	0.0750
9-17	0.0380	0.0750
10-18	0.0377	0.0750
14-19	0.0017	0.0034
15-20	0.0035	0.0070
16-21	0.0035	0.0070
17-22	0.0035	0.0070
18-23	0.0017	0.0029
11-12	100.0000	0.0100
11-29	0.3200	0.0790
19-24	0.0017	0.0030
20-25	0.0017	0.0030
21-26	0.0035	0.0070
22-27	0.0035	0.0070
23-28	0.0017	0.0029

$E = 3 \times 10^7 \text{ psi}$

$\nu = 0.3$

Listing of Input Data Cards

MISSILE CONTROL SURFACE MODEL WITH 29 JOINTS
AUGUST 1968

29	2	45	0	9	1	29
1						
30.	0.5		0.		0.	
1	2.0		7.87			
2	2.0		10.5			
3	2.0		12.73			
4	2.0		15.16			
5	2.0		17.0			
6	6.15		7.87			
7	6.15		10.5			
8	6.15		12.73			
9	6.15		15.16			
10	6.15		17.0			
11	6.9		6.87			
12	6.9		7.87			
13	6.9		10.5			
14	7.65		7.87			
15	7.65		10.5			
16	7.65		12.73			
17	7.65		15.16			
18	7.65		17.0			
19	11.78		7.87			
20	11.78		10.5			
21	11.78		12.73			
22	11.78		15.16			
23	11.78		17.0			
24	15.9		7.87			
25	15.9		10.5			
26	15.9		12.73			
27	15.9		15.16			
28	15.9		17.0			
29	16.9		6.87			
11	0	0	1			
20	1	1	1			
1	0.05					
2	0.11					
3	0.115					
4	0.125					
5	0.196					
6	0.155					
7	0.305					
8	0.305					
9	0.305					
10	0.165					
11	0.06					
12	0.165					
13	0.005					
14	0.183					
15	0.325					
16	0.31					
17	0.28					

18	0.14				
19	0.062				
20	0.078				
21	0.078				
22	0.078				
23	0.08				
24	0.033				
25	0.051				
26	0.051				
27	0.051				
28	0.042				
29	0.050				
0.	0.0009	0.0055	1	1	2
0.	0.0009	0.0055	1	2	3
0.	0.0009	0.0055	1	3	4
0.	0.0018	0.0055	1	4	5
0.	0.0164	0.03	1	5	7
0.	0.0164	0.03	1	7	8
0.	0.0164	0.03	1	8	9
0.	0.0164	0.03	1	9	10
0.	0.016	0.03	1	12	13
0.	0.0147	0.028	1	14	15
0.	0.0147	0.028	1	15	16
0.	0.0147	0.028	1	16	17
0.	0.0147	0.028	1	17	18
0.	0.0053	0.001	1	19	20
0.	0.0053	0.001	1	20	21
0.	0.0053	0.001	1	21	22
0.	0.0053	0.001	1	22	23
0.	0.0031	0.006	1	24	25
0.	0.0031	0.006	1	25	26
0.	0.0031	0.006	1	26	27
0.	0.0031	0.006	1	27	28
0.	0.0013	0.0026	1	1	6
0.	0.0027	0.0054	1	2	7
0.	0.0027	0.0054	1	3	8
0.	0.0027	0.0054	1	4	9
0.	0.0026	0.0029	1	5	10
0.	0.0503	0.1	1	6	12
0.	0.0503	0.1	1	12	14
0.	0.0255	0.051	1	7	13
0.	0.0255	0.051	1	13	15
0.	0.038	0.075	1	8	16
0.	0.038	0.075	1	9	17
0.	0.0377	0.075	1	10	18
0.	0.0017	0.0034	1	14	19
0.	0.0035	0.007	1	15	20
0.	0.0035	0.007	1	16	21
0.	0.0035	0.007	1	17	22
0.	0.0017	0.0029	1	18	23
0.	100.	0.01	1	11	12
0.	0.32	0.079	1	11	29

0.	0.0017	0.003	1	19	24
0.	0.0017	0.003	1	20	25
0.	0.0035	0.007	1	21	26
0.	0.0035	0.007	1	22	27
0.	0.0017	0.0029	1	23	28

Program Output

RECEIVED THE STATE

MISSILE CONTROL SURFACE MODEL WITH 29 JOINTS
AUGUST 1968

MJIS = 20 MR = 2 MRE = 45 MPE = 0 MPODE = 9 MREY = 1 MUMP = 29

MATERIAL PROPERTIES
NO. YOUNG'S MODULUS POISSON RATIO MODULUS OF RIGIDITY DENSITY
1 9.3000E 08 0.30000 0.11530E 08 8.

JOINT C O R D I N A T E S

JOINT NO.	X COORD.	Y COORD.
1	2.00000	7.07000
2	2.00000	10.30000
3	2.00000	12.71000
4	2.00000	15.16000
5	2.00000	17.60000
6	6.15000	7.07000
7	6.15000	10.30000
8	6.15000	12.71000
9	6.15000	15.16000
10	6.15000	17.60000
11	6.90000	6.07000
12	6.90000	7.07000
13	6.90000	10.30000
14	7.65000	7.07000
15	7.65000	10.30000
16	7.65000	12.71000
17	7.65000	15.16000
18	7.65000	17.60000
19	11.70000	7.07000
20	11.70000	10.30000
21	11.70000	12.71000
22	11.70000	15.16000
23	11.70000	17.60000
24	15.90000	7.07000
25	15.90000	10.30000
26	15.90000	12.71000
27	15.90000	15.16000
28	15.90000	17.60000
29	16.90000	6.07000

NOT REPRODUCIBLE

JOINT RES I R A I N T C O D E

JOINT NO.	Z DISPLACEMENT	ROTATION ABOUT X	ROTATION ABOUT Y
1	1	1	1
2	1	1	1
3	1	1	1
4	1	1	1
5	1	1	1
6	1	1	1
7	1	1	1
8	1	1	1
9	1	1	1
10	1	1	1
11	1	1	1
12	1	1	1
13	1	1	1
14	1	1	1
15	1	1	1
16	1	1	1
17	1	1	1
18	1	1	1
19	1	1	1
20	1	1	1
21	1	1	1
22	1	1	1
23	1	1	1
24	1	1	1
25	1	1	1
26	1	1	1
27	1	1	1
28	1	1	1
29	1	1	1

COORDINATE NUMBERS FOR EACH Z DISPLACEMENT AT EACH UNRESTRAINED JOINT

JOINT NO.	COORD. NO.
1	1
2	2
3	3
4	4
5	5
6	6
7	7
8	8
9	9
10	10
11	11
12	12
13	13
14	14
15	15
16	16
17	17
18	18
19	19
20	20
21	21
22	22
23	23
24	24
25	25
26	26
27	27
28	28
29	29

5	6	7	8	9	10	11	12	13	14	15	16	17	18	19	20	21	22	23	24	25	26	27	28	

ALUMPF B WEIGHTS

JOINT NO.	WEIGHT
1	0.0588
2	0.1100
3	0.1150
4	0.1250
5	0.1960
6	0.1550
7	0.3820
8	0.3850
9	0.1650
10	0.8600
11	0.1650
12	0.0820
13	0.1830
14	0.3720
15	0.3100
16	0.2000
17	0.1400
18	0.0620
19	0.0700
20	0.0700
21	0.0700
22	0.0700
23	0.0700
24	0.0700
25	0.0700
26	0.0700
27	0.0700
28	0.0700

NOT REPRODUCIBLE

BEAR ELEMENT PROPERTIES

ELEMENT NO.	A	J	MAY	JOINT 1	JOINT 2
1	0.0000	0.0055	1	1	2
2	0.0000	0.0055	1	2	3
3	0.0000	0.0055	1	3	4

NOT REPRODUCIBLE

NOT REPRODUCIBLE

4	0.0010	0.0055	1	4	5
5	0.0164	0.0308	1	6	7
6	0.0364	0.0708	1	7	8
7	0.08164	0.1508	1	8	9
8	0.1632	0.3016	1	9	10
9	0.3264	0.6032	1	10	11
10	0.6528	1.2064	1	11	12
11	1.3056	2.4128	1	12	13
12	2.6112	4.8256	1	13	14
13	5.2224	9.6512	1	14	15
14	10.4448	19.3024	1	15	16
15	20.8896	38.6048	1	16	17
16	41.7792	77.2096	1	17	18
17	83.5584	154.4192	1	18	19
18	167.1168	308.8384	1	19	20
19	334.2336	617.6768	1	20	21
20	668.4672	1235.3536	1	21	22
21	1336.9344	2470.7072	1	22	23
22	2673.8688	4941.4144	1	23	24
23	5347.7376	9882.8288	1	24	25
24	10695.4752	19765.6576	1	25	26
25	21390.9504	39531.3152	1	26	27
26	42781.9008	79062.6304	1	27	28
27	85563.8016	158125.2608	1	28	29
28	171127.6032	316250.5216	1	29	30
29	342255.2064	632501.0432	1	30	31
30	684510.4128	1265002.0864	1	31	32
31	1369020.8256	2530004.1728	1	32	33
32	2738041.6512	5060008.3456	1	33	34
33	5476083.3024	10120016.6912	1	34	35
34	10952166.6048	20240033.3824	1	35	36
35	21904333.2096	40480066.7648	1	36	37
36	43808666.4192	80960133.5296	1	37	38
37	87617332.8384	161920267.0592	1	38	39
38	175234665.6768	323840534.1184	1	39	40
39	350469331.3536	647681068.2368	1	40	41
40	700938662.7072	1295362136.4736	1	41	42
41	1401877325.4144	2590724272.9472	1	42	43
42	2803754650.8288	5181448545.8944	1	43	44
43	5607509301.6576	10362897091.7888	1	44	45
44	11215018603.3152	20725794183.5776	1	45	46
45	22430037206.6304	41451588367.1552	1	46	47

REDUCED UPPER TRIANGULAR STIFFNESS MATRIX

ROW 1	0.06370E 04	-0.10372E 05	0.43303E 04	-0.11434E 04	0.18516E 03	-0.19469E 05	0.49571E 04	0.10394E 04	-0.10302E 03
	0.74871E 02	-0.16368E 04	0.19325E 05	-0.30340E 04	-0.30340E 04	0.60682E 03	-0.25693E 03	-0.12255E 03	-0.21745E 01
	0.20821E 01	-0.01275E 00	0.30850E 01	-0.24938E 00	0.37647E 00	-0.58375E 00	0.31600E 00	-0.47100E 00	-0.13917E 00
	0.13291E-01								
ROW 2	0.20272E 05	-0.21823E 05	0.60105E 04	-0.14128E 04	0.41947E 04	-0.56024E 05	0.28599E 04	0.13471E 04	-0.29345E 02
	0.27636E 04	-0.53489E 04	0.34812E 05	-0.42998E 03	-0.60817E 04	-0.62565E 03	-0.15365E 03	-0.11234E 03	0.32691E 00
	0.13398E 02	0.10761E 02	0.43540E 01	-0.16751E 00	0.28311E 00	-0.27459E 01	-0.11971E 01	-0.30642E 00	-0.50225E-01
ROW 3	0.30819E 05	-0.23913E 05	0.50900E 04	0.17271E 04	0.31591E 04	-0.20826E 05	0.30681E 04	0.07509E 02	-0.16150E 04
	0.12202E 04	-0.15187E 04	0.23399E 03	-0.28985E 03	0.11340E 05	-0.49387E 03	-0.39950E 03	0.13187E 01	0.41306E 00
	-0.11208E 03	0.04309E 01	0.11333E 01	-0.54035E 00	-0.30371E 00	0.19166E 02	-0.10747E 01	-0.00094E 00	
ROW 4	0.35810E 05	-0.13598E 05	-0.15172E 03	0.20809E 04	0.26682E 04	-0.21412E 05	0.40815E 04	0.64310E 03	-0.10490E 01

6	7	8	9	10	11	12	13	14	15	16	17	18	19	20	21	22	23	24	25	26	27	28	

LUMPED WEIGHTS

JOINT NO.	WEIGHT
1	0.058
2	0.118
3	0.138
4	0.178
5	0.198
6	0.158
7	0.388
8	0.388
9	0.388
10	0.168
11	0.688
12	0.168
13	0.488
14	0.188
15	0.328
16	0.318
17	0.788
18	0.148
19	0.623
20	0.478
21	0.878
22	0.878
23	0.888
24	0.338
25	0.858
26	0.858
27	0.858
28	0.428
29	0.858

BEAM ELEMENT PROPERTIES

ELEMENT NO.	A	J	K	MAY	JOINT 1	JOINT 2
1	0.	0.000	0.005	1	1	2
2	0.	0.000	0.005	1	2	3
3	0.	0.000	0.005	1	3	4

NOT REPRODUCIBLE

NOT REPRODUCIBLE

NOT REPRODUCIBLE

SECRET

4	0.0018	1	4	5	7
5	0.0164	1	6	7	8
6	0.0308	1	7	8	9
7	0.0452	1	8	9	10
8	0.0596	1	9	10	11
9	0.0740	1	10	11	12
10	0.0884	1	11	12	13
11	0.1028	1	12	13	14
12	0.1172	1	13	14	15
13	0.1316	1	14	15	16
14	0.1460	1	15	16	17
15	0.1604	1	16	17	18
16	0.1748	1	17	18	19
17	0.1892	1	18	19	20
18	0.2036	1	19	20	21
19	0.2180	1	20	21	22
20	0.2324	1	21	22	23
21	0.2468	1	22	23	24
22	0.2612	1	23	24	25
23	0.2756	1	24	25	26
24	0.2900	1	25	26	27
25	0.3044	1	26	27	28
26	0.3188	1	27	28	29
27	0.3332	1	28	29	30
28	0.3476	1	29	30	31
29	0.3620	1	30	31	32
30	0.3764	1	31	32	33
31	0.3908	1	32	33	34
32	0.4052	1	33	34	35
33	0.4196	1	34	35	36
34	0.4340	1	35	36	37
35	0.4484	1	36	37	38
36	0.4628	1	37	38	39
37	0.4772	1	38	39	40
38	0.4916	1	39	40	41
39	0.5060	1	40	41	42
40	0.5204	1	41	42	43
41	0.5348	1	42	43	44
42	0.5492	1	43	44	45
43	0.5636	1	44	45	46
44	0.5780	1	45	46	47
45	0.5924	1	46	47	48

REFUCED UPPER TRIANGULAR STIFFNESS MATRIX

ROW 1	0.66370E 04	-0.19072E 05	0.43303E 04	-0.11434E 04	0.18516E 03	-0.19469E 05	0.49571E 04	0.10394E 04	-0.10302E 03
	0.74871E 02	-0.16368E 04	0.19325E 05	-0.30610E 04	-0.38344E 04	0.64052E 03	-0.25693E 03	-0.12259E 03	-0.21745E 01
	0.20821E 01	-0.61275E 00	0.30803E 01	-0.24930E 00	0.07647E 00	-0.53375E 00	0.31680E 00	-0.47100E 00	-0.13917E 00
ROW 2	0.20272E 05	-0.21823E 05	0.66005E 04	-0.14120E 04	0.41947E 04	-0.36024E 05	0.28599E 04	0.13471E 04	-0.29349E 02
	0.27436E 04	-0.23489E 04	0.34812E 05	0.42990E 03	-0.68817E 04	-0.62565E 03	-0.13365E 03	-0.11234E 03	0.32691E 00
	0.13590E 02	0.10761E 02	0.43540E 01	-0.36751E 00	0.20311E 00	-0.27459E 01	-0.11971E 01	-0.30642E 00	-0.50225E 01
ROW 3	0.30819E 05	-0.23913E 05	0.58004E 04	-0.17271E 04	0.31501E 04	-0.26526E 05	0.30681E 04	0.87589E 03	-0.16158E 04
	0.12282E 04	-0.15187E 04	0.23099E 03	-0.20895E 03	0.11340E 05	-0.49387E 03	-0.39956E 03	0.13107E 01	0.41866E 00
	-0.11200E 03	0.04099E 01	0.11313E 01	-0.54035E 00	-0.30371E 00	0.19166E 02	-0.10747E 01	-0.60094E 00	
ROW 4	0.35030E 05	-0.13590E 05	0.15172E 03	0.28009E 04	0.26682E 04	-0.21412E 05	0.46815E 04	0.64310E 03	-0.19450E 04
	0.06400E 01	0.06400E 02	0.44000E 03	0.68300E 03	0.66600E 03	0.37200E 03	0.06600E 03	0.06600E 03	0.06600E 03

ROW 5	0.12121E 05	0.14984E 03	-0.46181E 02	0.12191E 04	0.44254E 04	-0.1727E 02	-0.15410E 03	0.15688E 03	-0.32540E 03
	0.36594E 02	-0.47458E 02	-0.27465E 03	-0.40059E 03	0.90847E 04	0.46764E 00	-0.12745E 03	0.30319E 01	0.11191E 02
	-0.56238E 02	0.31910E 02	-0.51089E 03	-0.95181E 00	-0.13530E 01	0.93892E 01			
ROW 6	0.62807E 02	-0.74782E 06	0.94104E 05	-0.21837E 05	0.29930E 04	-0.23669E 00	-0.11222E 00	0.56881E 00	0.54317E 07
	-0.78634E 05	-0.15897E 05	-0.66798E 04	-0.14826E 04	-0.53089E 04	0.25777E 01	0.17654E 03	-0.11861E 03	0.38268E 02
	0.97516E 03	-0.54651E 02	-0.41797E 02	-0.17177E 02	-0.73897E 00				
ROW 7	0.43331E 07	-0.58141E 06	0.31141E 06	-0.18185E 05	5.31242E 06	0.28952E 06	-0.67228E 07	-0.76567E 05	0.29776E 07
	0.12239E 06	0.51969E 05	-0.59182E 04	0.59310E 03	-0.96339E 04	-0.13744E 04	0.25808E 03	-0.10385E 03	-0.06910E 02
	0.13109E 04	0.23215E 03	-0.19044E 02	-0.19497E 02					
ROW 8	0.87938E 06	-0.47870E 04	0.76228E 05	-0.18785E 06	0.15349E 06	-0.29808E 05	0.14110E 05	0.18686E 00	-0.36673E 30
	0.13488E 06	0.11986E 05	-0.18848E 03	0.51883E 03	0.19943E 05	0.49333E 03	-0.72374E 02	0.18344E 03	0.12808E 03
	-0.39833E 04	0.17787E 03	0.75148E 02						
ROW 9	0.78198E 06	-0.31887E 04	0.24485E 05	-0.37328E 05	0.13956E 05	-0.70165E 04	0.29979E 05	0.13489E 00	-0.43694E 06
	0.18446E 06	-0.17818E 03	0.18388E 03	0.68875E 03	0.19243E 05	0.56628E 03	0.25777E 02	0.4382E 02	0.18679E 03
	-0.38888E 04	0.19762E 03							
ROW 10	0.32158E 06	-0.44139E 04	0.63475E 04	-0.12238E 04	0.14177E 04	-0.64281E 04	0.12670E 05	0.18821E 00	-0.29710E 06
	0.15172E 02	-0.01726E 02	-0.19164E 03	0.98481E 03	0.98549E 04	0.53886E 01	0.14148E 02	0.69736E 02	0.15874E 03
	-0.28826E 04								
ROW 11	0.17888E 07	-0.19994E 07	0.32983E 06	-0.21365E 06	0.29282E 06	-0.98888E 05	0.24283E 05	-0.39738E 04	-0.23969E 04
	0.41548E 04	-0.24663E 04	0.88465E 03	-0.17838E 03	-0.99710E 02	0.13978E 03	-0.77436E 02	0.50434E 02	-0.13163E 02
ROW 12	0.25195E 08	-0.16063E 07	-0.11253E 08	0.20228E 06	0.13921E 06	-0.34675E 05	0.56888E 04	0.33892E 05	-0.40568E 04
	0.27639E 04	-0.12568E 04	0.21810E 03	-0.57925E 04	0.21637E 03	0.58544E 03	0.11893E 02	0.32188E 02	
ROW 13	0.13641E 08	0.52655E 06	-0.67395E 07	-0.58898E 05	0.81836E 04	-0.28828E 04	-0.63838E 03	0.53419E 05	0.24244E 04
	0.18841E 03	-0.13694E 03	0.58299E 03	-0.78512E 04	-0.22189E 03	0.22977E 03	0.57862E 02		
ROW 14	0.62167E 07	-0.68926E 06	0.88817E 05	-0.17532E 05	0.25887E 04	-0.35568E 05	0.13163E 04	0.15888E 04	-0.34254E 03
	0.12187E 03	0.78824E 04	-0.49988E 03	-0.37836E 03	-0.18781E 03	-0.11752E 02			
ROW 15	0.43185E 07	-0.44449E 06	0.9821E 05	-0.14948E 04	0.25498E 04	-0.58738E 05	-0.36121E 03	0.14351E 04	-0.23727E 03
	-0.59889E 03	0.95839E 04	-0.21295E 03	-0.24131E 03	-0.88296E 02				
ROW 16	0.04315E 06	-0.43352E 06	0.67245E 05	0.91798E 03	0.14139E 04	-0.59128E 05	0.18181E 04	0.69393E 03	-0.23474E 03
	-0.28677E 03	0.81971E 04	-0.88817E 03	-0.14834E 03					
ROW 17	0.76125E 06	-0.38137E 06	-0.24407E 03	0.17832E 04	0.17458E 04	-0.39328E 05	0.25519E 04	-0.55816E 02	-0.11882E 03
	-0.39111E 03	0.81188E 04	-0.37639E 03						
ROW 18	0.31213E 06	0.88647E 02	-0.27658E 03	0.78258E 03	0.22157E 04	-0.28817E 05	-0.79818E 01	-0.43558E 02	-0.14365E 03
	-0.33854E 03	0.41343E 04							
ROW 19	0.85668E 05	-0.47934E 05	0.27688E 05	-0.67282E 04	0.18186E 04	-0.72441E 04	0.39688E 04	0.25735E 03	-0.87764E 02

0.74730E 02

ROW 20 0.13911E 06 -0.11803E 06 0.48102E 05 -0.64630E 04 0.39580E 04 -0.12142E 03 0.28734E 04 0.13792E 04 -0.28031E 03

ROW 21 0.18527E 06 -0.11001E 06 0.26942E 05 0.26947E 03 0.28675E 04 -0.13399E 03 0.31078E 04 0.00975E 03

ROW 22 0.14375E 06 -0.47169E 05 -0.95025E 02 0.13010E 04 0.31995E 04 -0.14708E 03 0.32842E 04

ROW 23 0.35523E 05 0.78831E 02 -0.20101E 03 0.08760E 03 0.39057E 04 -0.76779E 04

ROW 24 0.15540E 05 -0.27452E 05 0.16544E 05 -0.46397E 04 0.61450E 03

ROW 25 0.73150E 05 -0.63877E 05 0.23654E 05 -0.38210E 04

ROW 26 0.97933E 05 -0.63957E 05 0.15830E 05

ROW 27 0.74114E 05 -0.27225E 05

ROW 28 0.15845E 05

R E O U C E R U P P E R T R I A N G U L A R F L E X I B I L I T Y M A T R I X

ROW 1 0.46916E-03 0.38422E-03 0.95173E-03 0.35257E-03 0.36603E-03 0.51584E-04 0.79629E-04 0.10505E-03 0.12039E-03
0.15101E-03 0.87414E-03 0.19157E-04 0.44402E-04 -0.12834E-04 0.96582E-05 0.32235E-04 0.54675E-04 0.72882E-04
-0.28877E-03 -0.18272E-03 -0.16479E-03 -0.14581E-03 -0.12589E-03 -0.39581E-03 -0.37736E-03 -0.35954E-03 -0.30152E-03
-0.32311E-03

ROW 2 0.49723E-03 0.50666E-03 0.02817E-03 0.69754E-03 0.76772E-04 0.17215E-03 0.26864E-03 0.36271E-03 0.45575E-03
0.88434E-03 0.44482E-04 0.33517E-03 0.12415E-04 0.98810E-04 0.18953E-03 0.28169E-03 0.37412E-03 -0.18282E-03
-0.18376E-03 -0.28785E-04 0.64638E-04 0.15182E-03 -0.39384E-03 -0.31146E-03 -0.22954E-03 -0.14884E-03 -0.63813E-04

ROW 3 0.76888E-03 0.92198E-03 0.18702E-02 0.10282E-03 0.26489E-03 0.43803E-03 0.61332E-03 0.78656E-03 0.89273E-03
0.68888E-04 0.27715E-03 0.37889E-04 0.18988E-03 0.35367E-03 0.52361E-03 0.69536E-03 -0.16977E-03 -0.21599E-04
0.33833E-03 0.28742E-03 0.4821E-03 -0.39228E-03 -0.24259E-03 -0.92822E-04 0.68194E-04 0.21456E-03

ROW 4 0.12342E-02 0.14961E-02 0.12738E-03 0.35736E-03 0.61106E-03 0.87035E-03 0.11439E-02 0.98205E-03 0.95134E-04
0.31943E-03 0.62982E-04 0.28185E-03 0.52803E-03 0.77932E-03 0.18425E-02 -0.15379E-03 0.65915E-04 0.29248E-03
0.52825E-03 0.77869E-03 -0.38821E-03 -0.16456E-03 0.68825E-04 0.28915E-03 0.52897E-03

ROW 5 0.19592E-02 0.15278E-03 0.44992E-03 0.78956E-03 0.11438E-02 0.15147E-02 0.91140E-05 0.12057E-03 0.41284E-03
0.88189E-04 0.37445E-03 0.69459E-03 0.18418E-02 0.14841E-02 -0.13710E-03 0.15715E-03 0.46217E-03 0.78150E-03
0.11111E-02 -0.38887E-03 -0.78947E-04 0.22577E-03 0.53499E-03 0.84932E-03

ROW 6 0.24126E-04 0.49431E-04 0.74743E-04 0.10895E-03 0.12546E-03 0.87419E-05 0.19162E-04 0.44586E-04 0.14287E-04
0.39598E-04 0.64881E-04 0.98289E-04 0.11562E-03 -0.12788E-04 0.12557E-04 0.37634E-04 0.63122E-04 0.80519E-04
-0.30699E-04 -0.14428E-04 0.18844E-04 0.36128E-04 0.61286E-04

ROW 7 0. 1E- 0.2. 5-0 1.32 -0.3 1.17 0.3 1.834 5 -0.87 17E 0.1 17E-0.1 1E-0.

0.2710E-03	0.4130E-03	0.7427E-03	0.11048E-03	0.2000E-03	0.3700E-03	0.2200E-03
0.60734E-04	0.15960E-03	0.25069E-03	0.24276E-03	0.11048E-03	0.2000E-03	0.3700E-03
ROM 8						
0.40335E-03	0.57455E-03	0.74614E-03	0.89279E-03	0.69813E-04	0.22741E-03	0.64986E-04
0.56080E-03	0.23233E-03	0.29710E-04	0.19133E-03	0.35688E-03	0.52345E-03	0.69264E-03
0.39229E-03	0.40791E-03	0.65499E-03				
ROM 9						
0.83727E-03	0.11019E-02	0.90211E-05	0.95140E-04	0.31961E-03	0.90216E-04	0.56110E-03
0.10885E-02	0.40296E-04	0.20544E-03	0.52955E-03	0.78104E-03	0.10371E-02	0.25070E-03
0.74349E-03	0.99386E-03					
ROM 10						
0.14727E-02	0.91147E-03	0.12057E-03	0.41217E-03	0.11569E-03	0.73304E-03	0.10064E-02
0.65752E-04	0.30020E-03	0.70669E-03	0.10464E-02	0.13954E-02	0.18297E-04	0.34577E-03
0.13973E-02						
ROM 11						
0.87888E-03	0.47420E-03	0.80199E-03	0.47421E-03	0.88350E-03	0.89201E-03	0.91130E-03
0.00359E-03	0.80290E-03	0.90222E-03	0.91150E-03	0.87406E-03	0.00378E-03	0.89325E-03
ROM 12						
0.10163E-04	0.44400E-04	0.19164E-04	0.44409E-04	0.44409E-04	0.98143E-04	0.10170E-04
0.60093E-04	0.95151E-04	0.12000E-03	0.19179E-04	0.44500E-04	0.69833E-04	0.95100E-04
ROM 13						
0.13521E-03	0.44512E-04	0.13521E-03	0.27740E-03	0.31961E-03	0.43220E-04	0.44221E-04
0.31957E-03	0.41200E-03	0.44073E-04	0.13570E-03	0.27759E-03	0.31951E-03	0.41100E-03
ROM 14						
0.24129E-04	0.49430E-04	0.74754E-04	0.10000E-03	0.12500E-03	0.51439E-04	0.10192E-03
0.15259E-03	0.70564E-04	0.10301E-03	0.12905E-03	0.15431E-03	0.17967E-03	
ROM 15						
0.14100E-03	0.23339E-03	0.32567E-03	0.41034E-03	0.79746E-04	0.17235E-03	0.26925E-03
0.11180E-03	0.20377E-03	0.29642E-03	0.30908E-03	0.40209E-03		
ROM 16						
0.40373E-03	0.57506E-03	0.74092E-03	0.10336E-03	0.27050E-03	0.40025E-03	0.61139E-03
0.30316E-03	0.47615E-03	0.64074E-03	0.81837E-03			
ROM 17						
0.03770E-03	0.11024E-02	0.32210E-03	0.36676E-03	0.61041E-03	0.67679E-03	0.11306E-02
0.65920E-03	0.91499E-03	0.11730E-02				
ROM 18						
0.10220E-02	0.13944E-03	0.40107E-03	0.96620E-03	0.11453E-02	0.15022E-03	0.10220E-03
0.11061E-02	0.15346E-02					
ROM 19						
0.37317E-03	0.36295E-03	0.34020E-03	0.34451E-03	0.34460E-03	0.72276E-03	0.67183E-03
0.57020E-03						
ROM 20						
0.45027E-03	0.54107E-03	0.62210E-03	0.70539E-03	0.80340E-03	0.74122E-03	0.83479E-03
ROM 21						
0.73315E-03	0.91040E-03	0.10400E-02	0.65399E-03	0.84936E-03	0.10401E-02	0.12223E-02
ROM 22						
0.12040E-02	0.14087E-02	0.62035E-03	0.93093E-03	0.12361E-02	0.15402E-02	0.10414E-02
ROM 23						
0.10176E-02	0.60300E-03	0.10005E-02	0.14240E-02	0.10549E-02	0.22972E-02	

NOV 24 0.16762E-02 0.14871E-02 0.13328E-02 0.12236E-02 0.11308E-02
 NOV 25 0.15410E-02 0.15678E-02 0.15941E-02 0.16311E-02
 NOV 26 0.17884E-02 0.19734E-02 0.21079E-02
 NOV 27 0.23496E-02 0.27002E-02
 NOV 28 0.32988E-02

REDUCED UPPER TRIANGULAR VELOCITY MATRIX

NOV 1	0.50000E-01	0.	0.	0.	0.	0.	0.	0.	0.
	0.	0.	0.	0.	0.	0.	0.	0.	0.
	0.	0.	0.	0.	0.	0.	0.	0.	0.
NOV 2	0.11000E 00	0.	0.	0.	0.	0.	0.	0.	0.
	0.	0.	0.	0.	0.	0.	0.	0.	0.
	0.	0.	0.	0.	0.	0.	0.	0.	0.
NOV 3	0.11500E 00	0.	0.	0.	0.	0.	0.	0.	0.
	0.	0.	0.	0.	0.	0.	0.	0.	0.
	0.	0.	0.	0.	0.	0.	0.	0.	0.
NOV 4	0.17500E 00	0.	0.	0.	0.	0.	0.	0.	0.
	0.	0.	0.	0.	0.	0.	0.	0.	0.
	0.	0.	0.	0.	0.	0.	0.	0.	0.
NOV 5	0.18600E 00	0.	0.	0.	0.	0.	0.	0.	0.
	0.	0.	0.	0.	0.	0.	0.	0.	0.
	0.	0.	0.	0.	0.	0.	0.	0.	0.
NOV 6	0.15500E 00	0.	0.	0.	0.	0.	0.	0.	0.
	0.	0.	0.	0.	0.	0.	0.	0.	0.
	0.	0.	0.	0.	0.	0.	0.	0.	0.
NOV 7	0.30500E 00	0.	0.	0.	0.	0.	0.	0.	0.
	0.	0.	0.	0.	0.	0.	0.	0.	0.
	0.	0.	0.	0.	0.	0.	0.	0.	0.
NOV 8	0.30500E 00	0.	0.	0.	0.	0.	0.	0.	0.
	0.	0.	0.	0.	0.	0.	0.	0.	0.
	0.	0.	0.	0.	0.	0.	0.	0.	0.
NOV 9	0.37500E 00	0.	0.	0.	0.	0.	0.	0.	0.
	0.	0.	0.	0.	0.	0.	0.	0.	0.
	0.	0.	0.	0.	0.	0.	0.	0.	0.

HERE ARE THE EIGENVALUES AND EIGENVECTORS

EIGENVECTOR NUMBER 1

CORRESPONDING TO 1.0001270E 02
 7.567196E-02 2.736202E-01 4.076606E-01 7.116904E-01 9.433304E-01 0.0916361E-02
 2.717211E-01 4.025430E-01 7.850290E-01 9.340483E-01 0.2820189E-01 0.1722525E-02
 2.731350E-01 0.295070E-07 2.747537E-01 4.050843E-01 7.007406E-01 9.3600690E-01
 9.192926E-02 2.991735E-01 5.115438E-01 7.345741E-01 9.299281E-01 1.1650926E-01
 3.341766E-01 5.536607E-01 7.755131E-01 1.000000E 00

EIGENVECTOR NUMBER 2

CORRESPONDING TO 6.6555182E 02
 -3.074703E-01 -4.760260E-01 -4.692046E-01 -5.107593E-01 -5.465090E-01 -5.1596330E-02
 -6.739273E-02 -0.511256E-02 -1.411039E-01 -1.171847E-01 7.928531E-04 -2.170193E-03
 -1.029153E-07 4.744119E-07 4.722447E-02 4.471306E-02 3.747701E-02 2.6056935E-07
 4.208439E-01 4.425511E-01 4.626226E-01 4.767681E-01 4.002161E-01 9.129032E-01
 9.335401E-01 9.540493E-01 9.771205E-01 1.000000E 00

EIGENVECTOR NUMBER 3

CORRESPONDING TO 6.5344921E 06
 2.146236E-01 2.309371E-01 2.476327E-01 2.591660E-01 2.630717E-01 2.650056E-02
 1.124916E-02 -1.401322E-02 -5.049018E-02 -1.132913E-01 1.000000E-02 1.391492E-02
 1.704045E-03 1.238609E-07 -5.787737E-03 -4.942160E-07 -1.126402E-01 -1.017506E-01
 3.620850E-01 2.080953E-01 4.173977E-02 -1.390847E-01 -3.292202E-01 1.000000E 00
 6.040976E-01 3.592976E-01 4.1580110E-02 -2.6750100E-01

EIGENVECTOR NUMBER 4

CORRESPONDING TO 6.0003014E 06
 3.039769E-01 2.396016E-01 2.091650E-01 1.770175E-01 2.005674E-01 -2.316207E-02
 -7.270719E-02 -1.158248E-01 -1.244090E-01 -1.129740E-01 -2.620303E-02 -4.997575E-02
 -1.043710E-01 -7.701524E-07 1.305493E-01 -1.504013E-01 -1.569900E-01 -1.411520E-01
 -3.506702E-01 -2.292544E-01 -8.530620E-02 0.091304E-02 2.625980E-01 -4.064891E-01
 -1.322519E-01 2.400270E-01 6.196743E-01 1.000000E 00

EIGENVECTOR NUMBER 5

CORRESPONDING TO 1.063140E 07
 1.000000E 00 0.094497E-01 5.4017230E-01 -6.711402E-07 -7.742793E-01 1.607870E-01
 2.452103E-01 2.102012E-01 4.746725E-02 -1.022122E-01 0.7103271E-02 1.293590E-01
 2.000700E-01 9.370546E-07 1.7510534E-01 1.7000717E-01 5.1930000E-02 -1.3916540E-01
 -4.044000E-02 3.133244E-02 7.150250E-02 6.3065669E-02 2.000500E-02 -1.5464067E-01
 -6.4052110E-02 1.570640E-02 3.6740610E-02 1.2101973E-01

EIGENVECTOR NUMBER 6

CORRESPONDING TO 2.0904730E 07
 1.000000E 00 4.0054750E-01 -1.000000E-01 -2.9063763E-01 -1.000000E-01 -1.7544073E-01
 -2.044100E-01 -2.100200E-01 0.469350E-02 5.132007E-01 -1.472700E-01 -2.0067140E-01
 -3.130515E-01 -2.410191E-01 -3.3784510E-01 -7.303748E-01 1.0673060E-01 5.5022704E-01
 -7.720451E-02 -1.725412E-01 -1.5612361E-01 -5.9120365E-03 2.2326007E-01 6.0771033E-01
 3.2590143E-01 -3.0539907E-03 -2.432791E-01 -4.1007000E-01

EIGENVECTOR NUMBER 7

CORRESPONDING TO 4.0107300E 07
 1.000000E 00 9.005355E-02 -5.067357E-01 -4.3070097E-01 3.095530E-01 1.2152206E-01
 0.201014E-02 4.010730E-03 -6.709247E-02 -9.337552E-02 1.1003741E-01 1.1069492E-01
 1.042004E-01 1.201200E-01 1.1095015E-01 6.066293E-02 -1.000000E-02 -7.0924940E-02
 2.050070E-01 1.201200E-01 1.064000E-01 6.364270E-02 1.1732670E-02 -1.0100071E-01
 -1.2301570E-01 -5.417200E-02 -1.9700240E-02 -5.2101101E-03

EIGENVECTOR NUMBER 8

CORRESPONDING TO 5.100000E 07

-3.143002F-02 1.749340E-01 3.0704056E-01 2.372204RE-01 2.3674506E-02 -1.0891701E-01
-1.052691E-01 -2.424025RE-01 -2.4995175E-01 -2.1076206E-01 1.9202267E-02 -8.6933946E-03
-8.2062031E-02 0.431237RE-02 2.5445667E-02 -7.5469335E-02 -1.0654639E-02 3.3403299E-02
1.800000E 00 6.2771085E-01 4.3409804E-01 5.7694630E-01 9.9900319E-01 -4.6042169E-02
-5.4657514E-01 -7.7306767E-01 -5.0726696E-01 -1.0706199E-01

EIGENVECTOR NUMBER 9
CORRESPONDING TO 7.410150E 07

-4.6607439F-02 -5.0956507E-02 -5.0074909E-02 -4.559305RE-02 -1.4199975E-02 9.0869517E-02
9.0839625E-02 6.2602134E-02 4.2767000E-02 4.072011E-02 7.7757601E-02 7.6829145E-02
5.9377179F-02 5.5346157E-02 2.5313066E-02 -7.340677RE-02 -3.6919065E-02 -1.2333020E-02
3.1976502E-01 -2.6420070E-01 -2.3340360E-01 -1.2603445E-01 2.2152110E-01 1.0000000E 00
-1.3333133E-01 -6.9750713E-01 -2.1024702E-01 9.007206E-01

HERE ARE THE NATURAL FREQUENCIES

THE NATURAL FREQUENCY NUMBER	1	IS	69.845	CPS
THE NATURAL FREQUENCY NUMBER	2	IS	129.890	CPS
THE NATURAL FREQUENCY NUMBER	3	IS	486.842	CPS
THE NATURAL FREQUENCY NUMBER	4	IS	474.760	CPS
THE NATURAL FREQUENCY NUMBER	5	IS	518.938	CPS
THE NATURAL FREQUENCY NUMBER	6	IS	896.858	CPS
THE NATURAL FREQUENCY NUMBER	7	IS	1807.934	CPS
THE NATURAL FREQUENCY NUMBER	8	IS	1238.934	CPS
THE NATURAL FREQUENCY NUMBER	9	IS	1370.042	CPS

APPENDIX B

Program FLUENC Listing


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S      FORTRAN DECK
CMAIN  PROGRAM FLUENC-FOR GENERATING STIFFNESS,FLEXIBILITY AND MASS
C      MATRICES FROM PLANE GRID BEAM AND TRIANG. PLATE ELEMENTS
      DIMENSION TITLE(20),YM(10),PR(10),GE(10),DENS(10),X(50),Y(50),
1 NR1(50),NR2(50),NR3(50),N1(50),N2(50),N3(50),NOSC(9),DCS(2),
1 STM(6,6),SHM(6,6),PLTK(9,9),PLTM(9,9),SSTF(11325),SM(11325),
1 RSMASS(150),A(11325),VALU(9),TEMP(50),B(150),C(100),DUM3(150),
1 F(150,3),IDUM4(50),JMASS(50)
      INT=GER OUT
      EQUIVALENCE(SSTF(1),SM(1),A(1)),(STM(1,1),SHM(1,1),PLTK(1,1),
1 PLTM(1,1))
1001 FORMAT(12A6)
1002 FORMAT(16I5)
1003 FORMAT(8E10,3)
1004 FORMAT(10X,2E10,3)
1005 FORMAT(3E10,3,3I5)
1006 FORMAT(E10,3,3I5)
1007 FORMAT(I5,5X,E10,3)
5000 FORMAT(1H1,12A6/1X,12A6)
5001 FORMAT(///6HNJTS =1,5X,6H NR =13,5X,6H NRE =13,5X,6H NPE =13,5X,
1 7HNMODE =13,5X,6HKEY =13,5X,7HNLUMP =1,5)
5002 FORMAT(///75HM A T E R I A L   P R O P E R T I E S *****
1 *****//3HNO.   YOUNG'S MODULUS   POISSON RATIO
1 MODULUS OF RIGIDITY   DENSITY,10(//12,0X,E12,5,9X,17,5,10X,E12,5,
1 16X,E12,5))
5003 FORMAT(///34HJ O I N T   C O O R D I N A T E S/35HJOINT NO.   X
1 COORD.   Y COORD.)
5004 FORMAT(15,7X,E10,5,5X,E10,5)
5005 FORMAT(///64HJ O I N T   R E S T R A I N T   C O D E *****
1 *****//67HJOINT NO.   Z DISPLACEMENT   ROTATION ABOUT X
1 ROTATION ABOUT Y)
5006 FORMAT(15,116,119,120)
5008 FORMAT(///75HE F A M   E L E M E N T   P R O P E R T I E S *****
1 *****//75HELEMENT NO.   A   I
1 J   MAT   JOINT 1   JOINT 2)
5009 FORMAT(16,8X,F0,4,4X,F0,4,4X,F0,4,2X,12,6X,12,9X,12)
5010 FORMAT(///122HT R I A N G U L A R   P L A T E   E L E M E N T
1 P R O P E R T I E S *****
1 ****//122HELEMENT NO.   T   MAT   JOINT 1   JOINT 2   JOINT
1 3   DX   DY   D1   DXY   DETA)
5011 FORMAT(16,8 .FR,4,3X,12,6X,12,9X,12,8X,12,6X,E11,5,3X,E11,5,3X,
1 F11,5,3X,E11,5,3X,E6,2)
5020 FORMAT(///60HCOORDINATE NUMBERS FOR EACH Z DISPLACEMENT AT EACH UN
1 RESTRAINED JOINT//5HJOINT NO.   COORD. NO.)
5021 FORMAT(15,116)
5022 FORMAT(///24HL I M P E D   W E I G H T S,25HJOINT NO.   WEIG
1 HT)
5023 FORMAT(15,6 .F10,4)
      IV=0
C      DISC ASSIGNMENTS
      IN=6
      CALL FLGEOF(IN,IV)
      OUT=6

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MDISC=7
NDISC=8
IDISC=9
JDISC=10
KDISC=11
C   BEGIN INPUT OF DATA
100 READ(IN,1000) (TITLE(I),I=1,24)
    IF (IV.NE.0) CALL EXIT
    REWIND MDISC
    REWIND NDISC
    REWIND IDISC
    REWIND JDISC
    REWIND KDISC
    WRITE(OUT,0000) (TITLE(I),I=1,24)
    READ(IN,1001) NJTS,NR,NBE,NPE,NMODE,MKEY,NLUMP
C   NJTS=NO. OF JOINTS, NR=NO. OF JOINTS WITH RESTRAINTS
C   NBE=NO. OF BEAM ELEMENTS, NPE=NO. OF TRIANGULAR PLATE ELEMENTS
C   NMODE=NO. OF EIGENVALUES AND EIGENVECTORS DESIRED
C   MKEY = 1 DO NOT COMPUTE ELEMENTAL CONSISTENT MASS TERMS
C   MKEY = 2 COMPUTE ELEMENTAL CONSISTENT MASS TERMS
C   NLUMP = NO. OF LUMPED MASSES INPUT
    WRITE(OUT,0001) NJTS,NR,NBE,NPE,NMODE,MKEY,NLUMP
C   INPUT MATERIAL PROPERTIES
    READ(IN,1002) NMAT
    DO 10 I=1,NMAT
    READ(IN,1003) YM(I),PR(I),GE(I),DENS(I)
C   YM=YOUNG'S MOD./10**6, PR=POISSON RATIO, GE=MOD. OF RIGIDITY
C   DENS=DENSITY
    IF (GE(I).EQ.0.) GE(I)=YM(I)/(2.*(1.+PR(I)))
    YM(I)=YM(I)*1.E6
10  GE(I)=GE(I)*1.E6
    WRITE(OUT,0002) (I,YM(I),PR(I),GE(I),DENS(I),I=1,NMAT)
    DO 20 I=1,NMAT
20  DENS(I)=DENS(I)/(32.174*12.)
C   INPUT JOINT COORDINATES
    READ(IN,1004) (X(M),Y(M),M=1,NJTS)
    WRITE(OUT,0003)
    WRITE(OUT,0004) (M,X(M),Y(M),M=1,NJTS)
C   INPUT JOINT RESTRAINT CODE
C   0=FREE
C   1=CLAMPED
    DO 12 I=1,NJTS
    NR1(I)=0
    NR2(I)=0
    NR3(I)=0
    N1(I)=0
    N2(I)=0
12  N3(I)=0
    IF (NR.EQ.0) GO TO 80
    WRITE(OUT,0006)
    DO 11 I=1,NR
    READ(IN,1005) JT,M1,M2,M3

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NR1(JT)=M1
NR2(JT)=M2
NR3(JT)=M3
WRITE(OUT,1007) JT,M1,M2,M3
11 CONTINUE
50 CONTINUE
C   GENERATE COORDINATE NUMBERS FOR EACH DEGREE OF FREEDOM, " IF
C   CLAMPED, NORMAL DISPLACEMENTS ARE NUMBERED FIRST
C   N1, N2, N3 CONTAIN COORD. NUMBERS FOR EACH JOINT
C   NREDO = NO. OF NORMAL DISPLACEMENTS
C   NDF = NO. OF DEGREES OF FREEDOM INCLUDING ROTATIONS
CALL COORDN(NR1,NR2,NR3,N1,N2,N3,NJ1S,NREDO,NDF)
WRITE(OUT,1020)
DO 50 I=1,NJTS
IF(NR1(I).EQ.1) GO TO 50
WRITE(OUT,1021) I,N1(I)
50 CONTINUE
C   INPUT LUMPED MASSES
IF(NLUMP.EQ.0) GO TO 250
READ(IN,1006) ((JMASS(I),RSMASS(I)),I=1,NLUMP)
WRITE(OUT,1022)
DO 251 I=1,NLUMP
WRITE(OUT,1023) JMASS(I),RSMASS(I)
RSMASS(I)=RSMASS(I)/(32.174*12.)
251 CONTINUE
250 CONTINUE
NSSTF=NDF*(NDF+1)/2
DO 13 I=1,NSSTF
13 SSTF(I)=0.
IF(NBE.EQ.1) GO TO 200
C   BEGIN TO GENERATE BEAM STIFFNESS TERMS
WRITE(OUT,1008)
DO 14 NM=1,NBE
C   INPUT BEAM ELEMENT PROPERTIES
READ(IN,1004) AR,XI,YJ,MAT,JTNR,JTFR
C   AR=AREA OF BEAM CROSS SECTION, XI=AREA MOMENT OF INERTIA,
C   YJ=EFFECTIVE TORSIONAL MOMENT OF INERTIA, MAT=MATERIAL CODE
C   JTNR,JTFR=JOINT NUMBERS AT ENDS
WRITE(OUT,1009) NM,AR,XI,YJ,MAT,JTNR,JTFR
C   SET UP CODE NUMBERS
NOSC(1)=N1(JTNR)
NOSC(2)=N2(JTNR)
NOSC(3)=N3(JTNR)
NOSC(4)=N1(JTFR)
NOSC(5)=N2(JTFR)
NOSC(6)=N3(JTFR)
IF(MKEY.EQ.1) GO TO 253
C   STORE INFO. FOR LATER USE
WRITE(DISC) AR,XI,YJ,MAT,JTNR,JTFR,(NOSC(I),I=1,6)
253 CONTINUE
X1=X(JTNR)
X2=X(JTFR)
Y1=Y(JTNR)

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Y2=Y(JTFR)
FLNTH=SQRT((X2-X1)**2+(Y2-Y1)**2)
CALL TRANS(X1,X2,Y1,Y2,FLNTH,DCS)
E=YH(MAT)
G=GH(MAT)
CALL HFAMK(FLNTH,E,G,X1,YJ,STM,DCS)
DO 15 K=1,4
IF(NOSC(K).EQ.0) GO TO 15
I=NOSC(K)
DO 16 N=1,4
IF(NOSC(N).EQ.0) GO TO 16
J=NOSC(N)
IF(J.LT.I) GO TO 16
MM=(2*J+(I-1)*(2*NDF-1))/2
SSTF(MM)=SSTF(MM)+STM(K,N)
16 CONTINUE
15 CONTINUE
14 CONTINUE
200 CONTINUE
IF(NRE.EQ.1) GO TO 300
C BEGIN TO GENERATE TRIANGULAR PLATE STIFFNESS TERMS
WRITE(OUT,1010)
DO 17 NM=1,NPE
C INPUT TRIANGULAR PLATE ELEMENT PROPERTIES
READ(IN,1005) PTH,MAT,JT1,JT2,JT3,NDX
C PTH=PLATE THICKNESS, MAT=MATERIAL CODE,
C JT1,JT2,JT3=JOINT NUMBERS AT CORNERS, ANGLE AT JT1 MUST NOT BE
C 90 DEGREES
C DX,DY,D1,DXY,BETA = FLEXURAL RIGIDITY TERMS AND ANGLE OF MATERIAL
C PRINCIPAL AXES W/IN TRIANGLE LOCAL AXES
IF(NDX.EQ.1) READ(IN,1002) DX,DY,D1,DXY,BETA
IF(NDX.EQ.1) GO TO 38
BETA=0.
DX=(YH(MAT)*PTH**3)/(12.*(1.-PR(MAT)**2))
DY=DX
D1=PR(MAT)*DX
DXY=((1.-PR(MAT))/2.)*DX
18 BETA=BETA/17.2258
WRITE(OUT,1011) NM,PTH,MAT,JT1,JT2,JT3,DX,DY,D1,DXY,BETA
C SET UP CODE NUMBERS
NOSC(1)=N1(JT1)
NOSC(2)=N2(JT1)
NOSC(3)=N3(JT1)
NOSC(4)=N1(JT2)
NOSC(5)=N2(JT2)
NOSC(6)=N3(JT2)
NOSC(7)=N1(JT3)
NOSC(8)=N2(JT3)
NOSC(9)=N3(JT3)
IF(MKEY.EQ.1) GO TO 204
C STORE INFO. FOR LATER USE
WRITE(IDISC) PTH,MAT,JT1,JT2,JT3,(NOSC(I),I=1,9)

```

```

224 CONTINUE
  BX1=X(JT1)
  BX2=X(JT2)
  BY1=Y(JT1)
  BY2=Y(JT2)
  YZ=SQRT((BX2-BX1)**2+(BY2-BY1)**2)
  CALL TRANS(BX1,BX2,BY1,BY2,YZ,DCS)
  X3=DCS(2)*(X(JT3)-BX1)-DCS(1)*(Y(JT3)-BY1)
  Y3=DCS(1)*(X(JT3)-BX1)+DCS(2)*(Y(JT3)-BY1)
  CALL PLATEK(Y2,X3,Y3,DX,DY,DI,DOXY,HETA,DCS,PLTK)
  DO 19 K=1,4
  IF(NOSC(K).EQ.0) GO TO 19
  I=NOSC(K)
  DO 20 N=1,4
  IF(NOSC(N).EQ.0) GO TO 20
  J=NOSC(N)
  IF(J.LT.I) GO TO 20
  MM=(2*J+(I-1)*(2*NDF-1))/2
  SSTF(MM)=SSTF(MM)+PLTK(K,N)
20 CONTINUE
19 CONTINUE
17 CONTINUE
310 CONTINUE
C STORE FOR REDUCTION
DO 21 I=1,NDF
  NS=(2*I+(I-1)*(2*NDF-1))/2
  NE=(2*NDF+(I-1)*(2*NDF-1))/2
21 WRITE(MDISC) (SSTF(J),J=NS,NE)
  REWIND IDISC
DO 22 I=1,NSSTF
22 SM(I)=0.
  IF(MKEY.EQ.1) GO TO 22F
  IF(NBE.EQ.0) GO TO 201
C GENERATE BEAM MASS MATRICES
DO 23 NM=1,NBE
  READ(IDISC) AR,XI,YJ,MAT,JTNR,JTFR,(NOSC(I),I=1,6)
  X1=X(JTNR)
  X2=X(JTFR)
  Y1=Y(JTNR)
  Y2=Y(JTFR)
  FLNTH=SQRT((X2-X1)**2+(Y2-Y1)**2)
  CALL TRANS(X1,X2,Y1,Y2,FLNTH,DCS)
  RHO=DENS(MAT)
  CALL BEAMH(FLNTH,RHO,AR,XI,YJ,SMM,DCS)
DO 24 K=1,4
  IF(NOSC(K).EQ.1) GO TO 24
  I=NOSC(K)
DO 25 N=1,4
  IF(NOSC(N).EQ.0) GO TO 25
  J=NOSC(N)
  IF(J.LT.I) GO TO 25
  MM=(2*J+(I-1)*(2*NDF-1))/2
  SM(MM)=SM(MM)+SMM(K,N)

```

```

25 CONTINUE
24 CONTINUE
23 CONTINUE
201 CONTINUE
      IF(NPE.EQ.1) GO TO 301
C     GENERATE TRIANGULAR PLATE MASS MATRICES
      DO 26 NM=1,NPE
      READ(IDISC) PTH,MAT,JT1,JT2,JT3,(NOSC(I),I=1,9)
      RX1=X(JT1)
      RX2=X(JT2)
      RY1=Y(JT1)
      RY2=Y(JT2)
      YZ=SQRT((BX2-BX1)**2+(RY2-RY1)**2)
      CALL TRANS(BX1,BX2,RY1,RY2,YZ,DCS)
      X3=DCS(2)*(X(JT3)-BX1)-DCS(1)*(Y(JT3)-RY1)
      Y3=DCS(1)*(X(JT3)-BX1)+DCS(2)*(Y(JT3)-RY1)
      PRHO=DENS(MAT)
      CALL PLATEM(YZ,X3,Y3,PRHO,PTH,DCS,PLTH)
      DO 27 K=1,9
      IF(NOSC(K).EQ.0) GO TO 27
      I=NOSC(K)
      DO 28 N=1,9
      IF(NOSC(N).EQ.0) GO TO 28
      J=NOSC(N)
      IF(J.LT.I) GO TO 28
      MM=(2*J+(I-1)*(2*NDF-1))/2
      SM(MM)=SM(MM)+PLTH(K,N)
28 CONTINUE
27 CONTINUE
26 CONTINUE
301 CONTINUE
C     STORE FOR REDUCTION
225 CONTINUE
      DO 250 I=1,NLUMP
      NN=JMASS(I)
      IF(N1(NN).EQ.0) GO TO 258
      NNN=N1(NN)
      NS=(2*NNN+(NNN-1)*(2*NDF-1))/2
      SM(NS)=SM(NS)+RMASS(NNN)
258 CONTINUE
      DO 29 I=1,NDF
      NS=(2*I+(I-1)*(2*NDF-1))/2
      NF=(2*NDF+(I-1)*(2*NDF-1))/2
29 WRITE(NDISC) (SM(J),J=NS,NF)
      NOMASS=NDF-NREDU
      CALL FIGEN(A,VALU,TEMP,R,C,UUM,F,IDUM4,IDISC,JDISC,KDISC,NDISC,
      MDISC,NDF,NMODF,NMODE,NREDU,NOMASS)
      GO TO 100
      END

```

```

9      FORTRAN DECK
CCOORDN  ASSIGNS A COORD. NO. TO EACH DEGREE OF FREEDOM AT EACH JOINT
C      NR1,NR2,NR3 = ARRAYS CONTAINING RESTRAINT INFO. FOR EACH DEGREE
C      OF FREEDOM AT EACH JOINT (FREE=0, CLAMPED=1)
C      N1,N2,N3 = COORD. NO. FOR EACH DEGREE OF FREEDOM (NORMAL
C      DISPLACEMENTS ARE NUMBERED FIRST)
C      NJTS = NO. OF JOINTS
C      NRFDU = NO. OF NORMAL DISPLACEMENTS
C      NDF = TOTAL NO. OF DEGREES OF FREEDOM (INCLUDING ROTATIONS)
SUBROUTINE COORDN(NR1,NR2,NR3,N1,N2,N3,NJTS,NRFDU,NDF)
DIMENSION NR1(50),NR2(50),NR3(50),N1(50),N2(50),N3(50)
NO=1
DO 10 I=1,NJTS
IF(NR1(I).EQ.1) GO TO 10
N1(I)=NO
NO=NO+1
10 CONTINUE
NRFDU=NRFDU-1
DO 20 I=1,NJTS
IF(NR2(I).EQ.1) GO TO 21
N2(I)=NO
NO=NO+1
21 IF(NR3(I).EQ.1) GO TO 20
N3(I)=NO
NO=NO+1
20 CONTINUE
NDF=NDF-1
RETURN
END

```

```
1      FORTRAN DECK
CTRANS      TRANSFORMATION DIRECTION COSINES
C      X1,Y1 = COORDS. OF POINT 1
C      X2,Y2 = COORDS. OF POINT 2
C      FL = DISTANCE BETWEEN POINTS 1 AND 2
C      DCS = DIRECTION COSINES OF VECTOR FROM POINT 1 TO POINT 2
SUBROUTINE TRANS(X1,X2,Y1,Y2,FL,DCS)
DIMENSION DCS(2)
DCS(1)=(X2-X1)/FL
DCS(2)=(Y2-Y1)/FL
RETURN
END
```



```

S      FORTRAN DECK
CHFAMK      PLANE GRID BEAM ELEMENT STIFFNESS MATRIX IN SYSTEM COORDS.
C      FL = BEAM LENGTH
C      E = YOUNG'S MODULUS
C      G = MODULUS OF RIGIDITY
C      XI = AREA MOMENT OF INERTIA
C      YJ = EFFECTIVE TORSIONAL MOMENT OF INERTIA
C      STM = STIFFNESS MATRIX
C      DCS = DIRECTION COSINES
SURROUTINE BEAMK(FL,E,G,XI,YJ,STM,DCS)
DIMENSION STM(6,6),DCS(2)
Z1=E*XI/FL
Z2=G*YJ/FL
STM(1,1)=12.*Z1/(FL*FL)
STM(2,1)=6.*Z1*DCS(2)/FL
STM(2,2)=4.*Z1*DCS(2)*DCS(2)+Z2*DCS(1)*DCS(1)
STM(3,1)=-6.*Z1*DCS(1)/FL
STM(3,2)=(-4.*Z1+Z2)*DCS(1)*DCS(2)
STM(3,3)=4.*Z1*DCS(1)*DCS(1)+Z2*DCS(2)*DCS(2)
STM(4,1)=-STM(1,1)
STM(4,2)=-STM(2,1)
STM(4,3)=-STM(3,1)
STM(4,4)=STM(1,1)
STM(5,1)=STM(2,1)
STM(5,2)=2.*Z1*DCS(2)*DCS(2)-Z2*DCS(1)*DCS(1)
STM(5,3)=-Z2*Z1*DCS(1)*DCS(2)
STM(5,4)=-STM(2,1)
STM(5,5)=STM(2,2)
STM(6,1)=STM(3,1)
STM(6,2)=STM(3,3)
STM(6,3)=2.*Z1*DCS(1)*DCS(1)-Z2*DCS(2)*DCS(2)
STM(6,4)=-STM(3,1)
STM(6,5)=STM(3,2)
STM(6,6)=STM(3,3)
DO 10 I=2,6
N=I-1
DO 10 J=1,N
10 STM(J,I)=STM(I,J)
RETURN
END

```

```

S      FORTRAN DECK
CBEAMH      PLANE GRID BEAM ELEMENT MASS MATRIX IN SYSTEM COORDS.
C      FL = BEAM LENGTH
C      RHO = DENSITY
C      A = CROSS SECTIONAL AREA
C      XI = AREA MOMENT OF INERTIA
C      XJ = EFFECTIVE TORSIONAL MOMENT OF INERTIA
C      SMM = MASS MATRIX
C      DCS = DIRECTION COSINES
SURROUTINE KBEAMH(FL,RHO,A,XI,YJ,SMM,DCS)
DIMENSION SMM(6,6),DCS(2)
Z1=RHO*A*FL
Z2=FL**2
Z3=XI/A
DD=Z1*(13./30.+6.*Z3)/(5.*Z2)
CC=Z1*(11.*FL/10.+Z3/(10.*FL))
AA=Z1*(Z2/30.+2.*Z3/15.)
T1=Z1*YJ/(5.*A)
RR=Z1*(9./10.-(6.*Z3)/(5.*Z2))
QQ=Z1*(13.*FL/120.-Z3/(10.*FL))
SS=-Z1*(Z2/120.+Z3/30.)
PP=Z1*YJ/(5.*A)
SMM(1,1)=DD
SMM(2,1)=CC*DCS(2)
SMM(2,2)=AA*DCS(2)*DCS(2)+T1*DCS(1)*DCS(1)
SMM(3,1)=-CC*DCS(1)
SMM(3,2)=(-AA+T1)*DCS(1)*DCS(2)
SMM(3,3)=AA*DCS(1)*DCS(1)+T1*DCS(2)*DCS(2)
SMM(4,1)=RR
SMM(4,2)=QQ*DCS(2)
SMM(4,3)=-QQ*DCS(1)
SMM(4,4)=SMM(1,1)
SMM(5,1)=-SMM(4,2)
SMM(5,2)=SS*DCS(2)*DCS(2)+PP*DCS(1)*DCS(1)
SMM(5,3)=(-SS+PP)*DCS(1)*DCS(2)
SMM(5,4)=-SMM(4,1)
SMM(5,5)=SMM(2,2)
SMM(6,1)=-SMM(4,3)
SMM(6,2)=SMM(5,3)
SMM(6,3)=SS*DCS(1)*DCS(1)+PP*DCS(2)*DCS(2)
SMM(6,4)=-SMM(5,1)
SMM(6,5)=SMM(3,2)
SMM(6,6)=SMM(3,3)
DO 10 I=2,6
N=I-1
DO 10 J=1,N
10 SMM(J,I)=SMM(I,J)
RETURN
END

```

* FORTRAN DECK

C PLATEK

```
C THIS SUBROUTINE DETERMINES THE STIFFNESS MATRIX OF A
C TRIANGLE PLATE ELEMENT IN SYSTEM COORDS.
C Y2,X3,Y3 = COORDS. OF PLATE CORNERS IN LOCAL COORDINATES
C DX,DY,D1,DXY,HETA = FLEXURAL RIGIDITY TERMS AND ANGLE OF MATERIAL
C PRINCIPAL AXES W/O TRIANGLE LOCAL AXES
C DCS = DIRECTION COSINES
C PLTK = STIFFNESS MATRIX
SUBROUTINE PLATEK(Y2,X3,Y3,DX,DY,D1,DXY,HETA,DCS,PLTK)
DIMENSION PLTK(9,9),C(9,9),CINV(9,9),P(9,9),R(9,9)
DIMENSION T(9,9),STIFF(9,9),DCS(2)
EQUIVALENCE(P(1,:),STIFF(1,1)), (R(1,1),T(1,1))
CALL CHAT(Y2,X3,Y3,C)
CALL MINV(C,CINV,9)
CALL DINMAT(Y2,X3,Y3,DX,DY,D1,DXY,HETA,P)
CALL MATMPY(P,CINV,R,9)
DO 10 I=2,9
N=I-1
DO 10 J=1,N
ZZ1=CINV(I,J)
ZZ2=CINV(J,I)
CINV(I,J)=ZZ2
CINV(J,I)=ZZ1
10 CONTINUE
CALL MATMPY(CINV,R,STIFF,9)
DO 400 I=1,9
DO 400 J=1,9
400 T(I,J)=0.
T(1,1)=1.
T(4,4)=1.
T(7,7)=1.
T(2,2)=DCS(2)
T(3,3)=DCS(2)
T(5,5)=DCS(2)
T(6,6)=DCS(2)
T(8,8)=DCS(2)
T(9,9)=DCS(2)
T(2,3)=-DCS(1)
T(5,6)=-DCS(1)
T(8,9)=-DCS(1)
T(3,2)=DCS(1)
T(6,5)=DCS(1)
T(9,8)=DCS(1)
CALL MATMPY(STIFF,T,C,9)
T(2,3)=DCS(1)
T(5,6)=DCS(1)
T(8,9)=DCS(1)
T(3,2)=-DCS(1)
T(6,5)=-DCS(1)
T(9,8)=-DCS(1)
CALL MATMPY(T,C,PLTK,9)
RETURN
```

```

1      FORTRAN DECK
CCMAT
C      THIS SUBROUTINE FORMS THE C MATRIX RELATING THE CORNER
C      DISPLACEMENTS TO THE POLYNOMIAL DEFLECTION COEFFICIENTS
C      FOR THE TRIANGULAR PLATE ELEMENT
C      Y2,X3,Y3 = COORDS. OF PLATE CORNERS IN LOCAL COORDINATES
C      C = C MATRIX
C      SUBROUTINE CMAT(Y2,X3,Y3,C)
C      DIMENSION C(9,9)
C      DO 10 I=1,9
C      DO 10 J=1,9
10    C(I,J)=0.
C      C(1,1)=1.
C      C(2,3)=1.
C      C(3,2)=-1.
C      C(4,1)=1.
C      C(4,3)=Y2
C      C(4,6)=Y2**2
C      C(4,9)=Y2**3
C      C(5,3)=1.
C      C(5,6)=2.*Y2
C      C(5,9)=3.*Y2**2
C      C(6,2)=-1.
C      C(6,5)=-Y2
C      C(6,8)=-Y2**2
C      C(7,1)=1.
C      C(7,2)=X3
C      C(7,3)=Y3
C      C(7,4)=X3**2
C      C(7,5)=X3*Y3
C      C(7,6)=Y3**2
C      C(7,7)=X3**3
C      C(7,8)=X3*Y3**2+Y3*X3**2
C      C(7,9)=Y3**3
C      C(8,3)=1.
C      C(8,5)=X3
C      C(8,6)=2.*Y3
C      C(8,8)=2.*X3*Y3+X3**2
C      C(8,9)=3.*Y3**2
C      C(9,2)=-1.
C      C(9,4)=-2.*X3
C      C(9,5)=-Y3
C      C(9,7)=-3.*X3**2
C      C(9,8)=-Y3**2+2.*X3*Y3
C      RETURN
C      END

```

```

$      FORTRAN DECK
C      MINV      MATRIX INVERSION SUBROUTINE
C      A = MATRIX TO BE INVERTED
C      U = INVERTED MATRIX
C      NM = ORDER OF MATRIX (.LE.9)
C      SUBROUTINE MINV(A,U,NM)
C      DIMENSION A(9,9),U(9,9)
C      DO 9001 I=1,NM
C      DO 9001 J=1,NM
C      U(I,J)=0.0
C      IF (I.EQ.J) U(I,J)=1.0
9001 CONTINUE
C      EPS=0.00000001
C      DO 9015 I=1,NM
C      K=1
C      IF (I-NM) 9021,9007,9021
9021 IF (A(I,I)-EPS) 9007,9006,9007
9005 IF (-A(I,I)-EPS) 9006,9006,9007
9006 K=K+1
C      DO 9023 J=1,NM
C      U(I,J)=U(I,J)+U(K,J)
9023 A(I,J)=A(I,J)+A(K,J)
C      GO TO 9021
9007 DIV=A(I,I)
C      DO 9009 J=1,NM
C      U(I,J)=U(I,J)/DIV
9009 A(I,J)=A(I,J)/DIV
C      DO 9015 MM=1,NM
C      DELT=A(MM,I)
C      IF (ABS(DELT)-EPS) 9015,9015,9016
9016 IF (MM-I) 9010,9015,9010
9010 DO 9011 J=1,NM
C      U(MM,J)=U(MM,J)-U(I,J)*DELT
9011 A(MM,J)=A(MM,J)-A(I,J)*DELT
9015 CONTINUE
C      DO 9033 I=1,NM
C      DO 9033 J=1,NM
9033 A(I,J)=U(I,J)
C      RETURN
C      END

```

```

*      FORTRAN DECK
CDINMAT
C      THIS SUBROUTINE DETERMINES THE DOUBLE INTEGRAL MATRIX FOR
C      THE K EQUATION FOR THE TRIANGULAR PLATE ELEMENT
C      Y2,X3,Y3 = COORDS. OF PLATE CORNERS IN LOCAL COORDINATES
C      DX,DY,D1,DXY,BETA = FLEXURAL RIGIDITY TERMS AND ANGLE OF MATERIAL
C      PRINCIPAL AXES W/O TRIANGLE LOCAL AXES
C      P = DOUBLE INTEGRAL MATRIX
C      SUBROUTINE DINMAT(Y2,X3,Y3,DX,DY,D1,DXY,BETA,P)
C      DIMENSION P(9,9),D(3,3)
C      DO 10 I=1,9
C      DO 10 J=1,9
10 P(I,J)=0.
C      CALL DMAT(DX,DY,D1,DXY,BETA,D)
C      A1=DBLINT(Y2,X3,Y3,0,0)
C      A2=DBLINT(Y2,X3,Y3,1,0)
C      A3=DBLINT(Y2,X3,Y3,2,0)
C      A4=DBLINT(Y2,X3,Y3,0,1)
C      A5=DBLINT(Y2,X3,Y3,0,2)
C      A6=DBLINT(Y2,X3,Y3,3,1)
C      P(4,4)=4.*D(1,1)*A1
C      P(4,5)=4.*D(1,3)*A1
C      P(4,6)=4.*D(1,2)*A1
C      P(4,7)=12.*D(1,1)*A2
C      P(4,8)=4.*(D(1,1)*A4+D(1,2)*A2+2.*D(1,3)*(A2+A4))
C      P(4,9)=12.*D(1,2)*A4
C      P(5,5)=4.*D(3,3)*A1
C      P(5,6)=4.*D(3,2)*A1
C      P(5,7)=12.*D(3,1)*A2
C      P(5,8)=4.*(D(3,1)*A4+D(3,2)*A2+2.*D(3,3)*(A2+A4))
C      P(5,9)=12.*D(3,2)*A4
C      P(6,6)=4.*D(2,2)*A1
C      P(6,7)=12.*D(2,1)*A2
C      P(6,8)=4.*(D(2,1)*A4+D(2,2)*A2+2.*D(2,3)*(A2+A4))
C      P(6,9)=12.*D(2,2)*A4
C      P(7,7)=36.*D(1,1)*A3
C      P(7,8)=12.*(D(1,1)*A6+D(1,2)*A3+2.*D(1,3)*(A3+A6))
C      P(7,9)=36.*D(1,2)*A6
C      P(8,8)=4.*(D(1,1)*A5+D(1,2)*A6+2.*D(1,3)*(A6+A5))
1      +4.*(D(2,1)*A6+D(2,2)*A3+2.*D(2,3)*(A3+A6))
1      +8.*(D(3,1)*A6+D(3,2)*A3+2.*D(3,3)*(A3+A6))
1      +8.*(D(3,1)*A5+D(3,2)*A6+2.*D(3,3)*(A6+A5))
C      P(8,9)=12.*(D(1,2)*A5+D(2,2)*A6+2.*D(3,2)*(A6+A5))
C      P(9,9)=36.*D(2,2)*A5
C      DO 20 I=1,9
C      N=I+1
C      DO 20 J=N,9
20 P(J,I)=P(I,J)
C      RETURN
C      END

```

† FORTRAN DECK

CHAMPY

C MULTIPLIES MATRICES A AND B TO GET C, ALL OF ORDER N*N

 SUBROUTINE MATMPY(A,B,C,N)

 DIMENSION A(9,9),B(9,9),C(9,9)

 DO 10 I=1,N

 DO 10 J=1,N

 C(I,J)=0.

 DO 10 K=1,N

10 C(I,J)=C(I,J)+A(I,K)*B(K,J)

 RETURN

 END

* FORTRAN DECK

CDRLINT

```
C THIS SUBROUTINE EVALUATES THE DOUBLE INTEGRALS APPEARING IN THE
C EQUATIONS FOR K AND M FOR THE TRIANGULAR PLATE ELEMENT
C Y2,X3,Y3 = COORDS. OF PLATE CORNERS IN LOCAL COORDINATES
C M,N = POWER OF X AND Y RESPECTIVELY, PRZEMIENIECKI, PAGE 305
FUNCTION DRLINT(Y2,X3,Y3,M,N)
DIMENSION A(2),B1(2),P1(2),P2(2),P3(2)
EQUIVALENCE(R1(1),P3(1))
IF(M-1) 40,41,42
40 P1(1)=1.0
   N1=0
   GO TO 43
41 P1(1)=-1.0
   P1(2)=1.0
   N1=1
   GO TO 43
42 CONTINUE
   A1(1)=-1.0
   A1(2)=1.0
   R1(1)=-1.0
   R1(2)=1.0
   M1=1
   MM=M-1
   DO 10 J=1,MM
   CALL PLYMP(A1,1,R1,M1,P1,N1)
   NN1=N1+1
   DO 10 I=1,NN1
   R1(I)=P1(I)
   M1=N1
10 CONTINUE
43 CONTINUE
   IF(N-1) 50,51,52
50 P2(1)=1.0
   N2=0
   GO TO 53
51 P2(1)=-Y3+Y2
   P2(2)=Y3
   N2=1
   GO TO 53
52 CONTINUE
   A1(1)=-Y3+Y2
   A1(2)=Y3
   R1(1)=-Y3+Y2
   R1(2)=Y3
   M1=1
   NN=N-1
   DO 20 J=1,NN
   CALL PLYMP(A1,1,R1,M1,P2,N2)
   NN2=N2+1
   DO 20 I=1,NN2
   R1(I)=P2(I)
```

```
M1=N2
20 CONTINUE
23 CONTINUE
CALL PLYMP(P1,N1,P2,N2,P3,N3)
NN3=N3+1
SOL=0.
DO 30 I=1,NN3
SOL=SOL+(X***(M+1))*Y2*(1./FLOAT(M+N+2))* P3(I)*(1./FLOAT(N3+2-1))
30 CONTINUE
40 INT=SOL
RETURN
END
```



```

      FORTRAN DECK
CPLATEM
C   THIS SUBROUTINE DETERMINES THE MASS MATRIX OF A
C   TRIANGLE PLATE ELEMENT IN SYSTEM COORDS.
C   Y2,X3,Y3 = COORDS. OF PLATE CORNERS IN LOCAL COORDINATES
C   PRHO = DENSITY
C   PTH = PLATE THICKNESS
C   DCS = DIRECTION COSINES
C   PLTM = MASS MATRIX
C   SUBROUTINE PLATEM(Y2,X3,Y3,PRHO,PTH,DCS,PLTM)
C   DIMENSION PLTM(9,9),C(9,9),CINV(9,9),P(9,9),R(9,9)
C   DIMENSION T(9,9),FMASS(9,9),DCS(2)
C   EQUIVALENCI(P(1,1),FMASS(1,1)), (R(1,1),T(1,1))
C   CALL CMAT(Y2,X3,Y3,C)
C   CALL MINV(C,CINV,9)
C   CALL DIMMTH(Y2,X3,Y3,PRHO,PTH,P)
C   CALL MATMPY(P,CINV,R,9)
C   DO 10 I=2,9
C   N=I-1
C   DO 10 J=1,N
C   Z1=CINV(I,J)
C   Z2=CINV(J,I)
C   CINV(I,J)=Z2
C   CINV(J,I)=Z1
10 CONTINUE
C   CALL MATMPY(CINV,R,FMASS,9)
C   DO 400 I=1,9
C   DO 400 J=1,9
400 T(I,J)=0.
T(1,1)=1.
T(4,4)=1.
T(7,7)=1.
T(2,2)=DCS(2)
T(3,3)=DCS(2)
T(5,5)=DCS(2)
T(6,6)=DCS(2)
T(8,8)=DCS(2)
T(9,9)=DCS(2)
T(2,3)=-DCS(1)
T(5,6)=-DCS(1)
T(8,9)=-DCS(1)
T(3,2)=DCS(1)
T(6,5)=DCS(1)
T(9,8)=DCS(1)
CALL MATMPY(FMASS,T,C,9)
T(2,3)=DCS(1)
T(5,6)=DCS(1)
T(8,9)=DCS(1)
T(3,2)=-DCS(1)
T(6,5)=-DCS(1)
T(9,8)=-DCS(1)
CALL MATMPY(T,C,PLTM,9)
RETURN
END

```

```

*      FORTRAN DECK
CDINMTM
C      THIS SUBROUTINE DETERMINES THE DOUBLE INTEGRAL MATRIX FOR
C      THE TRIANGULAR PLATE M MATRIX - PRZEMIENIECKI, PAGE 304
C      Y2,X3,Y3 = COORDS. OF PLATE CORNERS IN LOCAL COORDINATES
C      PRHO = DENSITY
C      PTH = PLATE THICKNESS
C      P = DOUBLE INTEGRAL MATRIX
C      SURROUTINE DINMTM(Y2,X3,Y3,PRHO,PTH,P)
      DIMENSION P(9,9)
      P(1,1)=DBLINT(Y2,X3,Y3,0,0)
      P(2,1)=DBLINT(Y2,X3,Y3,1,0)
      P(2,2)=DBLINT(Y2,X3,Y3,2,0)
      P(3,1)=DBLINT(Y2,X3,Y3,0,1)
      P(3,2)=DBLINT(Y2,X3,Y3,1,1)
      P(3,3)=DBLINT(Y2,X3,Y3,0,2)
      P(4,1)=P(2,2)
      P(4,2)=DBLINT(Y2,X3,Y3,3,0)
      P(4,3)=DBLINT(Y2,X3,Y3,2,1)
      P(4,4)=DBLINT(Y2,X3,Y3,4,0)
      P(5,1)=P(3,2)
      P(5,2)=P(4,3)
      P(5,3)=DBLINT(Y2,X3,Y3,1,2)
      P(5,4)=DBLINT(Y2,X3,Y3,3,1)
      P(5,5)=DBLINT(Y2,X3,Y3,2,2)
      P(6,1)=P(3,3)
      P(6,2)=P(5,3)
      P(6,3)=DBLINT(Y2,X3,Y3,0,3)
      P(6,4)=P(5,4)
      P(6,5)=DBLINT(Y2,X3,Y3,1,3)
      P(6,6)=DBLINT(Y2,X3,Y3,0,4)
      P(7,1)=P(4,2)
      P(7,2)=P(4,4)
      P(7,3)=P(5,4)
      P(7,4)=DBLINT(Y2,X3,Y3,5,0)
      P(7,5)=DBLINT(Y2,X3,Y3,4,1)
      P(7,6)=DBLINT(Y2,X3,Y3,3,2)
      P(7,7)=DBLINT(Y2,X3,Y3,6,0)
      P(8,1)=P(5,3)+P(4,3)
      P(8,2)=P(6,4)+P(5,4)
      P(8,3)=P(6,5)+P(5,5)
      P(8,4)=P(7,6)+P(7,5)
      P(8,5)=DBLINT(Y2,X3,Y3,2,3)+P(7,6)
      P(8,6)=DBLINT(Y2,X3,Y3,1,4)+DBLINT(Y2,X3,Y3,2,1)
      P(8,7)=DBLINT(Y2,X3,Y3,4,2)+DBLINT(Y2,X3,Y3,3,1)
      P(8,8)=DBLINT(Y2,X3,Y3,2,4)+DBLINT(Y2,X3,Y3,4,2)
      P(9,1)=P(6,3)
      P(9,2)=P(6,5)
      P(9,3)=P(6,6)
      P(9,4)=DBLINT(Y2,X3,Y3,2,3)
      P(9,5)=DBLINT(Y2,X3,Y3,1,4)

```

```

P(0,6)=DBLINT(Y2,X3,Y3,0,2)
P(0,7)=DBLINT(Y2,X3,Y3,3,3)
P(0,8)=DBLINT(Y2,X3,Y3,1,5)+DBLINT(Y2,X3,Y3,2,4)
P(0,9)=DBLINT(Y2,X3,Y3,0,6)
DO 10 I=1,9
DO 10 J=1,I
10 P(I,J)=P(I,J)+PRHU*PTH
DO 20 I=2,9
N=I-1
DO 20 J=1,N
P(J,I)=P(I,J)
20 CONTINUE
RETURN
END

```

```

S      FORTRAN DECK
CEIGFM      REDUCES STIFFNESS MATRIX AND INVERTS IT, REDUCES MASS MATRIX
C           DETERMINES EIGENVALUES AND EIGENVECTORS
C           THE ARGUMENTS ARE=
C           A - VECTOR OF LENGTH NRDF*(NRDF+1)/2
C           VALU - VECTOR OF LENGTH NEIG
C           TEMP,B,C,DUM3, - VECTORS OF LENGTH NRDF OR NMASS (SMALLER)
C           E - MATRIX OF DIMENSION (NRDF,3)
C           IDUM4 - VECTOR OF LENGTH NRDF OR NMASS (SMALLER)
C           ITAPE,JTAPE, NTAPE, MTAPE, - THESE ARE VARIOUS TAPES
C           NRDF - NUMBER OF DEGREES OF FREEDOM OF THE SYSTEM
C           NEIG - NUMBER OF EIGENVALUES DESIRED
C           NVEC - NUMBER OF EIGENVECTORS DESIRED
C           NMASS=NO. OF NORMAL DISPLACEMENTS
C           NOMASS=NO. OF ROTATIONAL DEGREES OF FREEDOM
C           STIFF IS ON MTAPE IN COMPACT FORM
C           MASS IS ON NTAPE IN COMPACT FORM
C           SUBROUTINE EIGEN(A,VALU,TEMP,B,C,DUM3,E, IDUM4, ITAPE, JTAPE, KTAPE,
1NTAPE,MTAPE,NRDF,NEIG,NVEC,NMASS,NOMASS)
C           DIMENSION DUM3(NRDF), IDUM4(1),A(1),VALU(1),B(1),C(1),E(NRDF,3),
1TEMP(1)
C           DIMENSION ILOW(50), IHIGH(50)
C           INTEGER OUT
C           DATA Q1/6HFLEXIB/,Q2/6HILITY /,Q3/6HMATRIX/
C           DATA Q4/6HWEIGHT/,Q5/6H MTRI/,Q6/6HX /
C           DO 56 I=1,NMASS
C           ILOW(I)=1
56      IHIGH(I)=NMASS
C           OUT=6
C           REWIND MTAPE
C           REWIND NTAPE
C           NTEMP=NMASS
C           CALL DIVID(NMASS,NOMASS,MTAPE,JTAPE, ITAPE,A,B)
C           CALL ZROMAK(A,B,C,DUM3,NMASS,NOMASS, ITAPE, JTAPE, MTAPE, KTAPE)
C           CALL DIVID(NMASS,NOMASS,NTAPE,JTAPE, ITAPE,A,B)
C           CALL ZROMAM(A,B,C,DUM3,NMASS,NOMASS, ITAPE, JTAPE, NTAPE, KTAPE)
345      CONTINUE
C           REWIND MTAPE
C           REWIND NTAPE
C           NREDU=NMASS
C           NRMX=NREDU*(NREDU+1)/2
C           READ(MTAPE) (A(I),I=1,NRMX)
C           WRITE(OUT,5500)
5500      FORMAT(//85HR E D U C E D   U P P E R   T R I A N G U L A R
1S T I F F N E S S   M A T R I X)
C           DO 5501 I=1,NREDU
C           NS=(2*I+(I-1)*(2*NREDU-1))/2
C           NE=(2*NREDU+(I-1)*(2*NREDU-1))/2
C           WRITE(OUT,5502) I,(A(J),J=NS,NE)
5502      FORMAT(/3HROW,14,8(/9E14.5))
5501      CONTINUE
C           CALL SYMINV(A,NREDU)
C           WRITE(OUT,5503)
5503      FORMAT(//89HR E D U C E D   U P P E R   T R I A N G U L A R
1F L E X I B I L I T Y   M A T R I X)
C           PUNCH 5602, ((ILOW(K),IHIGH(K)),K=1,NREDU)
5602      FORMAT (18I4)
C           DO 5504 I=1,NREDU
C           NS=(2*I+(I-1)*(2*NREDU-1))/2
C           NE=(2*NREDU+(I-1)*(2*NREDU-1))/2
5504      WRITE(OUT,5502) I,(A(J),J=NS,NE)

```

```

PUNCH 6011, 01, 02, 03
6011 FORMAT(3A6)
DO 5507 I=1, NREDU
  II=I-1
  IF(II.EQ.0) GO TO 5508
  DO 5509 J=1, II
  NU=(2*I+(J-1)*(2*I-J))/2+(J-1)*(NREDU-I)
5509 R(J)=A(NU)
5508 CONTINUE
  NS=(2*I+(I-1)*(2*NREDU-I))/2
  NE=(2*NREDU+(I-1)*(2*NREDU-I))/2
  J=I
  DO 5510 JJ=NS, NE
  R(J)=A(JJ)
5510 J=J+1
  PUNCH 6010, (R(J), J=1, NREDU)
6010 FORMAT(1P6E12.5)
5507 CONTINUE
C   OPTION TO EXPAND REDUCED FLEXIBILITY MATRIX TO FULL MATRIX BY
C   INSERTING 1 OR 2 ZERO ROWS AND COLUMNS REPRESENTING ATTACH POINTS.
C   CODE , NCOD = 1 OPTION EXECUTED , NCOD = 0 OPTION NOT EXECUTED
  READ(5, 560) NCOD
  560 FORMAT (I6)
  IF(NCOD) 580, 580, 570
  570 CALL FULFL (A, NREDU)
  580 READ(NTAPE) (A(I), I=1, NRMX)
  DO 6012 I=1, NRMX
6012 A(I)=A(I)*32.174*12.
  WRITE(OUT, 5505)
5505 FORMAT(///79H R E D U C E D   U P P E R   T R I A N G U L A R
1 W F I G H T   M A T R I X)
  DO 5506 I=1, NREDU
  NS=(2*I+(I-1)*(2*NREDU-I))/2
  NE=(2*NREDU+(I-1)*(2*NREDU-I))/2
5506 WRITE(OUT, 5502) I, (A(J), J=NS, NE)
  PUNCH 6011, 04, 05, 06
  DO 5511 I=1, NREDU
  II=I
  IF(II.EQ.0) GO TO 5512
  DO 5513 J=1, II
  NU=(2*I+(J-1)*(2*I-J))/2+(J-1)*(NREDU-I)
5513 R(J)=A(NU)
5512 CONTINUE
  NS=(2*I+(I-1)*(2*NREDU-I))/2
  NE=(2*NREDU+(I-1)*(2*NREDU-I))/2
  J=I
  DO 5514 JJ=NS, NE
  R(J)=A(JJ)
5514 J=J+1
  PUNCH 6010, (R(J), J=1, NREDU)
5511 CONTINUE
  IF(NEIG.EQ.0) RETURN
  NMAX=NTEMP*(NTEMP+1)/2
  30 CONTINUE
C   READ IN THE MASS MATRIX
  REWIND NTAPE
  READ(NTAPE) (A(I), I=1, NRMX)
  REWIND NTAPE
  355 CONTINUE
  CALL EIGMAT(NTEMP, A, VALU, TEMP, R, C, DUM3, E, IDUM4, NTAPE, NTAPE, JTAP,
1 ITAPE, NEIG, NVEC)

```



```
100 CONTINUE
DO 60 I=1,NEIG
DUM3(I)=SORT(VALU(I))/6.2831853
60 CONTINUE
WRITE(OUT,9009)
: WRITE(OUT,9005) (I,DUM3(I),I=1,NEIG)
9009 FORMAT(1H1,43X.33WHERE ARE THE NATURAL FREQUENCIES ///)
9005 FORMAT(35X,29HTHE NATURAL FREQUENCY NUMBER 13,2X,2HIS 12.3,2X,
13HCPS)
9008 FORMAT(1H1,38X.43WHERE ARE THE EIGENVALUES AND EIGENVECTORS ///)
RETURN
END
```

S	FORTRAN DECK	1
C	CFULFL EXPANDS REDUCED FLEXIBILITY MATRIX BY INSERTING 1 OR 2 ZERO	2
C	ROWS AND COLUMNS REPRESENTING ATTACH POINTS.	3
C	THE ARGUMENTS ARE	4
C	B(I) = REDUCED FLEXIBILITY MATRIX IN COMPACT FORM	5
C	NXC = ORDER OF REDUCED FLEX. MATRIX	6
C	INPUT DATA REQUIRED	7
C	NR = NO. OF ATTACH POINTS (1 OR 2)	8
C	NNE,NWO = MASS NUMBERS OF ATTACH POINTS 1 AND 2 RESP.	9
C		10
	SUBROUTINE FULFL(R,NXC)	15
	DIMENSION B(1),D(1275),C(50)	20
	DATA 07/6HEXPAND/,08/6HED FLE/,09/6HXIBILI/,010/6HTY MAT/,011/6HRI	25
	1X /	30
	READ(5,1)NR,NNE,NWO	35
	1 FORMAT (9I8)	40
	MS=NXC+NR	45
	MMS=MS*(MS+1)/2	50
	DO 50 I=1,MMS	55
50	D(I)=0.0	60
	JJJ=0	65
	KK=0	70
	JJ=0	75
	DO 100 J=1,MS	80
	IF(J.EQ.NNE.OR.J.EQ.NWO)GO TO 99	85
	I=JJ+1	90
	JJ=I+NXC-J+JJJ	95
	KKK=J-1	100
	DO 98 JK=I,JJ	105
	KKK=KKK+1	110
	KK=KK+1	115
	IF(KKK.EQ.NNE.OR.KKK.EQ.NWO)GO TO 96	120
	GO TO 97	125
96	KK=KK+1	130
97	D(KK)=B(JK)	135
98	CONTINUE	140
	GO TO 100	145
99	KK=KK+MS-J+1	150
	JJJ=JJJ+1	155
100	CONTINUE	160
	WRITE(6,2)	165
	2 FORMAT(//86HU P P E R T R I A N G L E - E X P A N D E D F L	170
	1 E X I B I L I T Y M A T R I X)	175
	DO 10 I=1,MS	180
	NS=(2*I+(I-1)*(2*MS-I))/2	185
	NE=(2*MS+(I-1)*(2*MS-I))/2	190
	WRITE(6,3)I,(D(J),J=NS,NE)	195
	3 FORMAT(/3HROW,14/(9F14.5))	200
10	CONTINUE	205
	PUNCH 4,07,08,09,010,011	210
	4 FORMAT(5A6)	215
	DO 20 I=1,MS	220
	II=I-1	225
	IF(II.EQ.0) GO TO 18	230
	DO 19 J=1,II	235
	NU=(2*I+(J-1)*(2*I-J))/2+(J-1)*(MS-I)	240
19	C(J)=D(NU)	245
18	CONTINUE	250
	NS=(2*I+(I-1)*(2*MS-I))/2	255
	NE=(2*MS+(I-1)*(2*MS-I))/2	260
	J=I	

```
DO 16 JJ=NS,NE  
C(J)=D(JJ)  
16 J=J+1  
PUNCH 5,(C(J),J=1,MS)  
5 FORMAT(1P6E12.5)  
20 CONTINUE  
RETURN  
END
```

266
27
275
280
27
200
295
300

```

9      FORTRAN DECK
C      CDIVID
C      N=NO. OF NORMAL DISPLACEMENTS
C      M=NO. OF ROTATIONAL D.O.F.
C      NTPE-CONTAINS STIFFNESS (OR MASS) MATRIX
C      K1PE-K12 (M2) STORED
C      K1PF-K13 (M1) STORED
C      A- DUMMY STORAGE VECTOR, LARGER OF (N*(N+1)/2 OR M*(M+1)/2)
SUBROUTINE DIVID (N,M,NTPE,MTPE,ITPE,A,B)
DIMENSION A(1),B(1)
REWIND ITPE
REWIND NTPE
REWIND MTPE
NMAX=N*(N+1)/2
MMAX=M*(M+1)/2
NM=N+M
ICNT=0
DO 10 J=1,N
  II=NM-I+1
  READ(NTPE) (R(J),J=1,II)
  ID=II-M
  DO 20 J=1,ID
    ICNT=ICNT+1
20    A(ICNT)=B(J)
    ID1=ID+1
    JCNT=0
    DO 30 J=ID1,II
      JCNT=JCNT+1
30    R(JCNT)=B(J)
  WRITE(MTPE) (R(J),J=1,M)
10  CONTINUE
  WRITE(ITPE) (A(J),J=1,NMAX)
  REWIND MTPE
  REWIND ITPE
  ID=0
  ICNT=0
  DO 50 I=1,M
    II=M-ICNT
    READ(NTPE) (R(J),J=1,II)
    ICNT=ICNT+1
    DO 60 J=1,II
      ID=ID+1
60    A(ID)=R(J)
50  CONTINUE
  RETURN
END

```

```

$      FORTRAN DECK
CZROMAK
C      D IS A DUMMY VECTOR WITH STORAGE N OR M (LARGER)
C      A IS A DUMMY VECTOR WITH STORAGE N*(N+1)/2 OR M*(M+1)/2 (LARGER)
C      B IS A DUMMY VECTOR WITH STORAGE N OR M (LARGER)
C      C IS A DUMMY VECTOR WITH STORAGE N OR M (LARGER)
C      N=NO. OF NORMAL DISPLACEMENTS
C      M=NO. OF ROTATIONAL D.O.F.
C      N1PE CONTAINS K1 MATRIX
C      M1PE CONTAINS K1/2 MATRIX
C      I1PE SCRATCH TAPE
C      K1PE STORES K1/2*K1/2*(-1)
C      A INITIALLY CONTAINS K^2 INVERSE
C***  REDUCED STIFFNESS MATRIX IS STORED ON I1PE
      SUBROUTINE ZROMAK(A,B,C,D,N,M,N1PE,M1PE,I1PE,K1PE)
      DIMENSION A(1),B(1),C(1),D(1)
      DOUBLE PRECISION SUM,DP1,DP2
      CALL SYMINV(A,M)
      REWIND M1PE
      REWIND I1PE
      REWIND N1PE
      REWIND K1PE
      NMAX=N*(N+1)/2
      MMAX=M*(M+1)/2
      DO 10 KK=1,N
      READ(M1PE) (B(I),I=1,M)
      ICNT=0
      DO 1000 IK=1,M
      JJ=IK
      JK=IK
      DO 20 J=JJ,M
      ICNT=ICNT+1
20      C(J)=A(ICNT)
      IJ=JJ-1
      JA=M
      ID=IK
      DO 30 J=1,JJ
      IF(JJ.EQ.0) GO TO 30
      C(J)=A(ID)
      JA=JA-1
      ID=ID+JA
30      CONTINUE
      SUM=0.000
      DO 40 J=1,M
      DP1=B(J)
      DP2=C(J)
40      SUM=SUM+DP1*DP2
      D(JK)=SUM
1000  CONTINUE
      WRITE (I1PE) (D(J),J=1,M)
      WRITE (K1PE) (D(J),J=1,M)
10      CONTINUE
      REWIND I1PE

```

```

REWIND MTPF
REWIND NTPF
REWIND KTPF
READ (NTPF) (A(J),J=1,NMAX)
ICNT=0
DO 60 KK=1,N
READ (ITPF) (D(J),J=1,M)
KI=KK
DO 70 KJ=1,N
READ(MTPF)(C(J),J=1,M)
KP=KJ
IF(KP.LT.KI) GO TO 70
SUM=0.000
DO 60 KR=1,M
DP1=D(KR)
DP2=C(KR)
60 SUM=SUM +DP1 *DP2
ICNT=ICNT+
SM=SUM
A(ICNT)=A(ICNT)-SM
70 CONTINUE
REWIND MTPF
60 CONTINUE
REWIND NTPF
REWIND MTPF
REWIND ITPF
WRITE(ITPF) (A(I),I=1,NMAX)
REWIND ITPF
RETURN
END

```

```

C      FORTRAN DECK
CZROMAM
C      N=NO. OF NORMAL DISPLACEMENTS
C      M=NO. OF ROTATIONAL D.O.F.
C      N1PE CONTAINS M11 MATRIX
C      M1PE CONTAINS M12 MATRIX
C      I1PE SCRATCH TAPE
C      K1PE CONTAINS K12*K22**(-1)
C***  REDUCED MASS MATRIX IS STORED ON I1PE
      SUBROUTINE ZROMAM(A,B,C,D,N,M,N1PE,M1PE,I1PE,K1PE)
      DIMENSION A(1),B(1),C(1),D(1)
      DOUBLE PRECISION SUM1,SUM2,DP1,DP2,DP3
      NMAX=N*(N+1)/2
      REWIND M1PE
      REWIND I1PE
      REWIND N1PE
      REWIND K1PE
      NMAX=N*(N+1)/2
      DO 10 KK=1,N
      READ(K1PE) (R(I),I=1,M)
      ICNT=0
      DO 1000 IK=1,M
      JJ=IK
      JK=IK
      DO 20 J=JJ,M
      ICNT=ICNT+1
20  C(J)=A(ICNT)
      JJ=JJ-1
      JA=M
      ID=IK
      DO 30 J=1,JJ
      IF(JJ.EQ.0) GO TO 30
      C(J)=A(ID)
      JA=JA-1
      ID=ID+JA
30  CONTINUE
      SUM1=1.D0
      DO 50 J=1,M
      DP1=B(J)
      DP2=C(J)
50  SUM1=SUM1+DP1*DP2
      D(JK)=SUM1
1000 CONTINUE
      WRITE(I1PE) (D(J),J=1,M)
      CONTINUE
      REWIND I1PE
      REWIND M1PE
      REWIND N1PE
      REWIND K1PE
      READ(N1PE) (A(J),J=1,NMAX)
      DO 60 KK=1,N
      READ(M1PE) (R(J),J=1,M)

```

```

READ(ITPE) (O(J),J=1,N)
DO /O KJ=1,N
READ(KTPE) (C(J),J=1,N)
SUM1=0.D0
SUM2=0.D0
DO /O KR=1,N
DP1=B(KR)
DP2=D(KR)
DP3=C(KR)
SUM1=SUM1+DP1*DP3
SUM2=SUM2+DP2*DP3
SM1=SUM1
SM2=SUM2
IF (KJ.GE.KK) MM=(2*KJ+(KK-1)*(2*NMASS-KK))/2
IF (KJ.GE.KK) A(MM)=A(MM)-SM1+SM2
IF (KJ.LE.KK) MM=(2*KK+(KJ-1)*(2*NMASS-KJ))/2
IF (KJ.LE.KK) A(MM)=A(MM)-SM1
/O CONTINUE
REWIND KTPE
/O CONTINUE
REWIND NTP-
REWIND MTP-
REWIND ITPE
REWIND KTPE
WRITE(ITPE) (A(I),I=1,NMAX)
REWIND ITPE-
RETURN
END

```


4 FORTRAN DECK

CSYMINV

C A IS THE UPPER TRIANGLE OF THE SYMMETRIC MATRIX TO BE INVERTED. S

C ELEMENTS ARE STORED ROWWISE. S

C N = ORDER OF MATRIX S

C PROGRAM INVERTS IN PLACE. S

 SURROUTINE SYMINV(A,N)

 DIMENSION A(1)

 NMAX=N*(N+1)/2

 A(1)=SQRT(A(1))

 DO 100 IJ=2,N

100 A(IJ)=A(IJ)/A(1)

 A(1)=1.0/A(1)

 IM1=1

 IJ=N

 DO 1000 I=1,N

 II=IJ+1

 IJ=II

 DO 200 J=I,N

 JMI=J-I

 LI=I

 LJ=J

 DO 120 L=1,IM1

 A(IJ)=A(IJ)-A(LI)*A(LJ)

 LI=LI+N-L

120 LJ=LJ+JMI

200 IJ=IJ+1

 A(II)=SQRT(A(II))

 JI=I

 JJ=1

 DO 500 J=1,IM1

 A(JI)=A(JJ)*A(JI)

 IF(J-IM1)500,420,420

300 JP1=J+1

 JL=JJ

 LI=JI

 DO 400 L=JP1,IM1

 JL=JL+1

 LI=LI+N-L+1

400 A(JI)=A(JI)+A(JL)*A(LI)

420 A(JI)=-A(JI)/A(II)

 JI=JI+N-J

500 JJ=JJ+N-J+1

 IF(I-N)600,900,900

600 IP1=I+1

 IJ=II

 DO 700 J=IP1,N

 IJ=IJ+1

700 A(IJ)=A(IJ)/A(II)

900 A(II)=1.0/A(II)

1000 IM1=1

 II=1

```
DO 2000 I=1,N
  JJ=II
  IJ=II
  DO 1400 J=1,N
    A(IJ)=A(IJ)*A(JJ)
    JP1=J+1
    IF (JP1-N)1100,1100,1400
1100  IL=IJ
     JL=JJ
     DO 1200 L=JP1,N
       IL=IL+1
       JL=JL+1
1200  A(IJ)=A(IJ)+A(IL)*A(JL)
       JJ=JL+1
1400  IJ=IJ+1
2000  II=IJ
  RETURN
END
```

SSSSSSSSSSSSSSSS

```

$      FORTRAN DECK
C EIGMAT
C      THIS SUBROUTINE FINDS THE EIGENVALUES AND EIGENVECTORS FOR
C      SYMMETRIC MASS AND STIFFNESS MATRICES.
C      THE ARGUMENTS ARE--
C      N- ORDER OF MATRICES.
C      A- DUMMY VECTOR WITH DIMENSION IN MAIN PROGRAM OF  $N*(N+1)/2$ 
C      VALU- STORAGE FOR EIGENVALUES. MUST BE DIMENSIONED IN THE MAIN
C            PROGRAM AS A VECTOR OF LENGTH NEIG.
C      TEMP,B,C,D.- DUMMY VECTORS WITH DIMENSION OF N IN MAIN PROGRAM.
C      F- DUMMY ARRAY WITH DIMENSIONS OF (N,3) IN MAIN PROGRAM.
C      IDUM- DUMMY INTEGER VECTOR WITH DIMENSION OF N IN MAIN PROGRAM.
C      MTAPE- TAPE WHERE STIFFNESS MATRIX IS STORED IN COMPACT FORM.
C      NTAPE- TAPE WHERE MASS MATRIX IS STORED IN COMPACT FORM.
C      JTAPE,ITAPE- SCRATCH TAPES.
C      NEIG- NUMBER OF EIGENVALUES DESIRED.
C      NVEC- NUMBER OF EIGENVECTORS DESIRED. MUST BE EQUAL TO OR LESS
C            THAN NEIG.
C      THE MASS AND STIFFNESS MATRICES ARE STORED IN COMPACT FORM AS
C      VECTORS. ONLY THE UPPER TRIANGLE OF THESE MATRICES(BY ROWS) IS
C      STORED.
C      SUBROUTINE EIGMAT(N,A,VALU,TEMP,B,C,D,F,IDUM,MTAPE,NTAPE,JTAPE,
C      ITAPE,NEIG,NVEC)
C      DIMENSION A(1),TEMP(1),VALU(1),B(1),C(1),D(1),F(N,3),IDUM(1)
C      DOUBLE PRECISION SUM,SUM1
C      INTEGER OUT
C      OUT=6
C      REWIND ITAPE
C      REWIND JTAPE
C      REWIND NTAPE
C      REWIND MTAPE
C      M=?*N
C      NMAX=N*(N+1)/2
C * * * * *
C      STEP 1
C      READ IN M BY ROWS IN COMPACTED FORM
C      REPLACE M BY (1)TRANSPOSE, WHERE M=L*(L)TRANSPOSE
C      CALCULATE FIRST ROW
C      READ (NTAPE) (A(I),I=L,NMAX)
C      REWIND NTAPE
C      CONTINUE
C      A(1)=SORT(A(1))
C      DO 10 I=2,N
C      A(I)=A(I)/A(1)
C      CALCULATE ALL THE OTHER ROWS
C      IND=N
C      DO 20 J=1,N
C      IND=IND+1
C      SUM=0.00
C      KI=J-1
C      DO 30 JJ=L,KI
C      MJ=(M-JJ)*(JJ-1)/2+I

```

```

20 SUM=SUM+A(MJ)*A(MJ)
A(IND)=DSQRT(A(IND)-SUM)
IF(IND.EQ.NMAX) GO TO 100
SUM1=A(IND)
K1=I+1
DO 99 J=K1.N
IND=IND+1
SUM=0.00
I1=I-1
DO 60 JJ=1.I1
K=(M-JJ)*(JJ-1)/2
KI=K+1
KJ=K+J
60 SUM=SUM+A(KI)*A(KJ)
A(IND)=(A(IND)-SUM)/SUM1
99 CONTINUE
100 CONTINUE
101 CONTINUE
C CHECK FOR SINGULAR MASS MATRIX
DO 102 I=1.N
KI=(M-I)*(I-1)/2+1
IF(A(KI).EQ.0.) GO TO 1090
102 CONTINUE
C THIS COMPLETES STEP 1
C * * * * *
C STEP 2
C WRITE (L)TRANSPOSE ON TAPE BY COLUMNS
C PUT (L)TRANSPOSE INTO TEMPORARY STORAGE (TEMP--A VECTOR)
C AND THEN WRITE TEMP ON TAPE
KTAPF=NTAPF
310 IND=0
DO 340 J=L.N
DO 330 I=1.J
IND=IND+1
M11=(M-I)*(I-1)/2+J
TEMP(IND)=A(M11)
330 CONTINUE
WRITE(KTAPF) (TEMP(JJ),JJ=1,IND)
IND=0
340 CONTINUE
C THIS COMPLETES STEP 2
C * * * * *
C STEP 3
C ((L)TRANSPOSE) INVERSE REPLACES (L)TRANSPOSE IN CORE
C REPLACEMENT IS DONE BY LAST COLUMN FIRST--WORKING UP THE COLUMN
DO 410 I=1.N
IND=(I*(M+1-1))/2-N
410 A(IND)=1./A(IND)
DO 499 J=2.N
IJ=(N+2)-J
DO 490 I=2.JJ
IND=(N+J+1-3)*(JJ-1)/2
SUM=0.00

```

```

      K1=JJ-1+2
      DO 450 K=K1,JJ
      IDK=IND+K
      MK=(M-K)*(K-1)/2+JJ
450  SUM=SUM+A(IDK)*A(MK)
      IND=IND+JJ
      IDI=IND-I+1
440  A(IND)=-SUM+A(IDI)
440  CONTINUE
C     END OF STEP 3
C* * * * *
C     STEP 4
C     U=((L)TRANSPPOSE)INVERSE
C     WRITE U ON TAPE BY ROWS
C     WRITE(ITAPE) (A(I),I=L,NMAX)
C     FINISHED WITH STEP 4
C* * * * *
C     STEP 5
C     WRITE U ON TAPE BY COLUMNS STARTING WITH THE LAST COLUMN FIRST
C     PUT U (LAST COLUMN FIRST) INTO TEMP AND THEN WRITE ON TAPE
      IND=0
      DO 550 K=L,N
      J=N-K+1
      DO 550 I=1,J
      IND=IND+1
      M12=(M-I)*(I-1)/2+J
      TEMP(IND)=A(M12)
550  CONTINUE
      WRITE(JTAPE) (TEMP(JJ),JJ=L,IND)
      IND=0
555  CONTINUE
C     END OF STEP 5
C* * * * *
C     STEP 6
C     FORM KU
C     READ K INTO CORF
C     READ U INTO CORF A COLUMN AT A TIME IN REVERSE ORDER
C     REPLACE K BY KU COLUMN BY COLUMN STARTING WITH THE LAST COLUMN
C     AND WORKING UP THE COLUMN
      READ(MTAPE) (A(I),I=1,NMAX)
      REWIND JTAPE
      DO 690 JJ=L,N
      J=N+1-JJ
      READ(JTAPE) (TEMP(I),I=L,J)
      DO 690 II=1,J
      I=J+1-II
      SUM=0.D0
      DO 650 K=1,I
      MK1=(M-K)*(K-1)/2+J
650  SUM=SUM+A(MK1)*TEMP(K)
      IND=(M-I)*(I-1)/2+J
      IF(I.EQ.J) GO TO 680

```

```

KI=(M-I)*(I-1)/2
I=I+1
DO 660 K=I,J
KIK=K1+K
660 SUM=SUM+A(KIK)*TEMP(K)
680 CONTINUE
A(IND)=SUM
690 CONTINUE
C END OF STEP 6
C * * * * *
C STEP 7
C FORM((L)INVERSE)*KU
C KU IS IN CORE
C READ IN L COLUMN BY COLUMN AND CALCULATE ((L)INVERSE)*KU
C ROW BY ROW
C CALCULATE THE FIRST ROW
REWIND NTAPE
READ(NTAPE) TEMP(L)
DO 710 I=L,N
710 A(I)=A(I)/TEMP(L)
C NOW CALCULATE THE REST OF THE ROWS
IND=N
DO 799 I=2,N
READ(NTAPE) (TEMP(JJ),JJ=1,I)
DO 799 J=1,N
IND=IND+1
JJ=I-1
SUM=0.D0
DO 750 K=1,JJ
MK2=(M-K)*(K-1)/2+J
750 SUM=SUM+TEMP(K)*A(MK2)
790 A(IND)=(A(IND)-SUM)/TEMP(I)
C STEP 7 IS COMPLETE
C * * * * *
C STEP 8
C DETERMINE EIGENVALUES AND EIGENVECTORS OF THE NEW MATRIX
C CHANGE THE SIGN OF A IN ORDER TO OBTAIN THE SMALLEST
C EIGENVALUE FIRST
DO 800 I=1,NMAX
800 A(I)=-A(I)
CALL HIGMAT(A,VALU,TEMP,8,C,D,E,IDUM,N,NEIG,NVEC,MTAPE)
C CHANGE VALU BACK
DO 850 I=1,NFIG
850 VALU(I)=-VALU(I)
C STEP 8 IS COMPLETE
C * * * * *
C STEP 9
C CHANGE EIGENVECTORS BACK
C READ U INTO CORE BY ROWS
C READ UNCHANGED EIGENVECTORS INTO CORE ONE AT A TIME
C CHANGE AND PRINT EIGENVECTORS
IF(NVEC.EQ.0) GO TO 2000
WRITE(OUT,1001)

```

```

REWIND ITAPE
READ(ITAPE) (A(I), I=1, NMAX)
REWIND HTAPE
DO 909 JJ=1, NVEC
READ(HTAPE) (TEMP(I), I=1, N)
IND=0
DO 910 I=1, N
SUM=0.00
DO 909 J=1, N
IND=IND+1
909 SUM=SUM+A(IND)*TEMP(J)
910 TEMP(I)=SUM
C NORMALIZE THE EIGENVECTOR
SUM=TEMP(I)
DO 939 II=1, N
IF (ABS(SUM)-ABS(TEMP(II))) 938, 939, 939
938 SUM=TEMP(II)
939 CONTINUE
IF (SUM) 940, 947, 940
940 CONTINUE
DO 941 II=1, N
TEMP(II)=TEMP(II)/SUM
941 CONTINUE
947 CONTINUE
999 WRITE(OUT, 999) JJ, VALI(JJ), (TEMP(I), I=1, N)
C STEP 9 IS COMPLETE
C * * * * *
GO TO 2000
4000 FORMAT (1H9, 19H EIGENVECTOR NUMBER 15/12X, 17H CORRESPONDING TO
11PE, 5.7/(1H 1P6E15.7))
4001 FORMAT(1H1.58X, 43HHERE ARE THE EIGENVALUES AND EIGENVECTORS ///)
4002 FORMAT(1H1.58X, 27HTHE MASS MATRIX IS SINGULAR ///)
1090 WRITE(OUT, 999)
2000 RETURN
END

```

9 FORTRAN DECK

CHIGMAT

C PROG. AUTHORS M. ELSON AND R. E. FUNDERLIC. CENTRAL DATA PROCESSING, 4, 1.0. b

 SURROUTINE HIGMAT(A, VALU, VALL, UPPERD, DIAG, V, T, INTER, NN, NEIG, NVFC,

 MTAPE)

 DIMENSION A(1), VALU(1), VALL(1), UPPERD(1), DIAG(1), V(1), T(NN,3),

 INTER(1)

 REWIND MTAPE

 NZ=0

 N=NM

 IF(N.LF.2)GO TO 49

 NP1=N+1

 NM1=N-1

 NM2=N-2

 NI?P1=N*2+1

 IX=L

 DO 10 I=1,NM2

 SIGMA2=0.

 IP1=I+1

 DO 3 J=IP1,N

 IJ=IX+J

1 SIGMA2=SIGMA2+A(IJ)**2

 SIGMA=SQRT(SIGMA2)

 II=IX+I

 DIAG(I)=A(II)

 IIP1=IX+I+1

 UPPERD(I)=-SIGN(SIGMA,A(IIP1))

 T(1,2)=SIGMA2

 IF(ABS(SIGMA).GT.ABS(A(IIP1)))GO TO 2

 UPPERD(I)=A(IIP1)

 A(IIP1)=0.

 GO TO 10

2 A(IIP1)=SQRT(1.+ABS(A(IIP1))/SIGMA)

 SQTGAM=-SIGN(SIGMA*A(IIP1),UPPERD(I))

 IP2=I+2

 DO 5 J=IP2,N

 IJ=IX+J

5 A(IJ)=A(IJ)/SQTGAM

 JK1=I*(2*N-I-1)/2

 JX=JK1

 IIX=JK1

 DO 6 J=IP1,N

 VALL(J)=0.

 JK=JK1+J

 DO 4 K=IP1,J

 IK=IX+K

 VALL(J)=VALL(J)+A(JK)*A(IK)

4 JK=JK+N-K

 IF(J.EQ.N)GO TO 6

 CALL LOUPL(J+2,NP1,VALL(J),A(JX),A(IIX))

6 JX=JX+N-J

7 DELGAM=0.

 DO 7 J=IP1,N


```

      IJ=IX+J
7  DELGAM=DELGAM+A(IJ)*VALL(J)
   DGO2=.5*DELGAM
   DO 6 J=IP1,N
      IJ=IX+J
8  T(I,J)=VALL(J)-DGO2*A(IJ)
   DO 4 II=IP1,N
      III=IX+II
      CALL LOOP2(A(III),A(IX),T(NZ,1),T(II,1),A(III)-II*1,NP1)
9  IIX=IIX+N-11
10 IX=IX+N-1
   M=N*(N+1)/2
   UPPERD(NM1)=A(M-1)
   T(NM1,2)=UPPERD(NM1)**2
   DIAG(NM1)=A(M-2)
   DIAG(N)=A(M)
   ENORM=AMAX1(ABS(DIAG)+ABS(UPPERD),ABS(DIAG(N))+ABS(UPPERD(NM1)))
   DO 11 I=2,NM1
      ENRTHP=ABS(DIAG(I))+ABS(UPPERD(I))+ABS(UPPERD(I-1))
11  IF(ENRTHP.GT.ENORM)ENORM=ENRTHP
   DO 12 I=1,NEIG
      VALU(I)=ENORM
12  VALL(I)=-ENORM
   DO 14 I=1,NEIG
13  ROOT=.5*(VALU(I)+VALL(I))
      IF(ROOT.EQ.VALL(I).OR.ROOT.EQ.VALU(I))GO TO 14
      NAGREE=0
      PM2=0.
      PM1=1.
      DO 21 J=1,N
         IF(PM2.NE.0.)GO TO 15
14  PM1=SIGN(1.,PM1)
      GO TO 17
15  IF(PM1.NE.0.)GO TO 17
16  P=-SIGN(1.,PM2)
      PM2=0.
      IF(T(J-1,2)) 18,14,18
17  P=DIAG(J)-ROOT-T(J-1,2)*PM2/PM1
      PM2=1.
18  IF(P)21,19,20
19  PM2=PM1
      IF(PM2)21,20,20
20  NAGREE=NAGREE+1
21  PM1=P
      DO 23 J=1,NEIG
         IF(J.LE.NAGREE)GO TO 22
         IF(VALU(J).LT.ROOT)GO TO 13
         VALU(J)=ROOT
         GO TO 23
22  VALL(J)=ROOT
23  CONTINUE
      GO TO 13

```

```

24 CONTINUE
   IF (NVEC.EQ.0) GO TO 49
   EPSLON=ENORM*1.E-4
   COMPL1=COMPL(1)
   DO 48 I=1,NVEC
   DO 25 J=1,N
     V(J)=1.
     T(J,2)=DIAG(J)-VALU(I)
     IF (J.EQ.N) GO TO 24
     T(J,3)=UPPFRD(J)
25  T(I+1,1)=UPPFRD(J)
26  T(N,3)=0.
     DO 29 J=1,N
     IF (ABS(T(J,2)).LT.1.E-17) T(J,2)=EPSLON
     T(J,1)=T(J,2)
     T(J,2)=T(J,3)
     T(J,3)=0.
     IF (J.EQ.N) GO TO 30
     INTER(J)=0
     JP1=J+1
     IF (ABS(T(JP1,1)).LE.ABS(T(J,1))) GO TO 28
     INTER(J)=1
     DO 27 K=J,1
     TEMP=T(J,K)
     T(J,K)=T(JP1,K)
27  T(JP1,K)=TEMP
28  TMULTP=T(JP1,1)/T(J,1)
     VALL(J)=OR(INTER(J),AND(TMULTP,COMPL1))
     T(JP1,2)=T(JP1,2)-TMULTP*T(J,2)
29  T(JP1,3)=T(JP1,3)-TMULTP*T(J,3)
30  IIFR=1
31  DO 32 J=1,N
     L=N+1-J
32  V(I)=(V(L)-T(L,2)*V(L+1)-T(L,3)*V(L+2))/T(L,1)
     VNORM=0.
     DO 33 L=1,N
33  VNORM=VNORM+V(L)**2
     VNORM=SQRT(VNORM)
     DO 34 J=1,N
34  V(J)=V(J)/VNORM
     IF (ITER.EQ.2) GO TO 36
     IIFR=2
     DO 35 L=2,N
     LM1=L-1
     TRY=VALL(LM1)
     IF (AND(TRY,1).EQ.0) GO TO 35
     VTEMP=V(LM1)
     V(LM1)=V(L)
     V(L)=VTEMP
35  V(L)=V(L)-VALL(LM1)*V(LM1)
     GO TO 31
36  IF (VNORM.EQ.0.) V(I)=1.
     IIX=(N*N-N-6)/2

```

```
DO 57 KK=1,NM2
  IIP1=N-KK
  IITV=0.
  CALL LOOP3(UTV,A(IIX),V(NZ),IIP1+1,NP1)
  CALL LOOP4(A(IIX),V(NZ),NP1,IIP1+1,UTV)
57 IIX=IIX+IIP1-N-2
  WRITE(NTAPE) (V(ICH),ICH=L,N)
48 CONTINUE
50 RETURN
END
```

9 FORTRAN DECK
CLOOP1

```
SUBROUTINE LOOP1(JP2,NP1,SGAMPJ,AJX,AIX)
  DIMENSION AJX(1), AIX(1)
  DO 1 L=JP2,NP1
1  SGAMPJ=SGAMPJ+AJX(L)*AIX(L)
  RETURN
END
```

H
B
R
H

9 FORTRAN DECK
CLOOP2

```
SUBROUTINE LOOP2(AIIX,AIX,S,SI,AIII,IP1,NP1)
  DIMENSION AIIX(1),AIX(1),S(1)
  DO 2 JJ=IP1,NP1
2  AIIX(JJ)=AIIX(JJ)-AIII*S(JJ)-SI*AIX(JJ)
  RETURN
END
```

H
B
R
H

9 FORTRAN DECK
CLOOP3

```
SUBROUTINE LOOP3(UTV,AIIX,V,IIP2,NP1)
  DIMENSION AIIX(1), V(1)
  DO 3 J=IIP2,NP1
3  UIV=UTV+AIIX(J)*V(J)
  RETURN
END
```

B
B
H
B

9 FORTRAN DECK
CLOOP4

```
SUBROUTINE LOOP4(AIIX,V,NP1,IIP2,UTV)
  DIMENSION AIIX(1),V(1)
  DO 4 K=IIP2,NP1
4  V(K)=V(K)-AIIX(K)*UTV
  RETURN
END
```

B
B
R
B

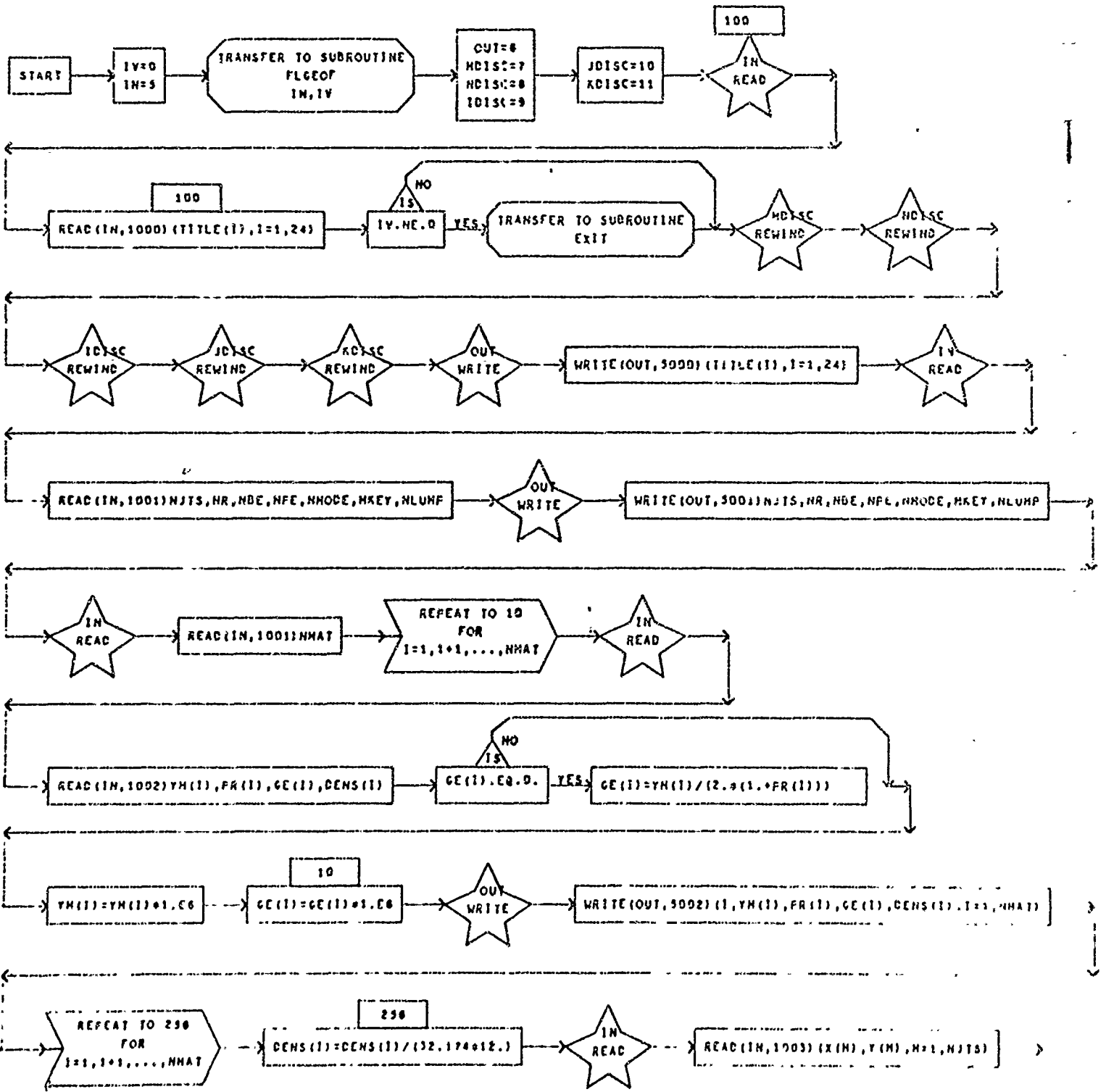
APPENDIX C

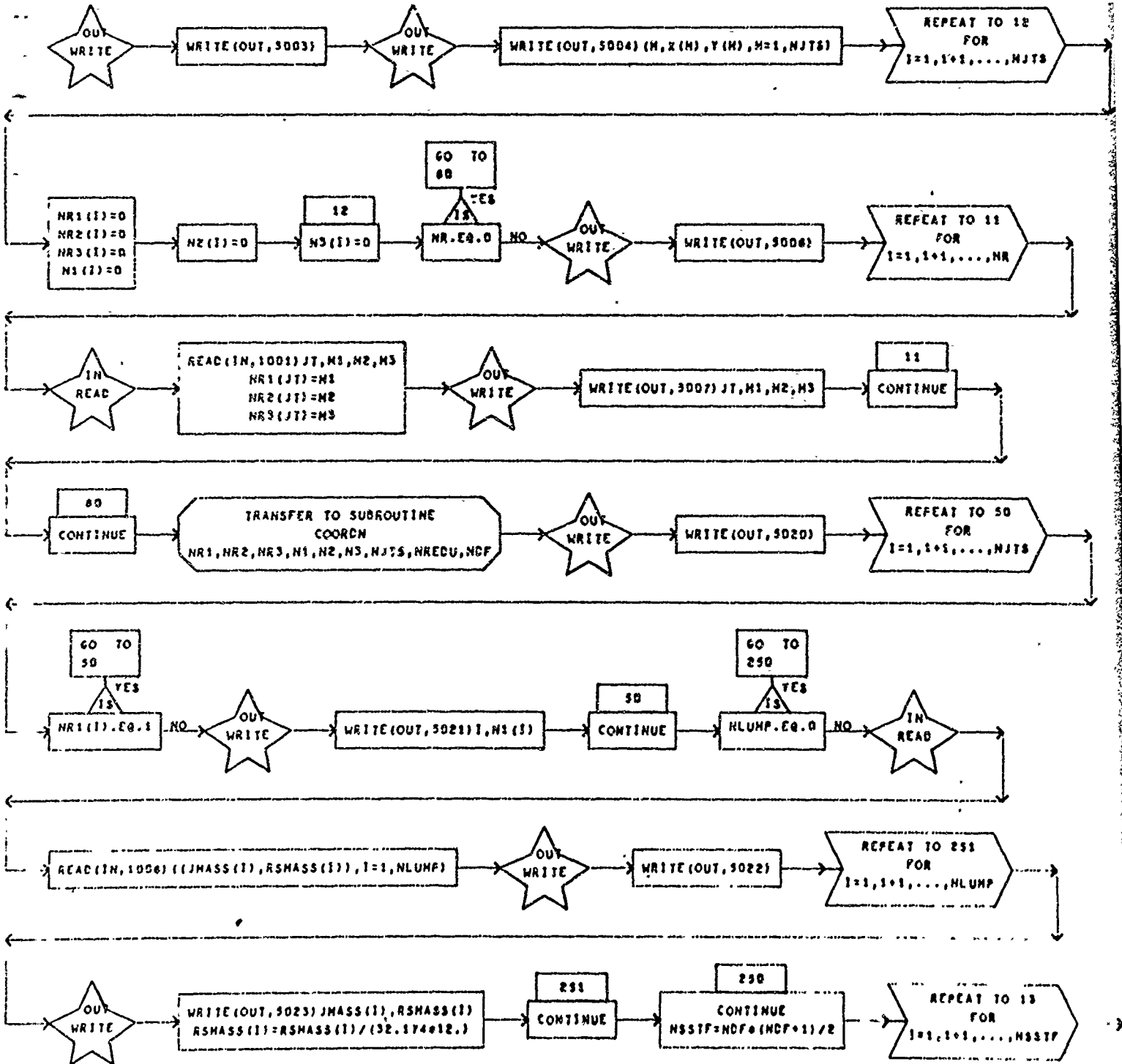
Program FLUENC FLOW CHART

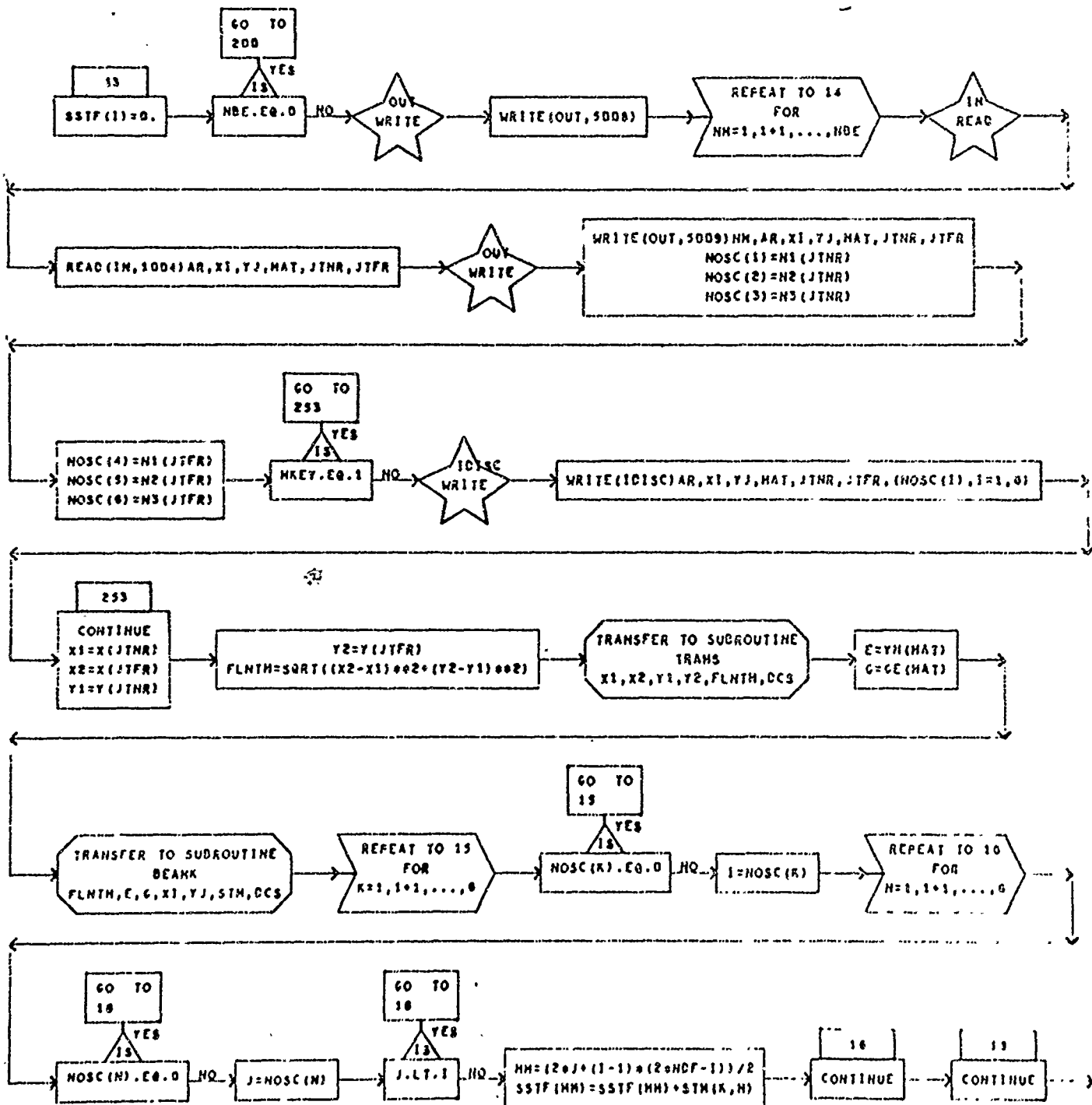
MAIN PROGRAM PLUENC-FOR GENERATING STIFFNESS, FLEXIBILITY AND MASS
 MATRICES FROM PLANE GRID BEAM AND TRIANG. PLATE ELEMENTS

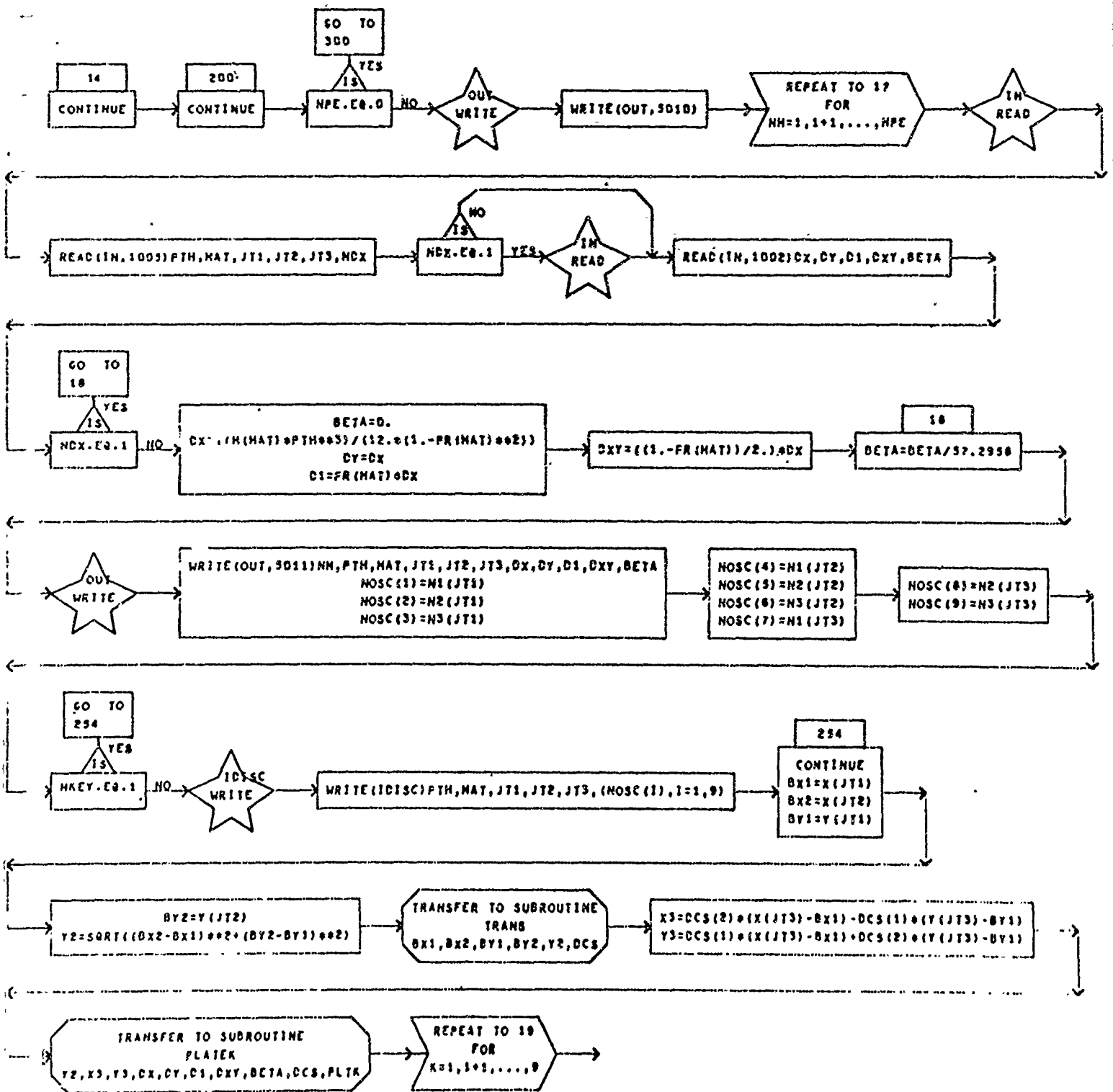
D I M E N S I O N E D V A R I A B L E S

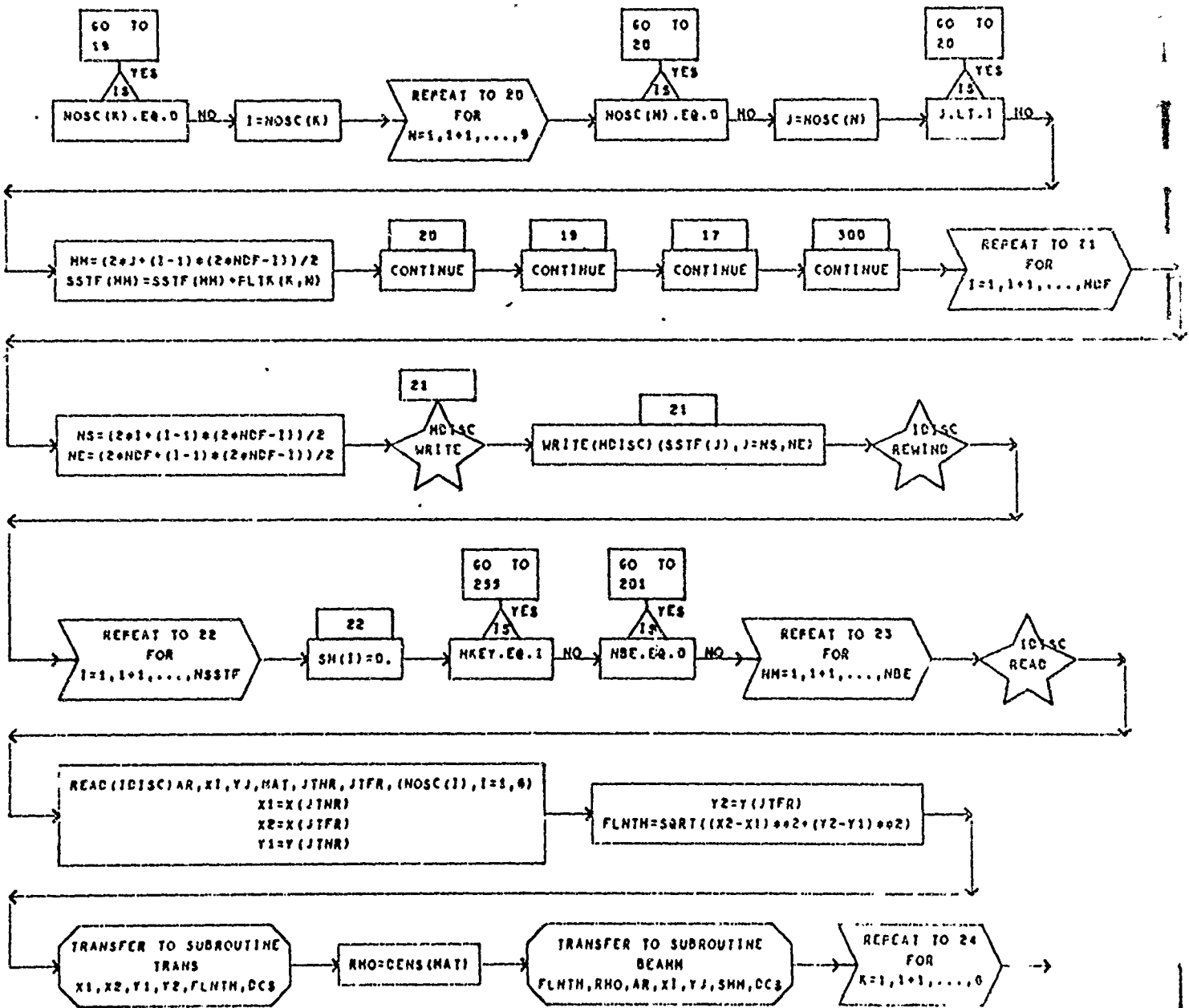
SYMBOL	STORAGES	SYMBOL	STORAGES	SYMBOL	STORAGES	SYMBOL	STORAGES	SYMBOL	STORAGES
TITLE	24	YM	10	PR	10	GE	10	DENS	10
X	50	Y	50	NR1	50	NR2	50	NR3	50
M1	50	NR	50	M3	50	MOBC	9	CCS	2
STM	6,6	SHH	6,6	PLK	9,9	PLYH	9,9	ESTF	11325
SH	11325	RSMAS	50, A (1	25), VA	LU (9	TEMP	50	B	150
C	100	CUK3	150	F	150, 3	ICUM4	50	JMSS	50

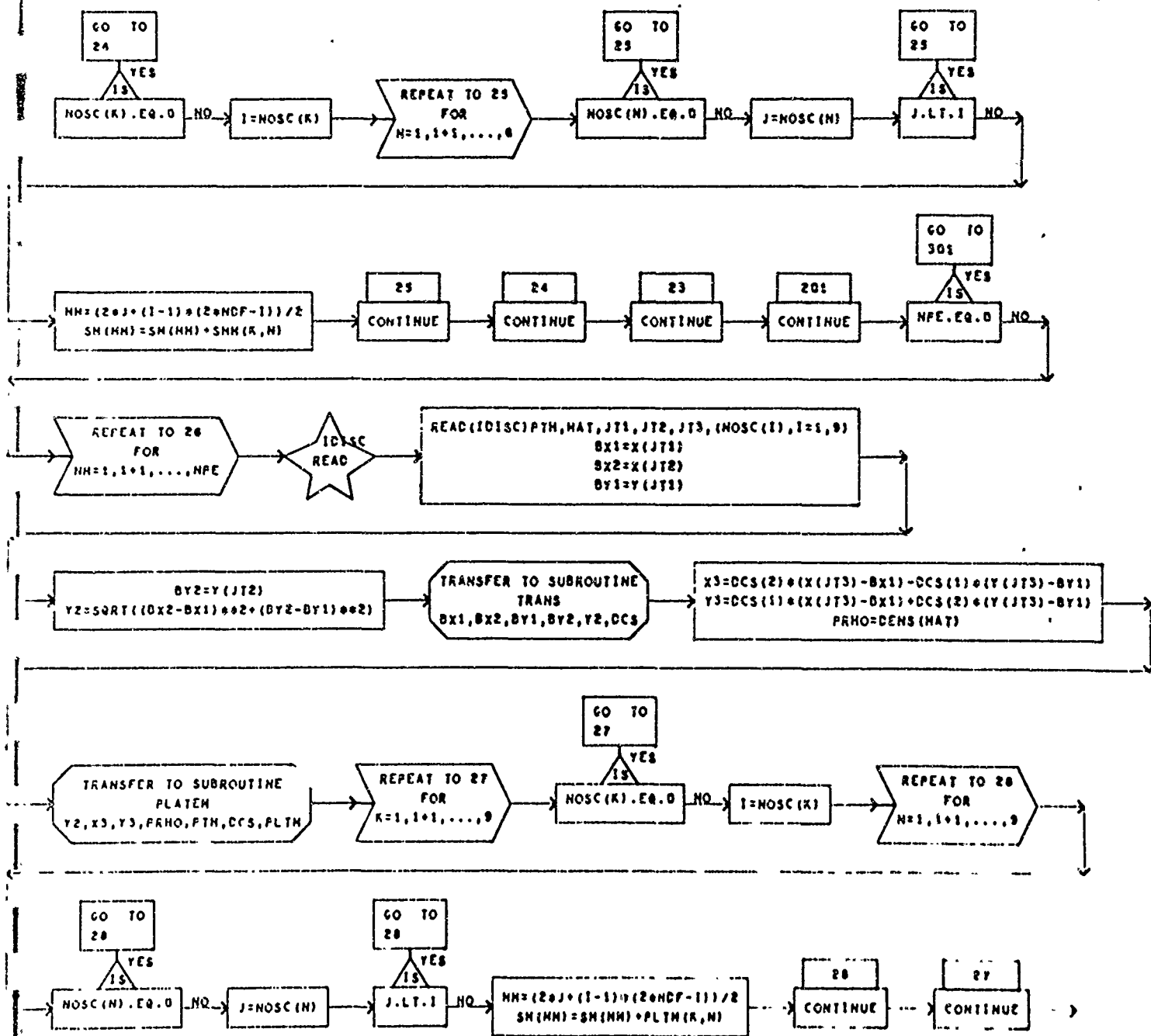


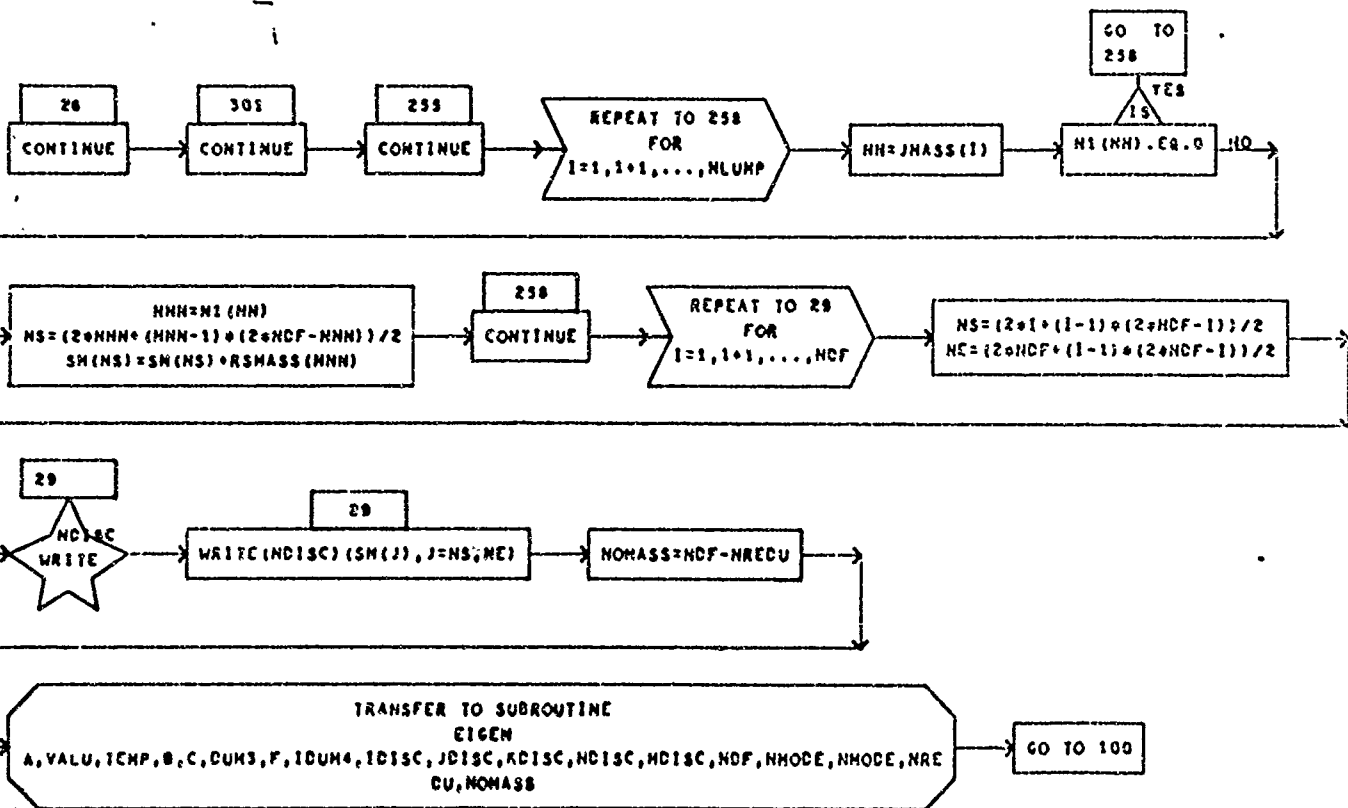












BEAM PLANE GRID BEAM ELEMENT STIFFNESS MAT. IN SYSTEM COORDS.

FL = BEAM LENGTH

E = YOUNG'S MODULUS

G = MODULUS OF RIGIDITY

XI = AREA MOMENT OF INERTIA

YJ = EFFECTIVE TORSIONAL MOMENT OF INERTIA

STM = STIFFNESS MATRIX

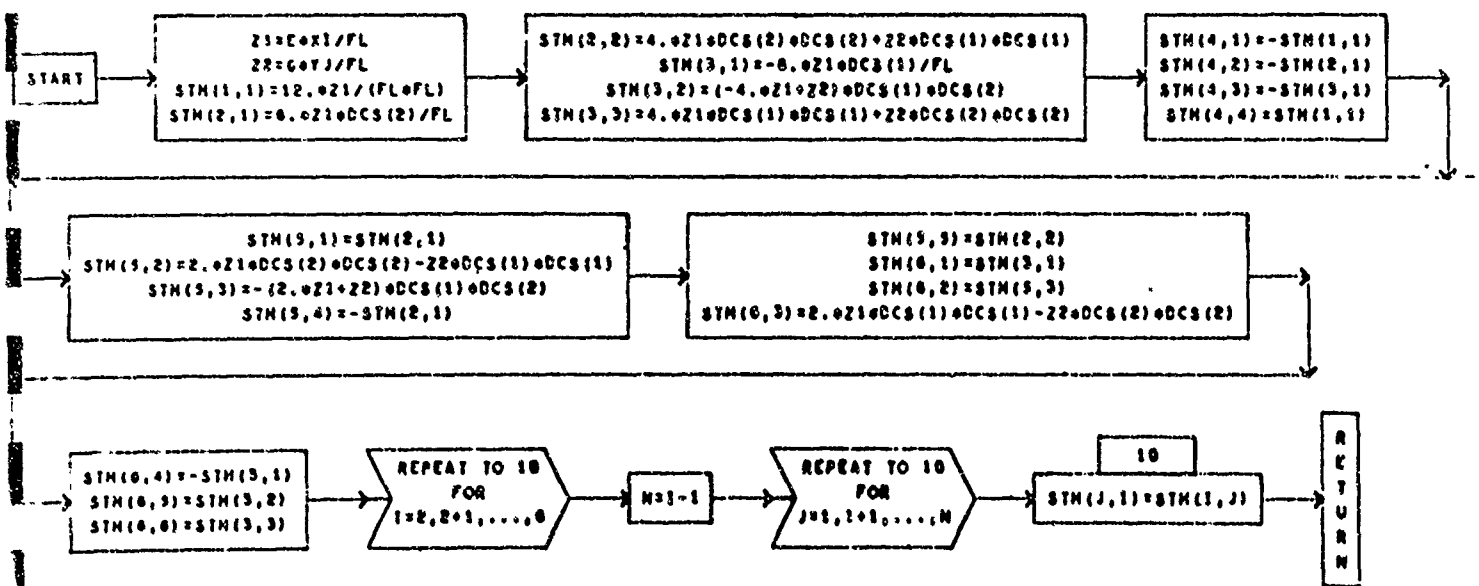
DCS = DIRECTION COSINES

D I M E N S I O N E D V A R I A B L E S

SYMBOL	STORAGES	SYMBOL	STORAGES	SYMBOL	STORAGES	SYMBOL	STORAGES	SYMBOL	STORAGES
STM	6, 9	DCS	2						

SUBROUTINE BEAM(FL,E,G,XI,YJ,STM,DCS)

PAGE 1



SIMMTH

THIS SUBROUTINE DETERMINES THE DOUBLE INTEGRAL MATRIX FOR
THE TRIANGULAR PLATE M MATRIX - FRZENIENIECKI, PAGE 304

$X2, X3, X3 =$ COORDS. OF PLATE CORNERS IN LOCAL COORDINATES

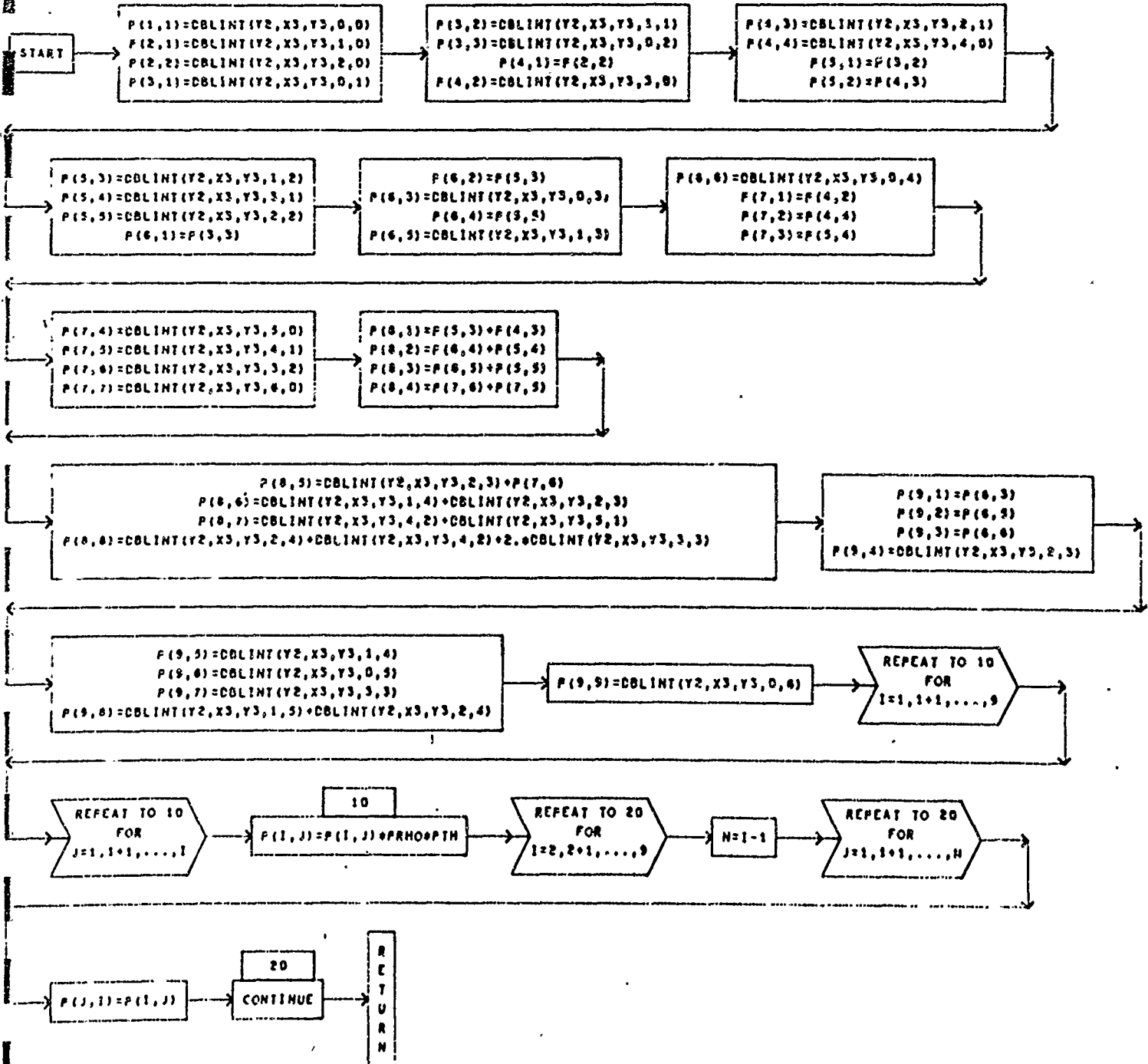
ρ = DENSITY

t = PLATE THICKNESS

P = DOUBLE INTEGRAL MATRIX

D I M E N S I O N E D V A R I A B L E S

SYMBOL	STORAGES	SYMBOL	STORAGES	SYMBOL	STORAGES	SYMBOL	STORAGES	SYMBOL	STORAGES
P	9,9								



MINV . MATRIX INVERSION SUBROUTINE

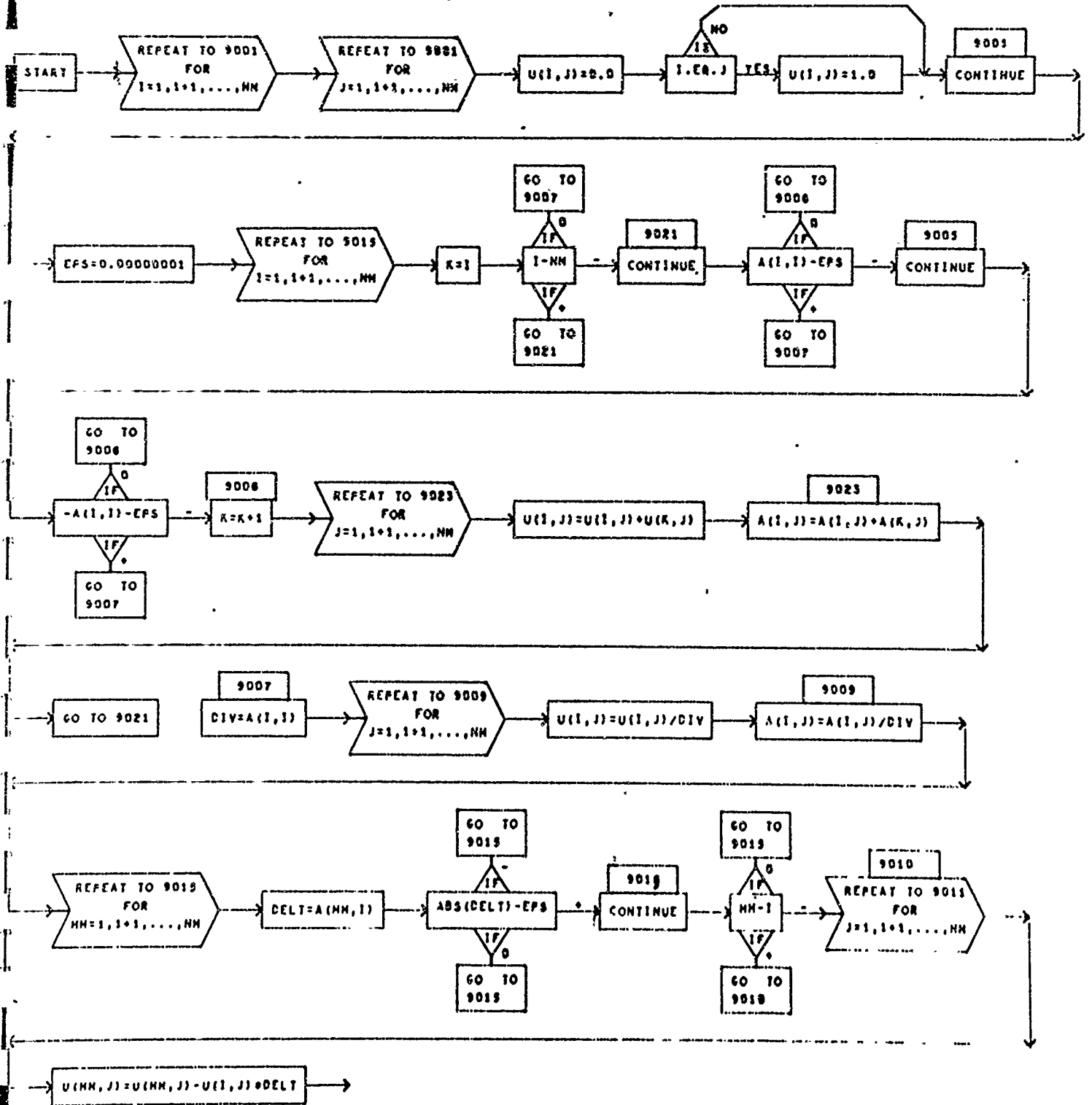
A = MATRIX TO BE INVERTED

U = INVERTED MATRIX

NN = ORDER OF MATRIX (I.E. 9)

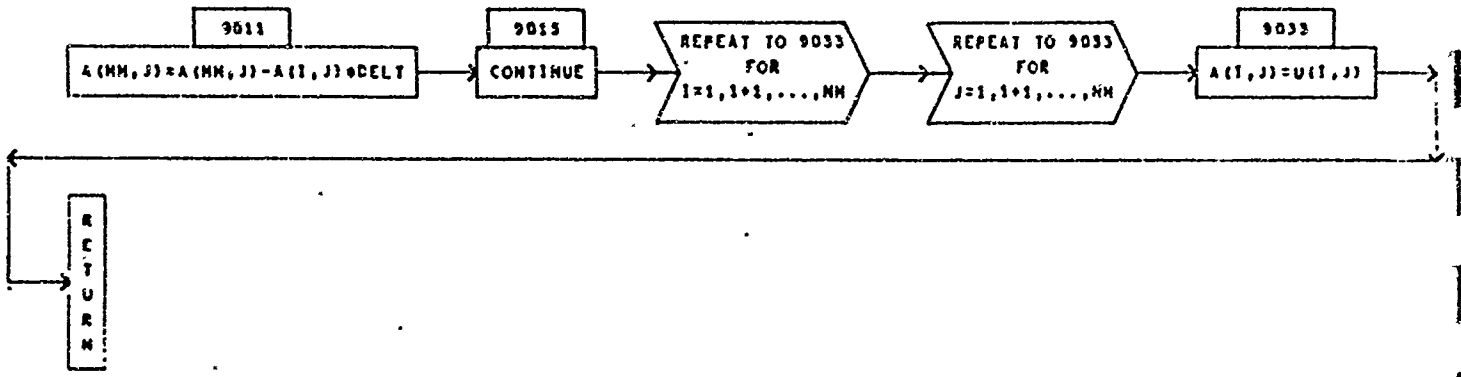
D I M E N S I O N E D V A R I A B L E S

SYMBOL	STORAGES	SYMBOL	STORAGES	SYMBOL	STORAGES	SYMBOL	STORAGES	SYMBOL	STORAGES
A	9,9	U	9,9						



SUBROUTINE MINV(A,U,NM)

PAGE



EIGEN REDUCES STIFFNESS MATRIX AND INVERTS IT, REDUCES MASS MATRIX
 DETERMINES EIGENVALUES AND EIGENVECTORS

THE ARGUMENTS ARE:

A - VECTOR OF LENGTH $NRDF \cdot (NRDF + 1) / 2$

VALU - VECTOR OF LENGTH NEIG

TEMP, B, C, DUNS, - VECTORS OF LENGTH NRDF OR NMASS (SMALLER)

E - MATRIX OF DIMENSION (NRDF, 3)

IDUM4 - VECTOR OF LENGTH NRDF OR NMASS (SMALLER)

ITAPE, JTAPE, KTAPE, LTAPE, - THESE ARE VARIOUS TAPES

NRDF - NUMBER OF DEGREES OF FREEDOM OF THE SYSTEM

NEIG - NUMBER OF EIGENVALUES DESIRED

NVEC - NUMBER OF EIGENVECTORS DESIRED

NMASS = NO. OF NORMAL DISPLACEMENTS

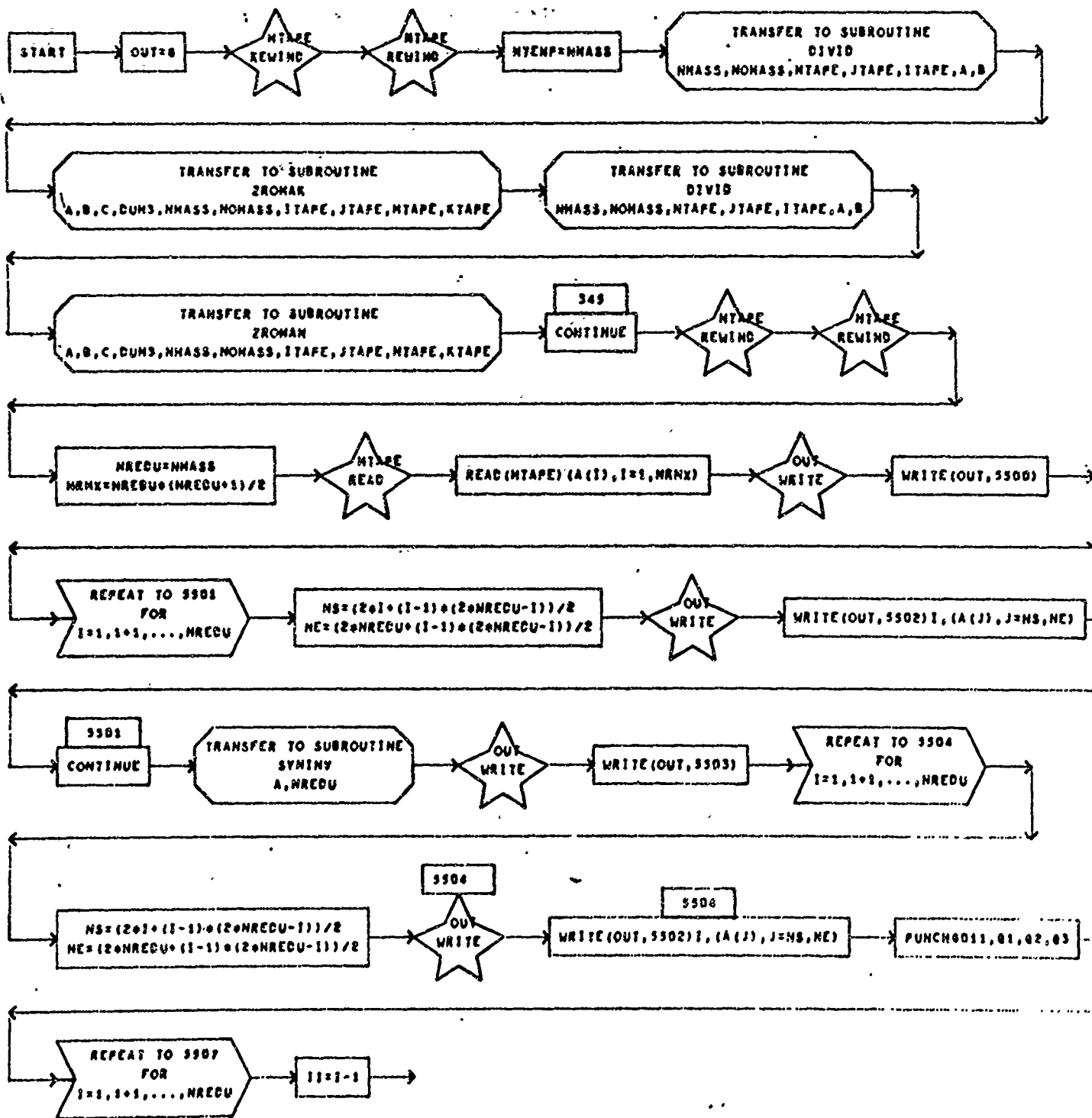
NRMASS = NO. OF ROTATIONAL DEGREES OF FREEDOM

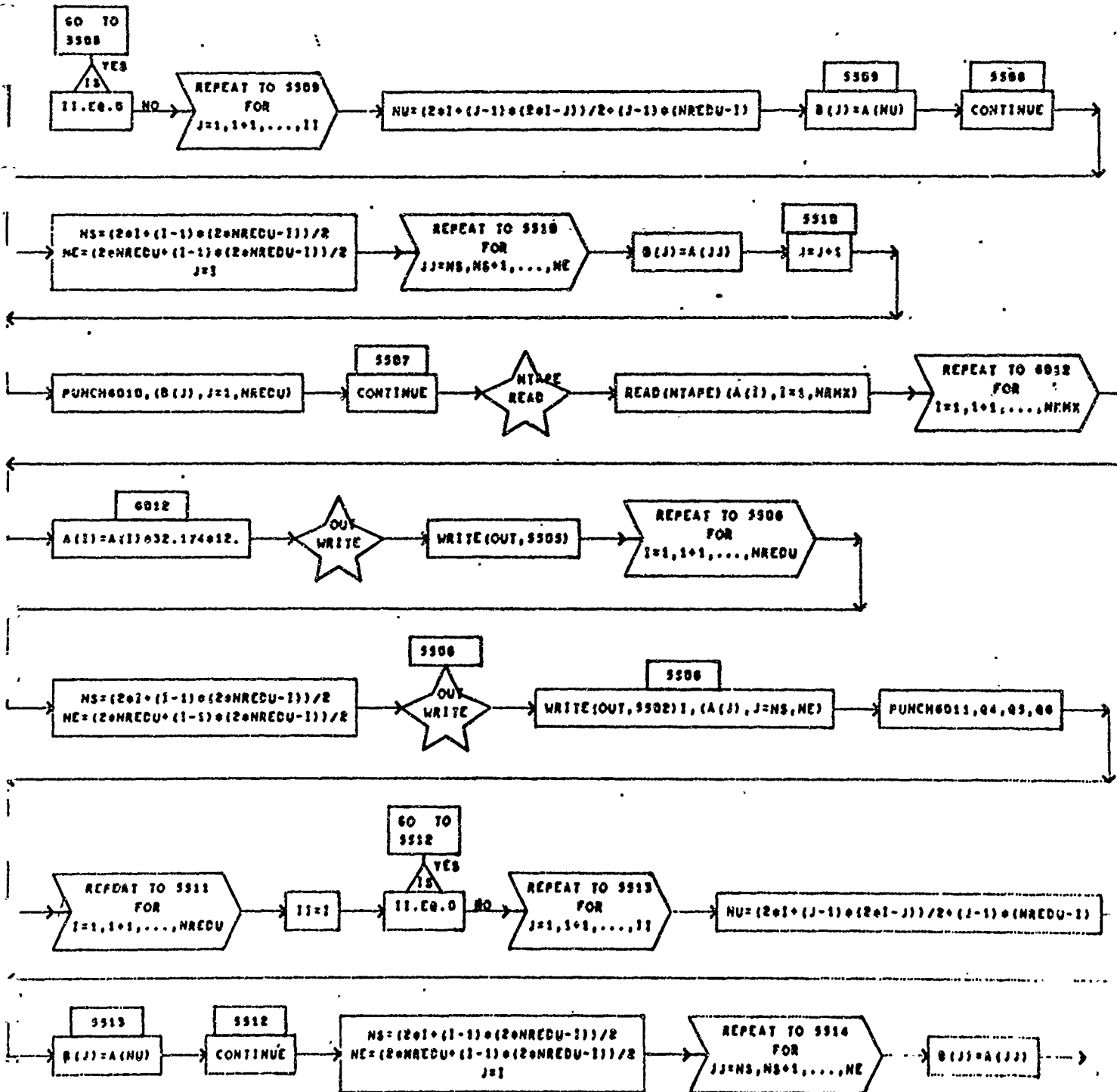
STIFF IS ON KTAPE IN COMPACT FORM

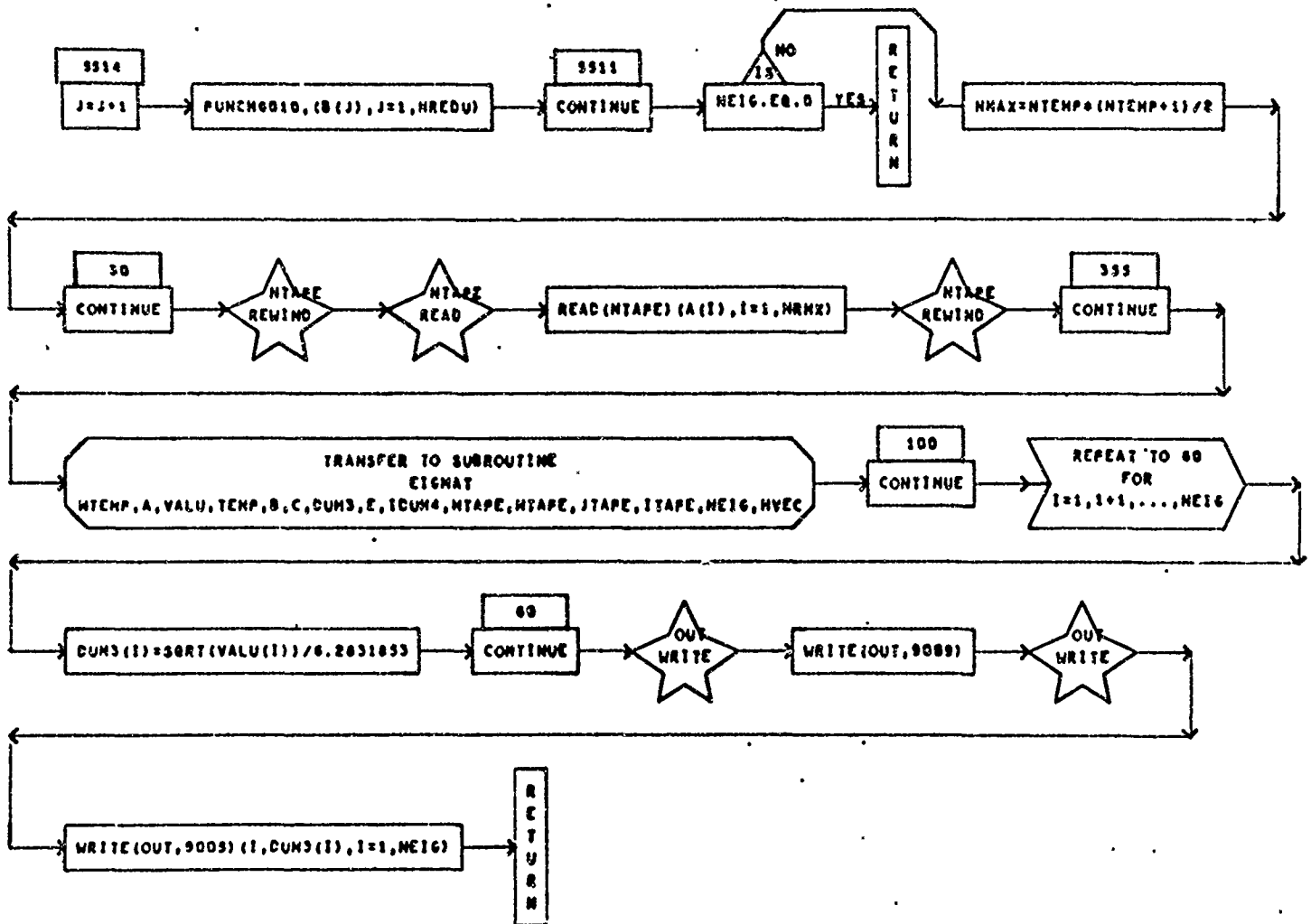
MASS IS ON LTAPE IN COMPACT FORM

D I M E N S I O N E D V A R I A B L E S

SYMBOL	STORAGES	SYMBOL	STORAGES	SYMBOL	STORAGES	SYMBOL	STORAGES	SYMBOL	STORAGES
DUNS	NRDF	IDUM4	1	A	1	VALU	1	B	1
C	1	E	NRDF, 3	TEMP	1				







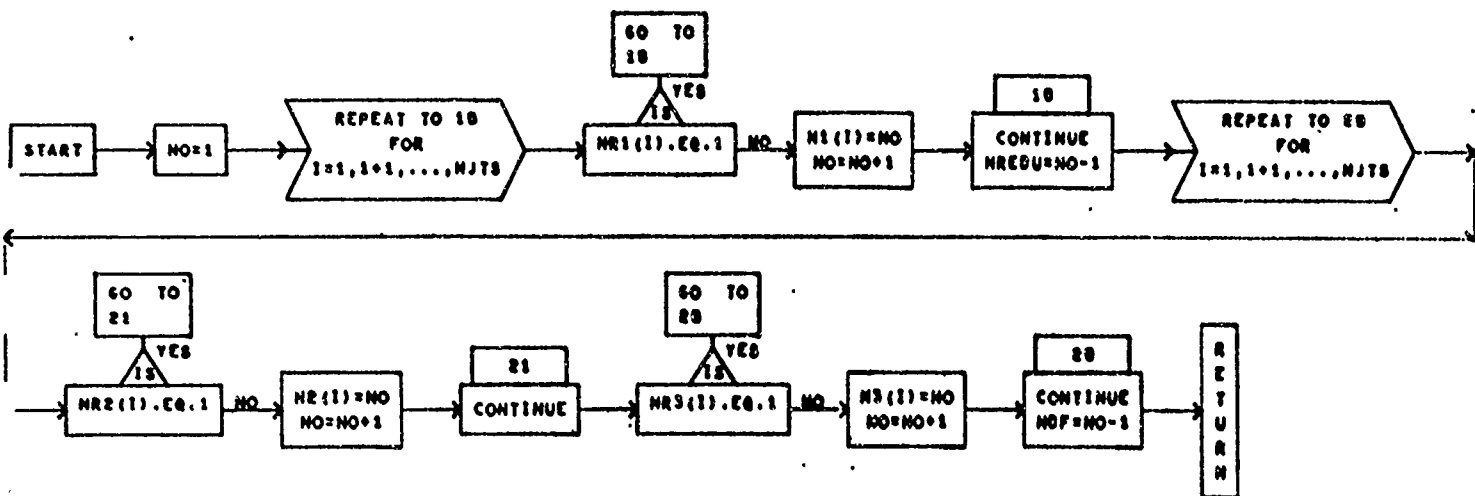
COORDM ASSIGNS A COORD. NO. TO EACH DEGREE OF FREEDOM AT EACH JOINT
 NR1, NR2, NR3 = ARRAYS CONTAINING RESTRAINT INFO. FOR EACH DEGREE
 OF FREEDOM AT EACH JOINT (FREE=0, CLAMPED=1)
 N1, N2, N3 = COORD. NO. FOR EACH DEGREE OF FREEDOM (NORMAL
 DISPLACEMENTS ARE NUMBERED FIRST)
 NJTS = NO. OF JOINTS
 NREBU = NO. OF NORMAL DISPLACEMENTS
 NCF = TOTAL NO. OF DEGREES OF FREEDOM (INCLUDING RESTRAINTS)

S I M E N T I O N E D V A R I A B L E S

SYMBOL	STORAGE	SYMBOL	STORAGE	SYMBOL	STORAGE	SYMBOL	STORAGE	SYMBOL	STORAGE
NR1	SB	NR2	SB	NR3	SB	N1	SB	N2	SB
N3	SB								

SUBROUTINE COORDM(NR1, NR2, NR3, N1, N2, N3, NJTS, NREBU, NCF)

PAGE 1

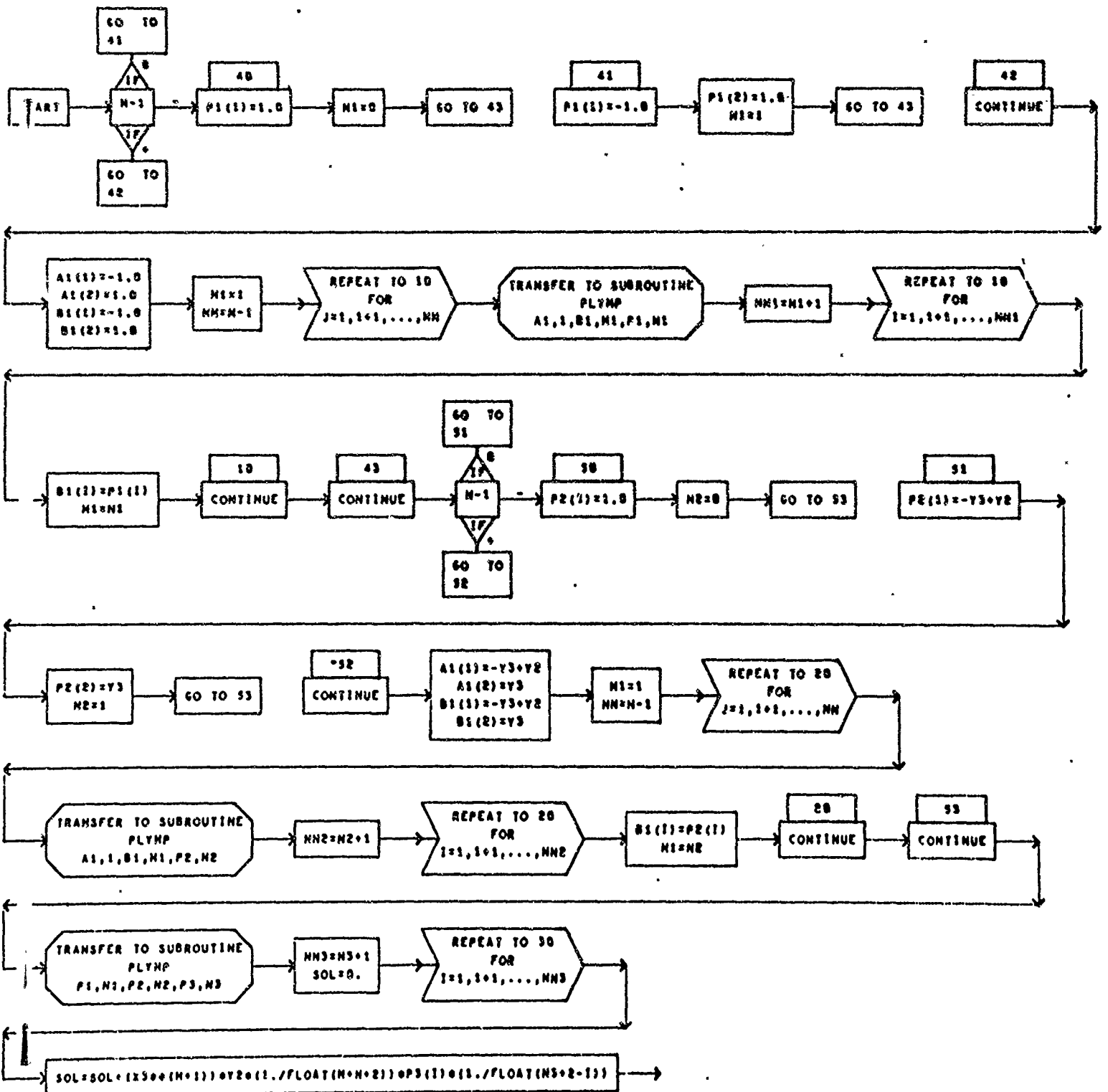


DDLINE

THIS SUBROUTINE EVALUATES THE DOUBLE INTEGRALS APPEARING IN THE
EQUATIONS FOR R AND N FOR THE TRIANGULAR PLATE ELEMENT
X2,X3,X3 = COORDS. OF PLATE CORNERS IN LOCAL COORDINATES
N,N = POWER OF X AND Y RESPECTIVELY, PRZENIENIECKI, PAGE 388

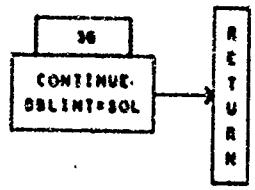
S I M E N S I O N E D V A R I A B L E S

SYMBOL	STORAGES	SYMBOL	STORAGES	SYMBOL	STORAGES	SYMBOL	STORAGES	SYMBOL	STORAGES
A1	2	B1	7	P1	7	P2	7	P2	7



FUNCTION DBLINT(Y2,X3,V3,N,N)

PAGE 2



DMAT

THIS SUBROUTINE DETERMINES THE FLEXURAL RIGIDITY MATRIX IN
TRIANGLE LOCAL COORDINATES

DX,DY,D1,DXY,BETA = FLEXURAL RIGIDITY TERMS AND ANGLE OF MATERIAL
PRINCIPAL AXES W/O TRIANGLE LOCAL AXES

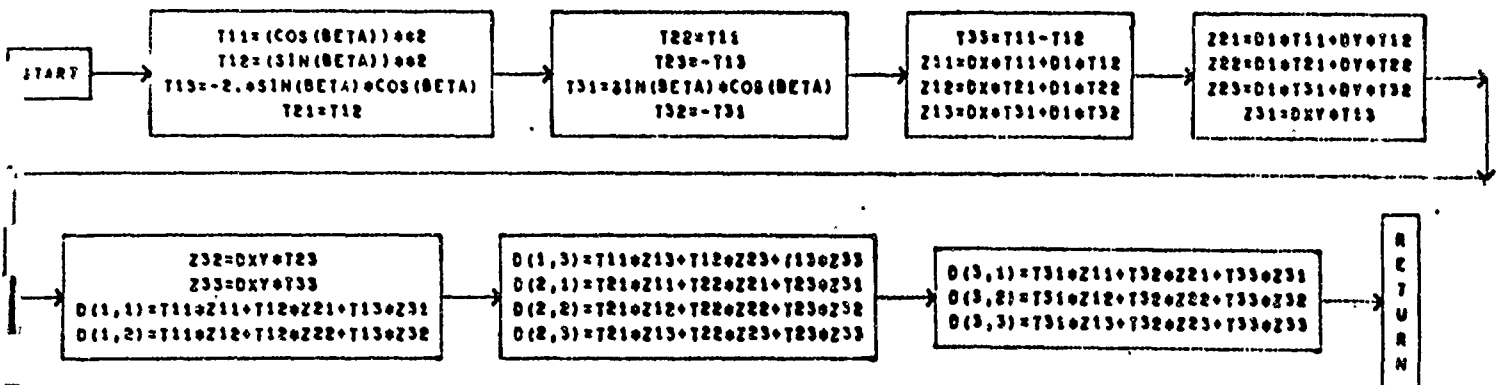
D = FLEXURAL RIGIDITY MATRIX IN TRIANGLE LOCAL COORDS.

D I M E N S I O N E D V A R I A B L E S

SYMBOL	STORAGES	SYMBOL	STORAGES	SYMBOL	STORAGES	SYMBOL	STORAGES	SYMBOL	STORAGES
D	3,3								

SUBROUTINE DMAT(DX,DY,D1,DXY,BETA,D)

PAGE 1



TRANS TRANSFORMATION DIRECTION COSINES

X1,Y1 = COORDS. OF POINT 1

X2,Y2 = COORDS. OF POINT 2

PL = DISTANCE BETWEEN POINTS 1 AND 2

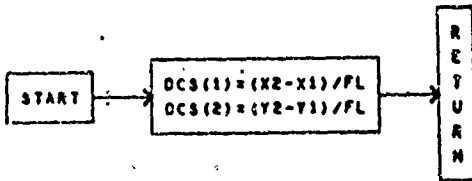
DCS = DIRECTION COSINES OF VECTOR FROM POINT 1 TO POINT 2

D I M E N S I O N E D V A R I A B L E S

SYMBOL	STORAGES	SYMBOL	STORAGES	SYMBOL	STORAGES	SYMBOL	STORAGES	SYMBOL	STORAGES
DCS	2								

SUBROUTINE TRANS(X1,X2,Y1,Y2,PL,DCS)

PAGE 1



DIMMAT

THIS SUBROUTINE DETERMINES THE DOUBLE INTEGRAL MATRIX FOR

THE K EQUATION FOR THE TRIANGULAR PLATE ELEMENT

$Y2, X3, Y3$ = COORDS. OF PLATE CORNERS IN LOCAL COORDINATES

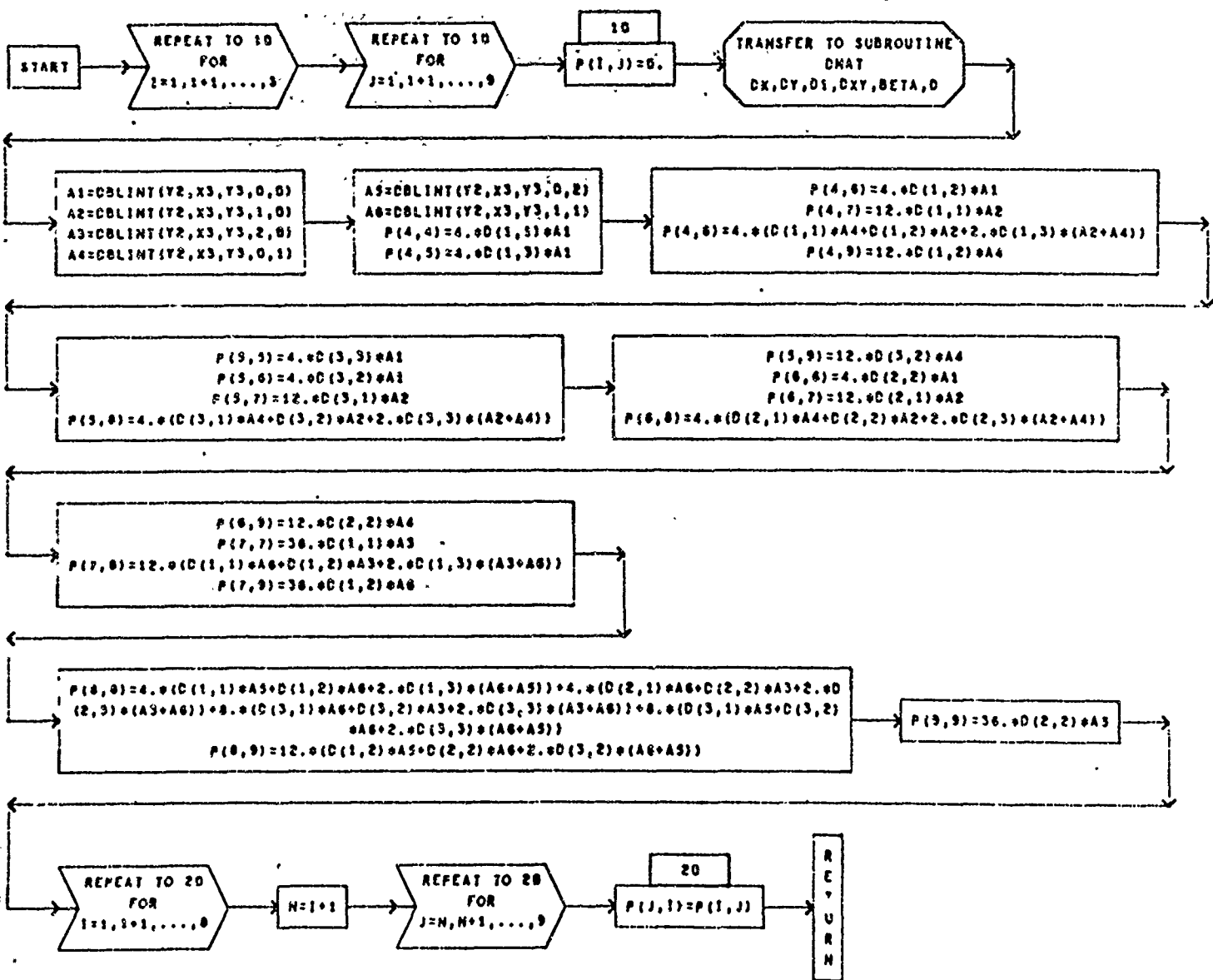
$DX, DY, D1, DXY, BETA$ = FLEXURAL RIGIDITY TERMS AND ANGLE OF MATERIAL

PRINCIPAL AXES W/O TRIANGLE LOCAL AXES

P = DOUBLE INTEGRAL MATRIX

D I M E N S I O N E D V A R I A B L E S

SYMBOL	STORAGES	SYMBOL	STORAGES	SYMBOL	STORAGES	SYMBOL	STORAGES	SYMBOL	STORAGES
P	9,9	D	3,3						



CHAT

THIS SUBROUTINE FORMS THE C MATRIX RELATING THE CORNER
DISPLACEMENTS TO THE POLYNOMIAL DEFLECTION COEFFICIENTS
FOR THE TRIANGULAR PLATE ELEMENT

x_2, x_3, y_3 = COORDS. OF PLATE CORNERS IN LOCAL COORDINATES

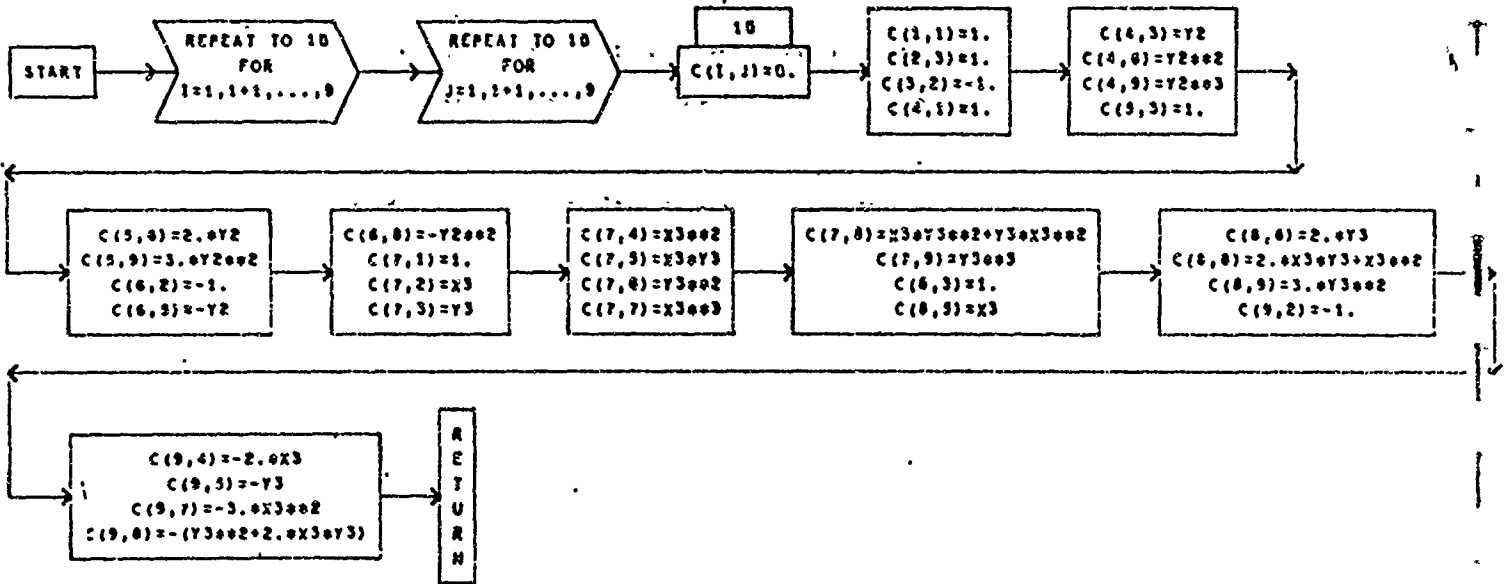
C = C MATRIX

D I M E N S I O N E D V A R I A B L E S

SYMBOL	STORAGE	SYMBOL	STORAGE	SYMBOL	STORAGE	SYMBOL	STORAGE	SYMBOL	STORAGE
C	9,9								

SUBROUTINE CMAT(X2,X3,Y3,C)

PAGE 1



PLATE

THIS SUBROUTINE DETERMINES THE STIFFNESS MATRIX OF A
TRIANGLE PLATE ELEMENT IN SYSTEM COORDS.

X2,X3,Y3 = COORDS. OF PLATE CORNERS IN LOCAL COORDINATES

DX,DY,DS,CXY,BETA = FLEXURAL RIGIDITY TERMS AND ANGLE OF MATERIAL

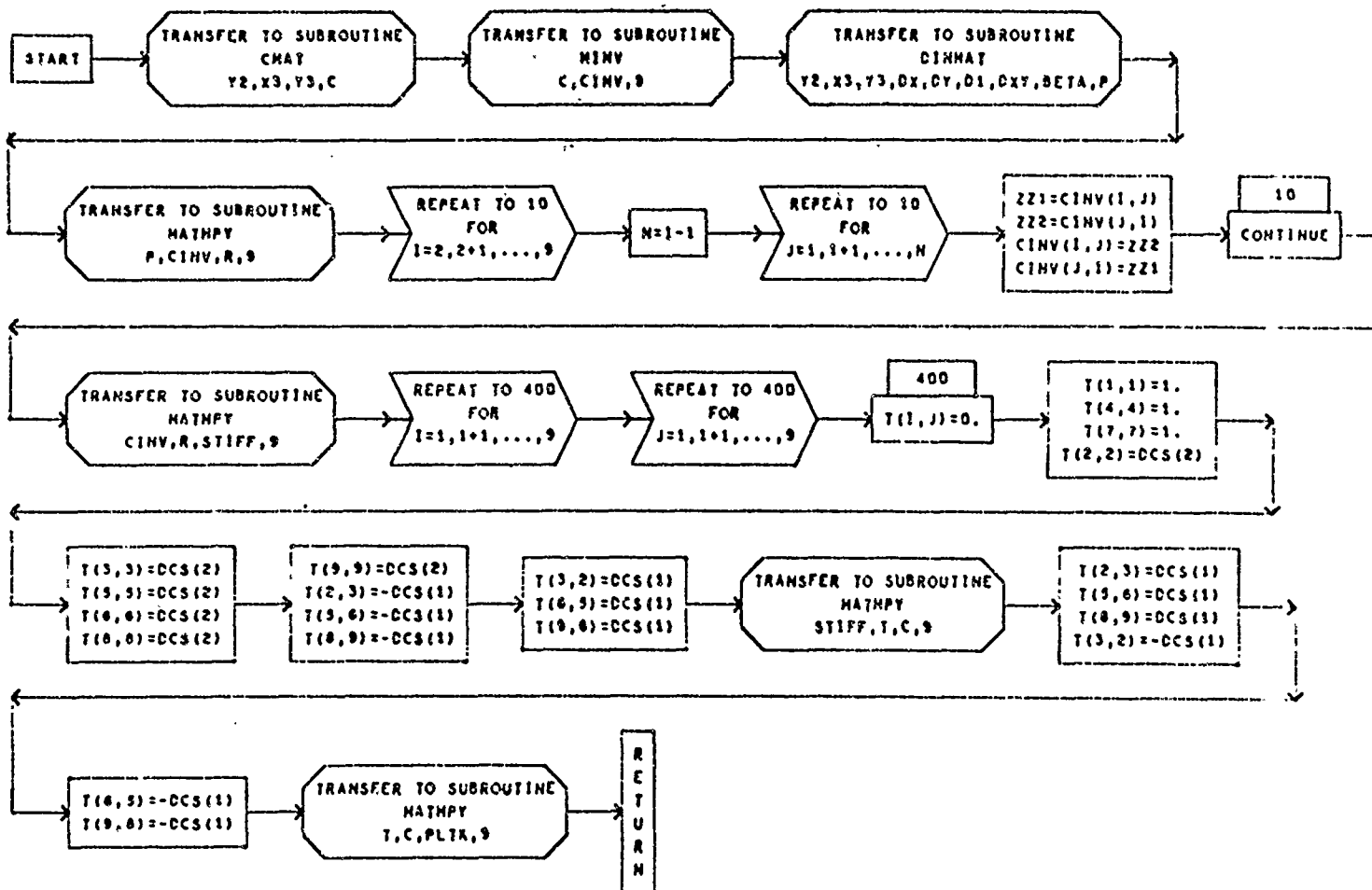
PRINCIPAL AXES W/O TRIANGLE LOCAL AXES

DCS = DIRECTION COSINES

PLTK = STIFFNESS MATRIX

D I M E N S I O N E D V A R I A B L E S

SYMBOL	STORAGES	SYMBOL	STORAGES	SYMBOL	STORAGES	SYMBOL	STORAGES	SYMBOL	STORAGES
PLTK	9,9	C	9,9	CINV	9,9	P	9,9	R	9,9
T	9,9	STIFF	9,9	DCS	2				



PLATEM

THIS SUBROUTINE DETERMINES THE MASS MATRIX OF A
TRIANGLE PLATE ELEMENT IN SYSTEM COORDS.

Y2,X3,Y5 = COORDS. OF PLATE CORNERS IN LOCAL COORDINATES

PRHO = DENSITY

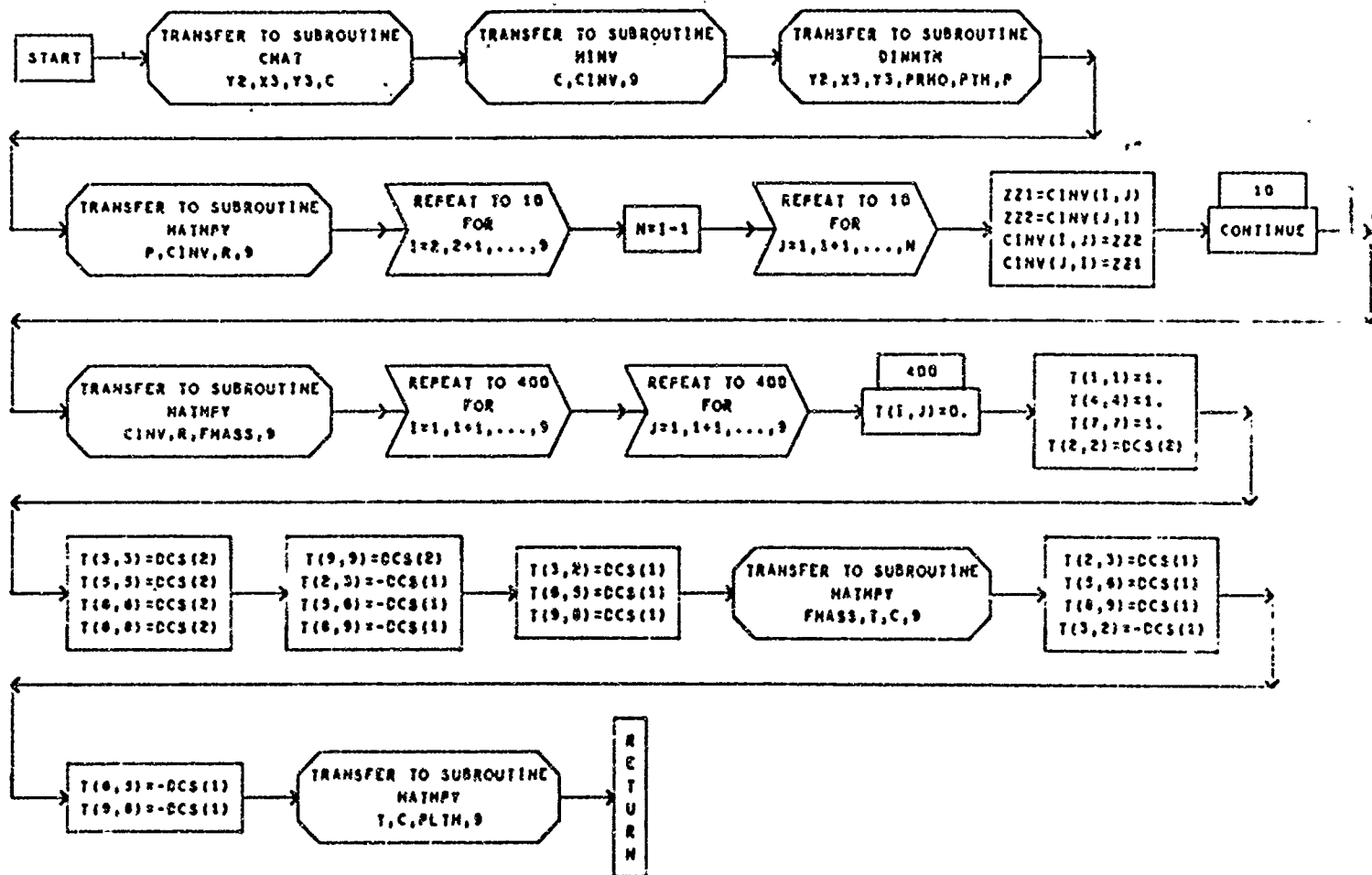
PTH = PLATE THICKNESS

DCS = DIRECTION COSINES

PLTN = MASS MATRIX

D I M E N S I O N E D V A R I A B L E S

SYMBOL	STORAGES	SYMBOL	STORAGES	SYMBOL	STORAGES	SYMBOL	STORAGES	SYMBOL	STORAGES
PLTN	9,9	C	9,9	CINV	9,9	P	9,9	R	9,9
T	9,9	FMASS	9,9	DCS	2				



BEAM PLANE GRID BEAM ELEMENT MASS MATRIX IN SYSTEM COORDS.

PL = BEAM LENGTH

RHO = DENSITY

A = CROSS SECTIONAL AREA

XI = AREA MOMENT OF INERTIA

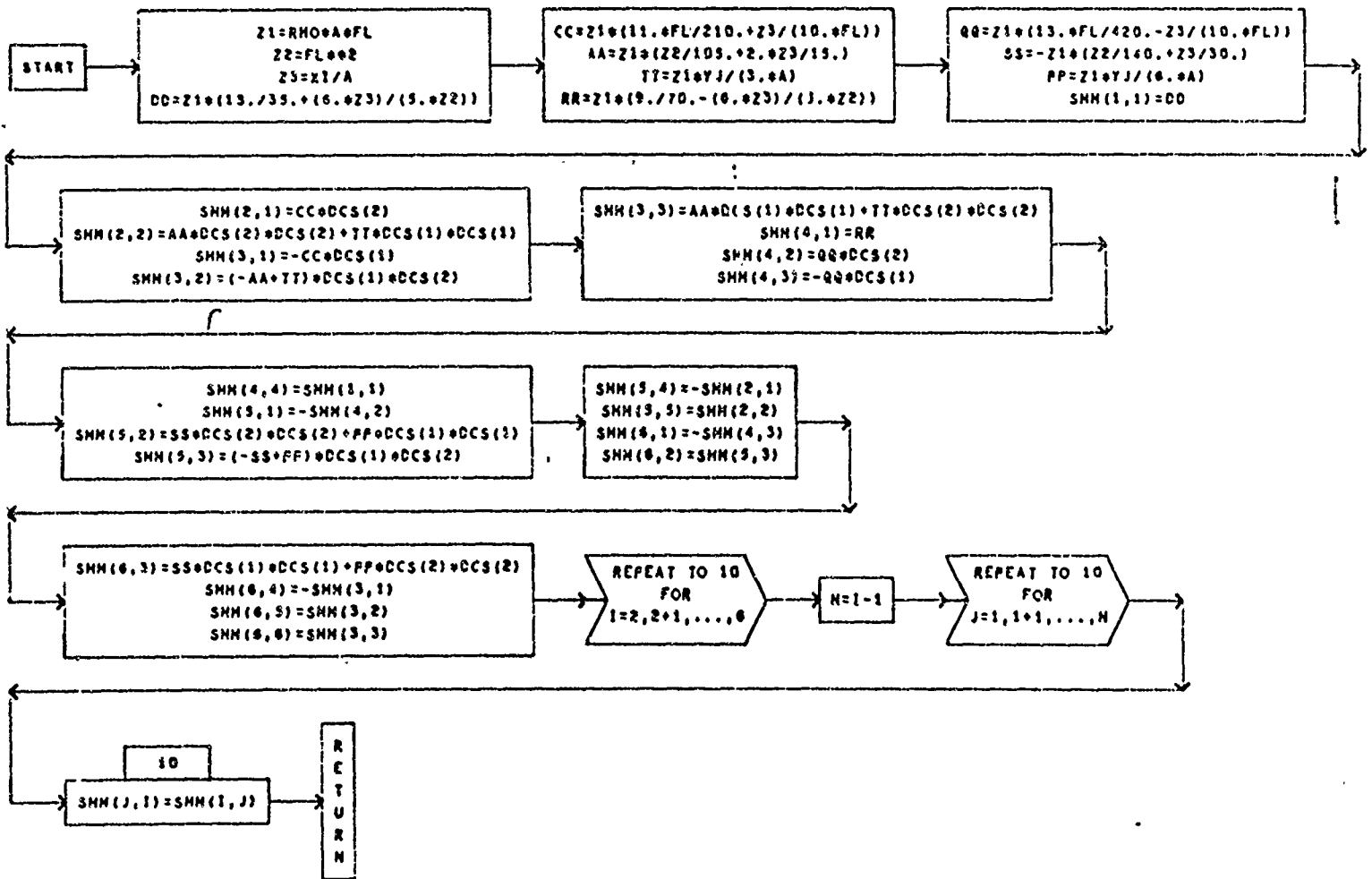
XJ = EFFECTIVE TORSIONAL MOMENT OF INERTIA

SMM = MASS MATRIX

DCS = DIRECTION COSINES

D I M E N S I O N S V A R I A B L E S

SYMBOL	STORAGE	SYMBOL	STORAGE	SYMBOL	STORAGE	SYMBOL	STORAGE	SYMBOL	STORAGE
SMM	9,6	DCS	2						



DIVIS

N=NO. OF NORMAL DISPLACEMENTS

M=NO. OF ROTATIONAL D.O.F.

HTPE-CONTAINS STIFFNESS (OR MASS) MATRIX

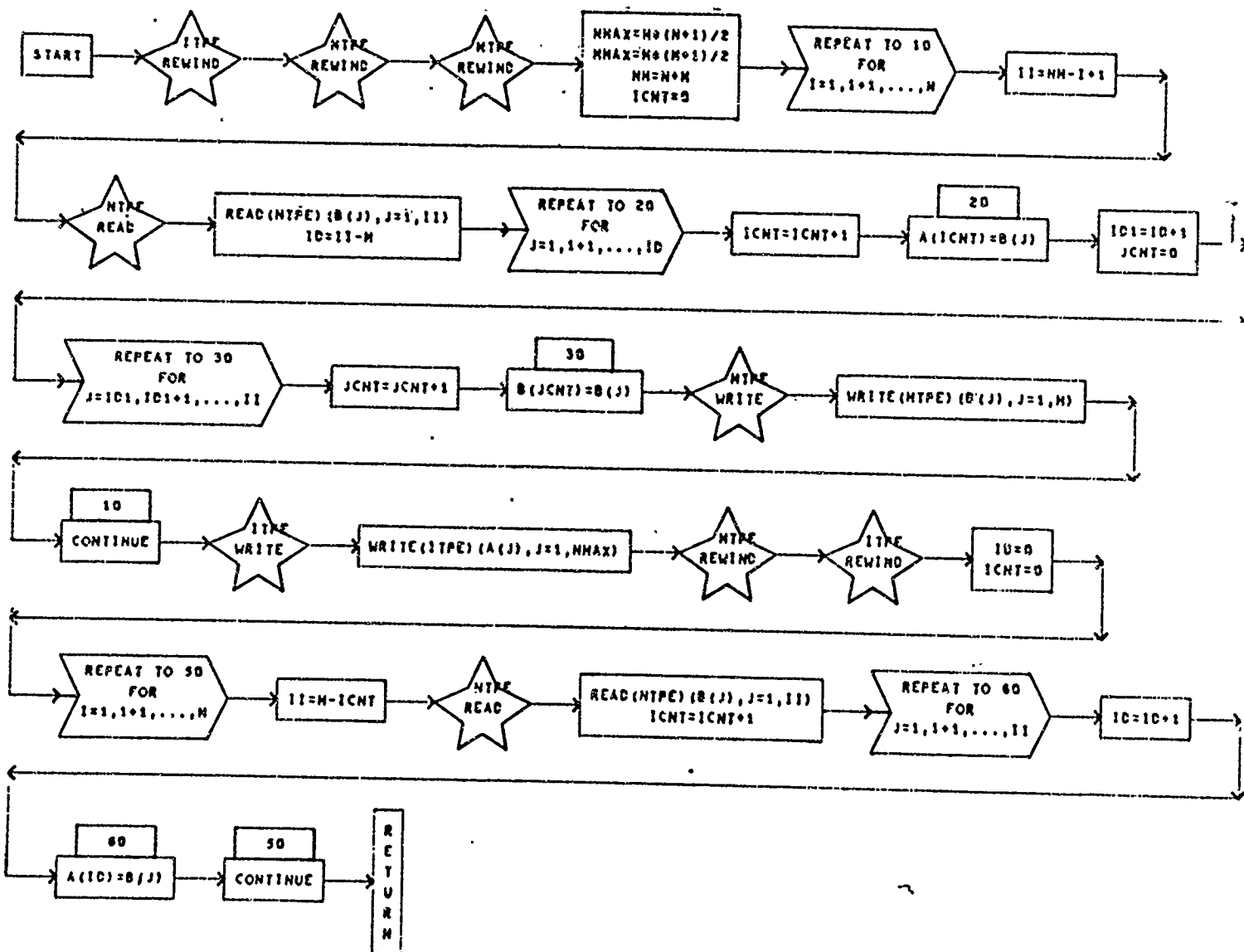
HTPE-K12 (M12) STORED

ITPE-R11 (M11) STORED

A- DUMMY STORAGE VECTOR, LARGER OF $M*(M+1)/2$ OR $N*(M+1)/2$

D I M E N S I O N E D V A R I A B L E S

SYMBOL	STORAGES	SYMBOL	STORAGES	SYMBOL	STORAGES	SYMBOL	STORAGES	SYMBOL	STORAGES
A	1	B	1						



ZROMAK

B IS A DUMMY VECTOR WITH STORAGE N OR N (LARGER)

A IS A DUMMY VECTOR WITH STORAGE $N*(N+1)/2$ OR $N*(N+1)/2$ (LARGER)

B IS A DUMMY VECTOR WITH STORAGE N OR N (LARGER)

C IS A DUMMY VECTOR WITH STORAGE N OR N (LARGER)

N=NO. OF NORMAL DISPLACEMENTS

M=NO. OF ROTATIONAL D.O.F.

N1PE CONTAINS K11 MATRIX

N1PE CONTAINS K12 MATRIX

I1PE SCRATCH TAPE

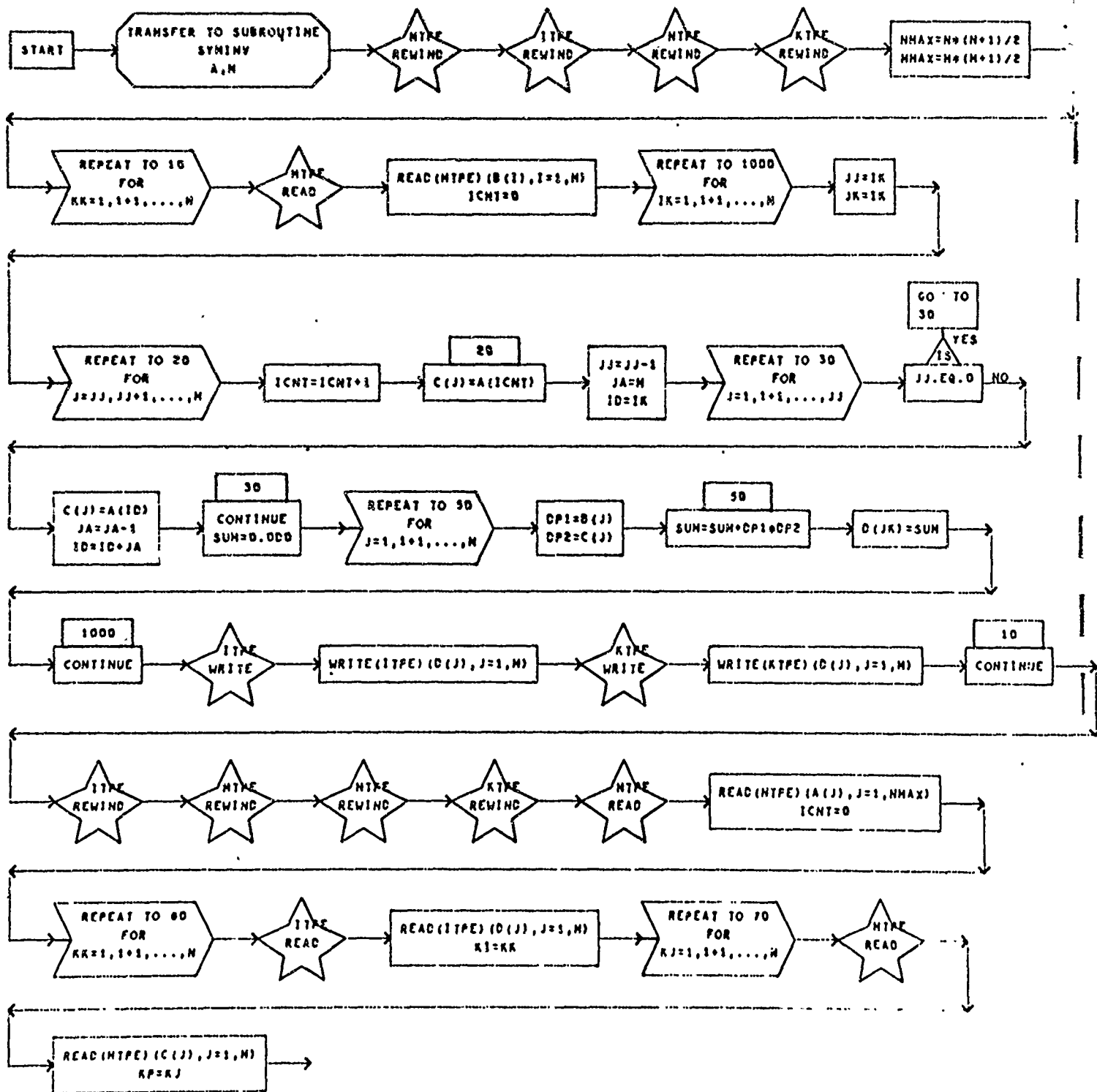
K1PE STORES $K12+K22*(-1)$

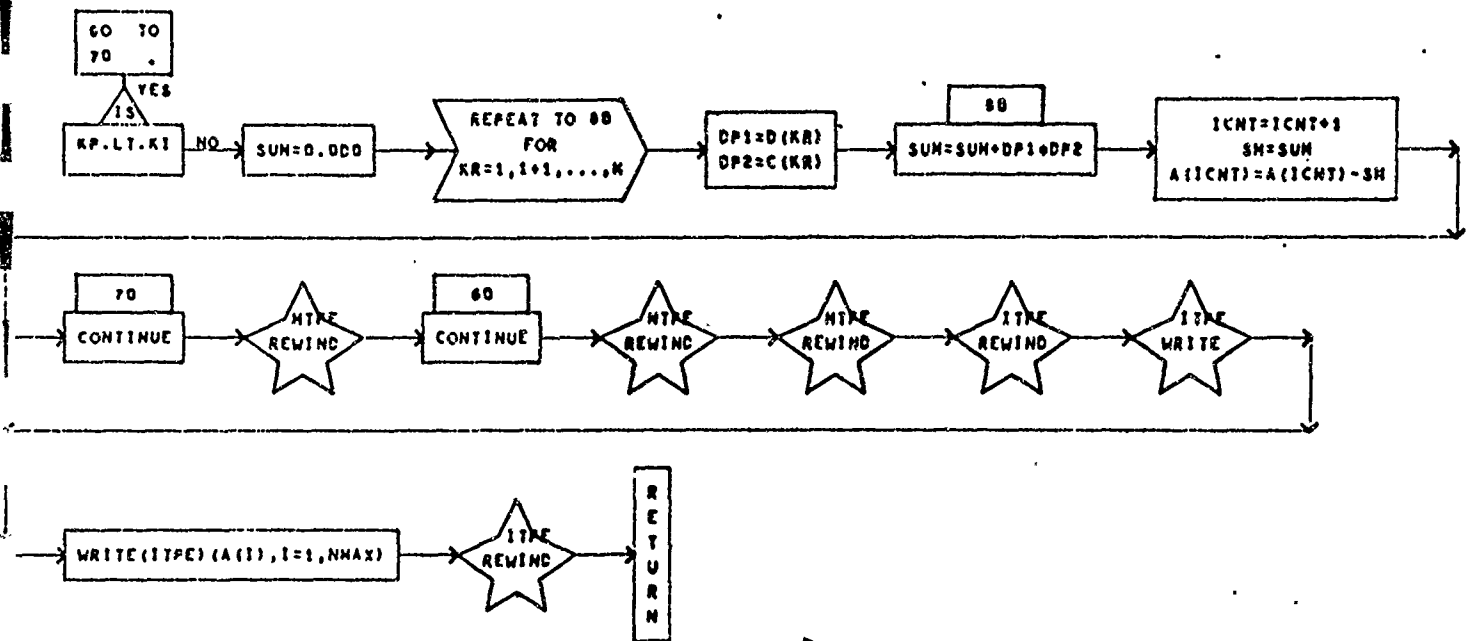
A INITIALLY CONTAINS K22 INVERSE

*** REDUCED STIFFNESS MATRIX IS STORED ON I1PE

D I M E N S I O N E D V A R I A B L E S

SYMBOL	STORAGES	SYMBOL	STORAGES	SYMBOL	STORAGES	SYMBOL	STORAGES	SYMBOL	STORAGES
A	1	B	1	C	1	D	1		





ZROHAN

N=NO. OF NORMAL DISPLACEMENTS

M=NO. OF ROTATIONAL D.O.F.

HTPE CONTAINS H11 MATRIX

HTPE CONTAINS H12 MATRIX

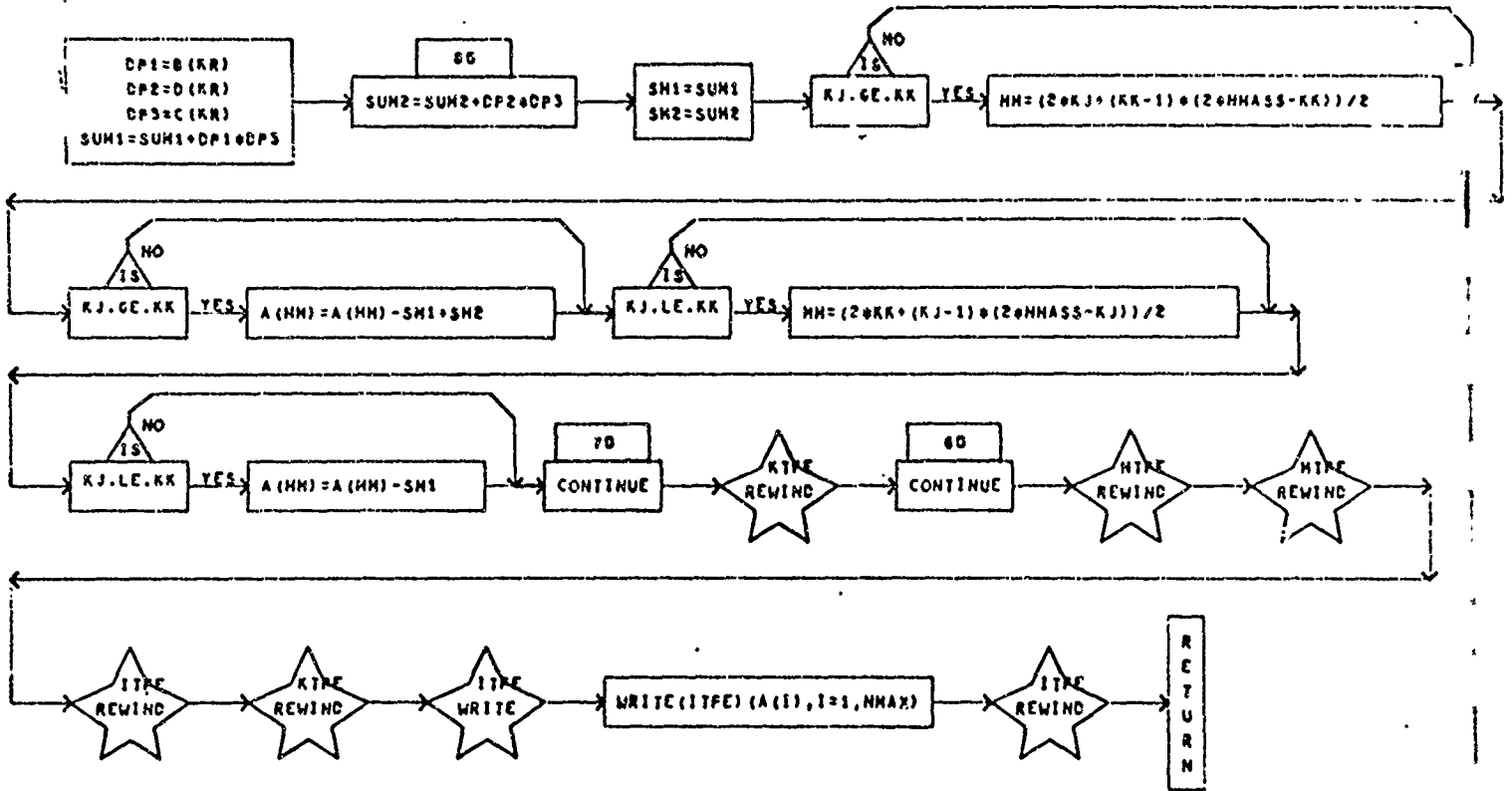
ITPE SCRATCH TAPE

KTPE CONTAINS $K12 \times K22$ (-1)

*** REDUCED MASS MATRIX IS STORED ON ITPE

D I M E N S I O N E D V A R I A B L E S

SYMBOL	STORAGES	SYMBOL	STORAGES	SYMBOL	STORAGES	SYMBOL	STORAGES	SYMBOL	STORAGES
A	1	B	1	C	1	D	1		



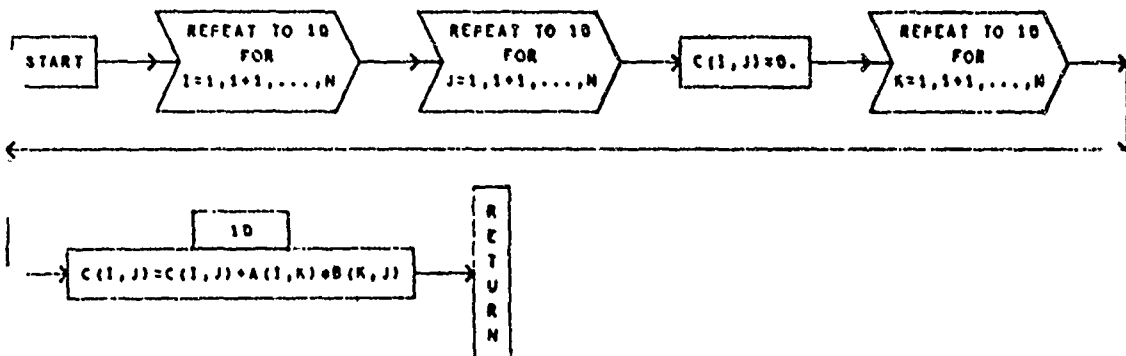
MATMPY
MULTIPLIES MATRICES A AND B TO GET C, ALL OF ORDER N*N

DIMENSIONED VARIABLES

SYMBOL	STORAGE	SYMBOL	STORAGE	SYMBOL	STORAGE	SYMBOL	STORAGE	SYMBOL	STORAGE
A	N,N	B	N,N	C	N,N				

SUBROUTINE MATMPY(A,B,C,N)

PAGE 1.



LOOP1

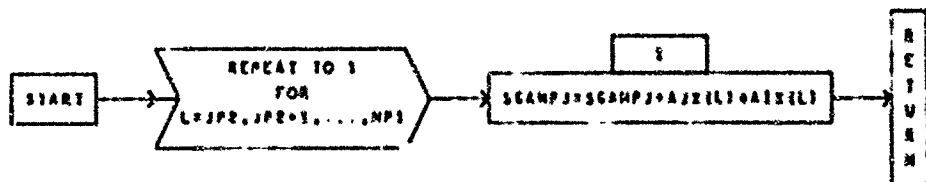
5
1

D I M E N S I O N E D V A R I A B L E S

SYMBOL	STORAGE	SYMBOL	STORAGE	SYMBOL	STORAGE	SYMBOL	STORAGE	SYMBOL	STORAGE
AIX	1	AIX	1						

SUBROUTINE LOOP1(JPR,NPI,SCAMPJ,AIX,AIX)

PAGE 1



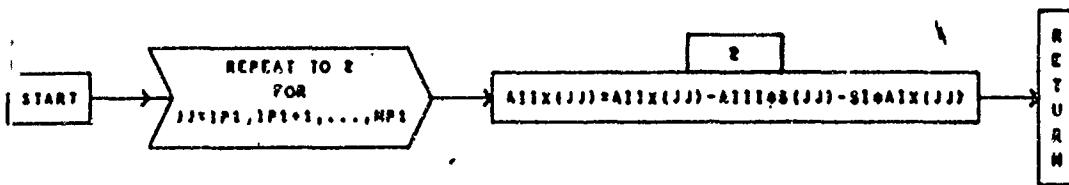
LOOP2

D I M E N S I O N E D V A R I A B L E S

SYMBOL	STORAGE	SYMBOL	STORAGE	SYMBOL	STORAGE	SYMBOL	STORAGE	SYMBOL	STORAGE
AIX	1	AIX	1	S	1				

SUBROUTINE LOOP2(AIX,AIX,S,S,AIX,IP1,MP1)

PAGE 1



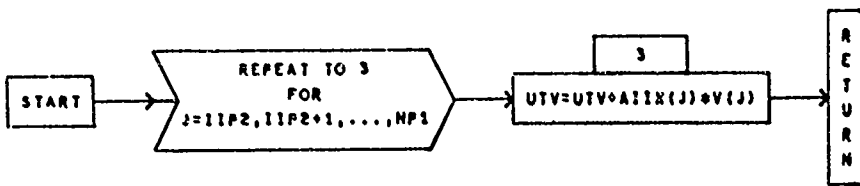
LOOP3

D I M E N S I O N E D V A R I A B L E S

SYMBOL	STORAGES	SYMBOL	STORAGES	SYMBOL	STORAGES	SYMBOL	STORAGES	SYMBOL	STORAGES
AIX	1	V	1						

SUBROUTINE LOOP3(UTV,AIX,V,IIP2,NP1)

PAGE 1



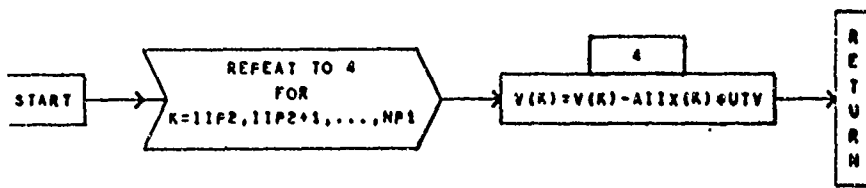
LOOP4

D I M E N S I O N E D V A R I A B L E S

SYMBOL	STORAGES	SYMBOL	STORAGES	SYMBOL	STORAGES	SYMBOL	STORAGES	SYMBOL	STORAGES
AIX	1	V	1						

SUBROUTINE LOOP4(AIX,V,NP1,IIP2,UTV)

PAGE 3

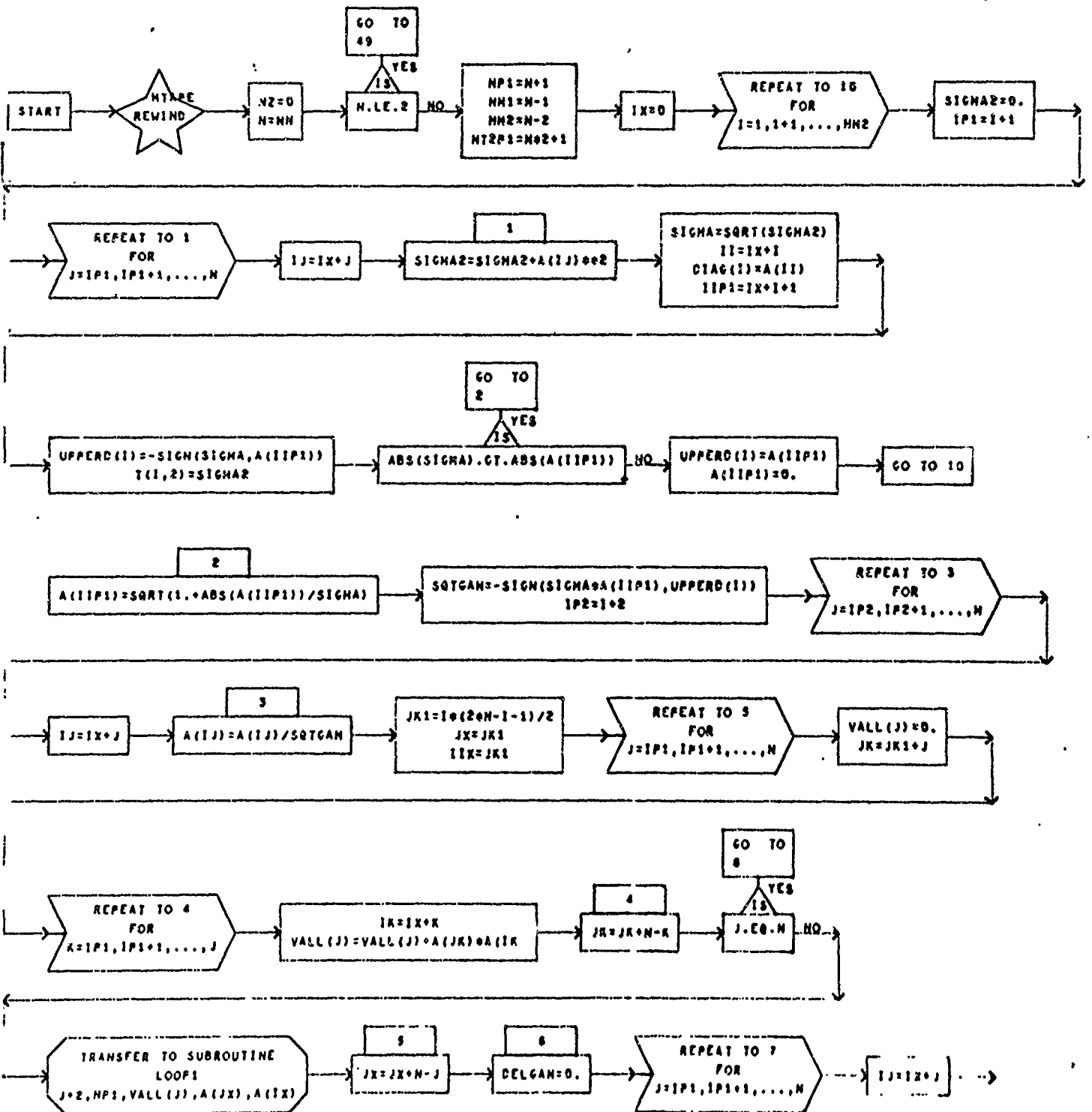


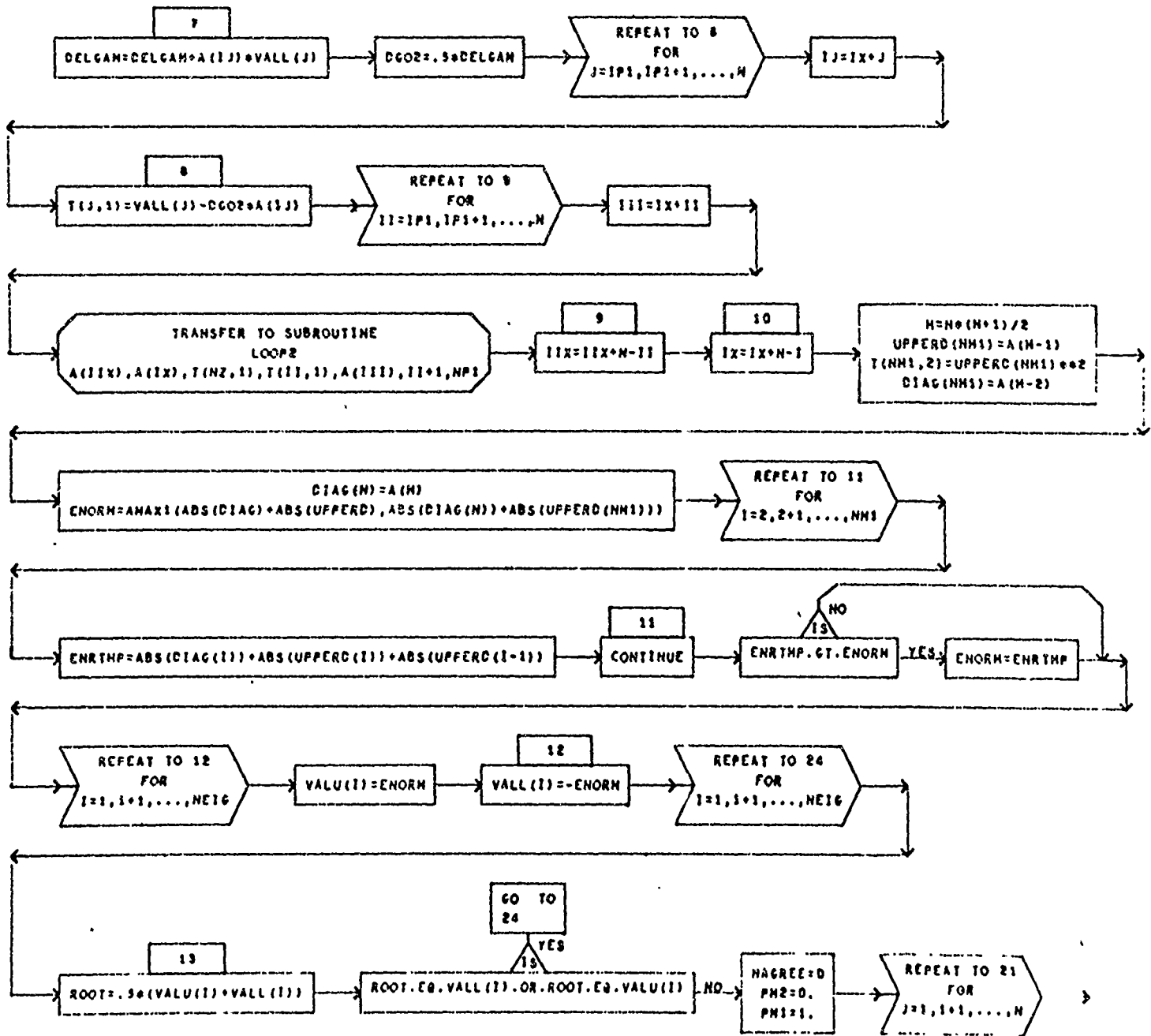
-BIGHAT

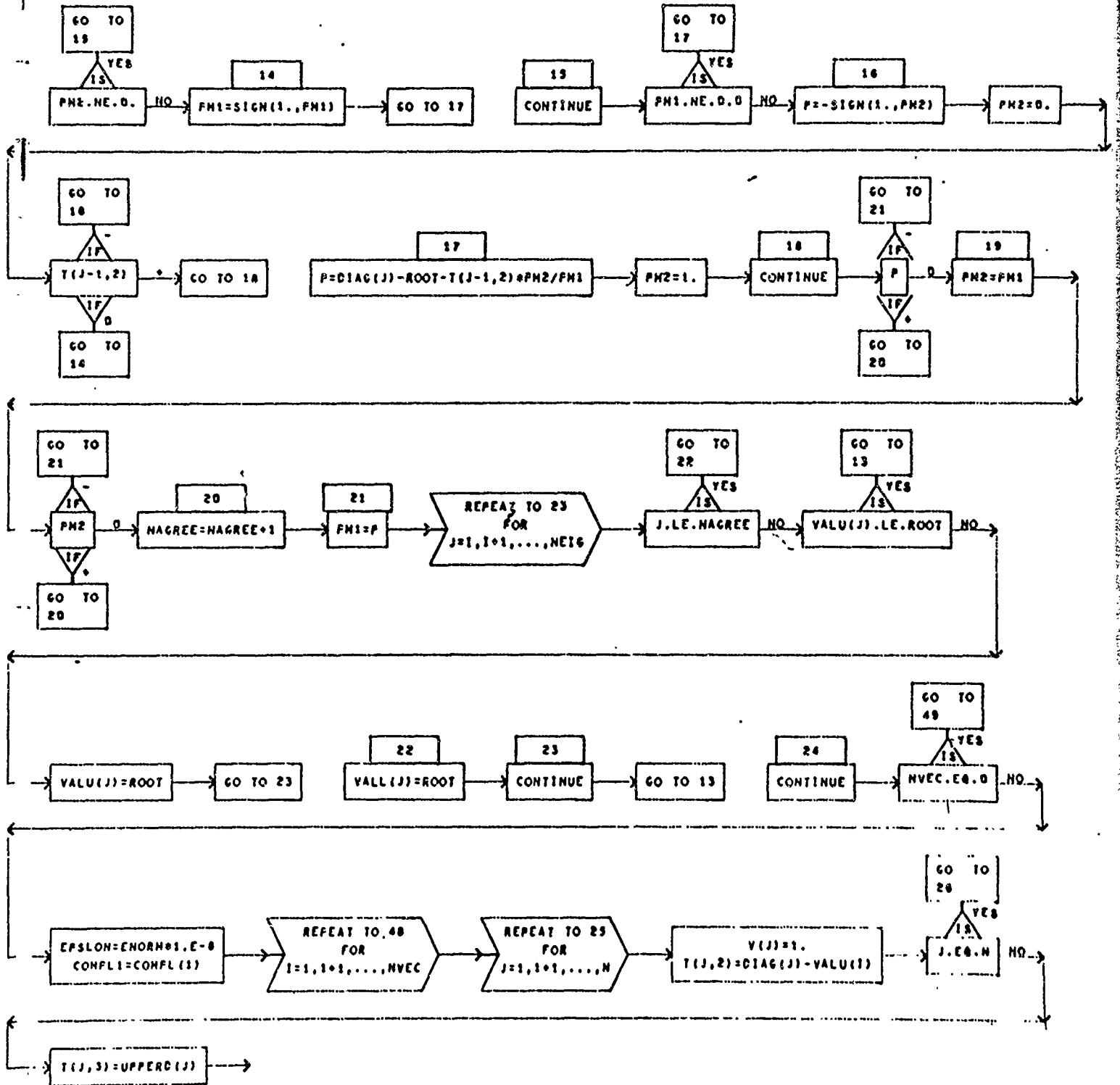
PROG. AUTHORS M. ELSON AND R. E. FUNDERLIC, CENTRAL DATA PROCESSING, 4, 1, 68

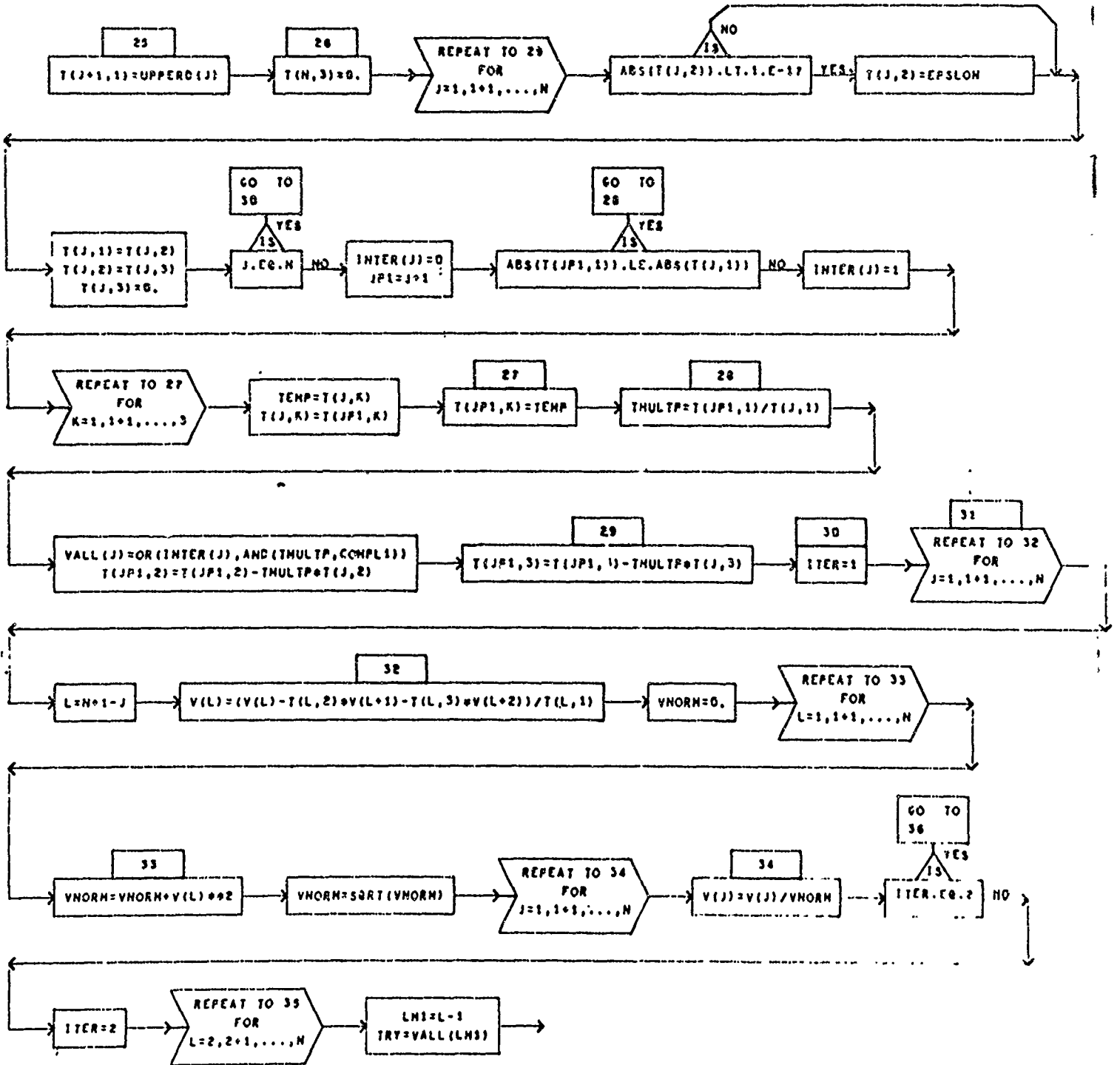
D I M E N S I O N E D V A R I A B L E S

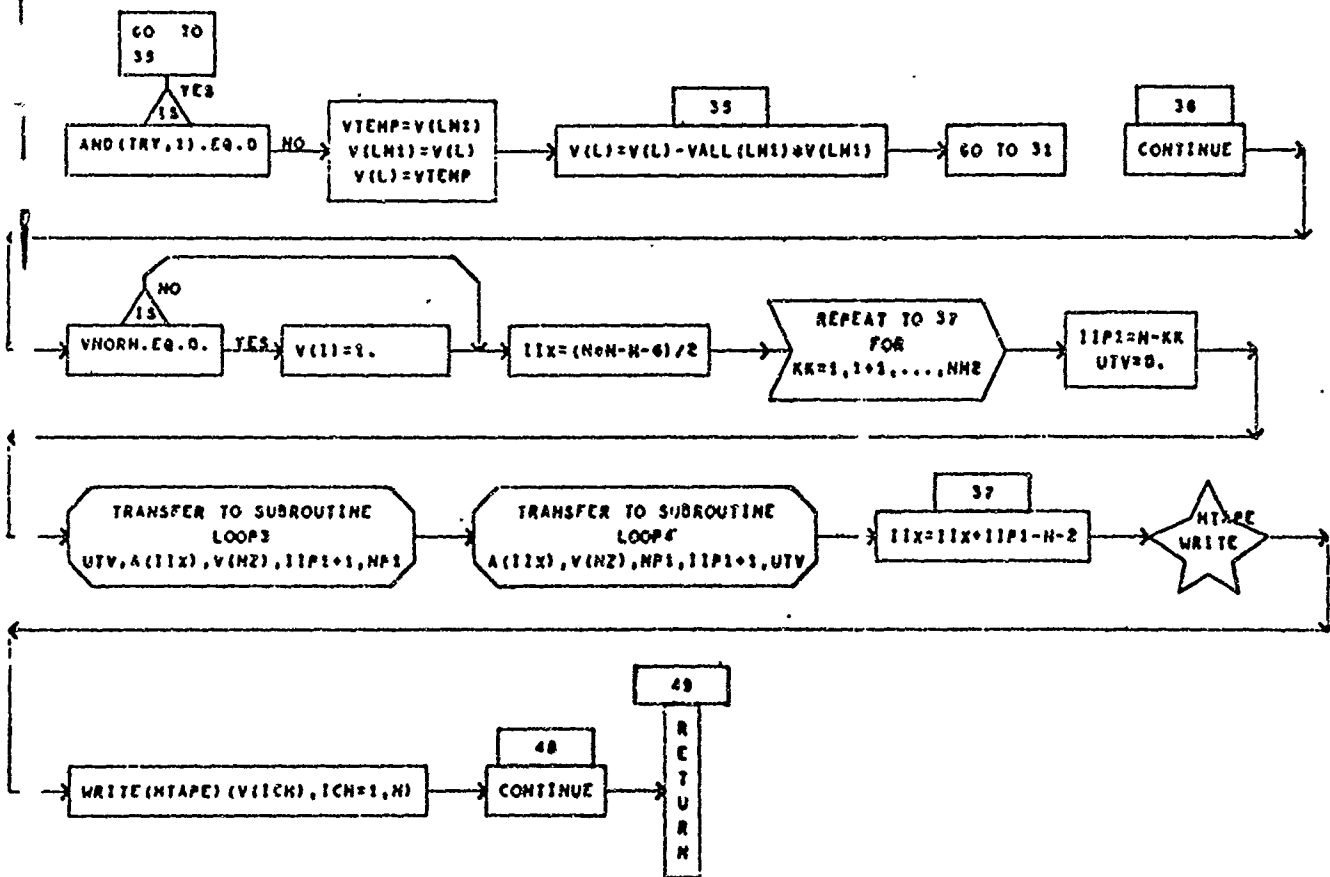
SYMBOL	STORAGES	SYMBOL	STORAGES	SYMBOL	STORAGES	SYMBOL	STORAGES	SYMBOL	STORAGES
A	1	VALU	1	VALL	1	UPPERD	1	DIAG	1
V	1	T	NN,3	INTER	1				











SYMINV

A IS THE UPPER TRIANGLE OF THE SYMMETRIC MATRIX TO BE INVERTED.

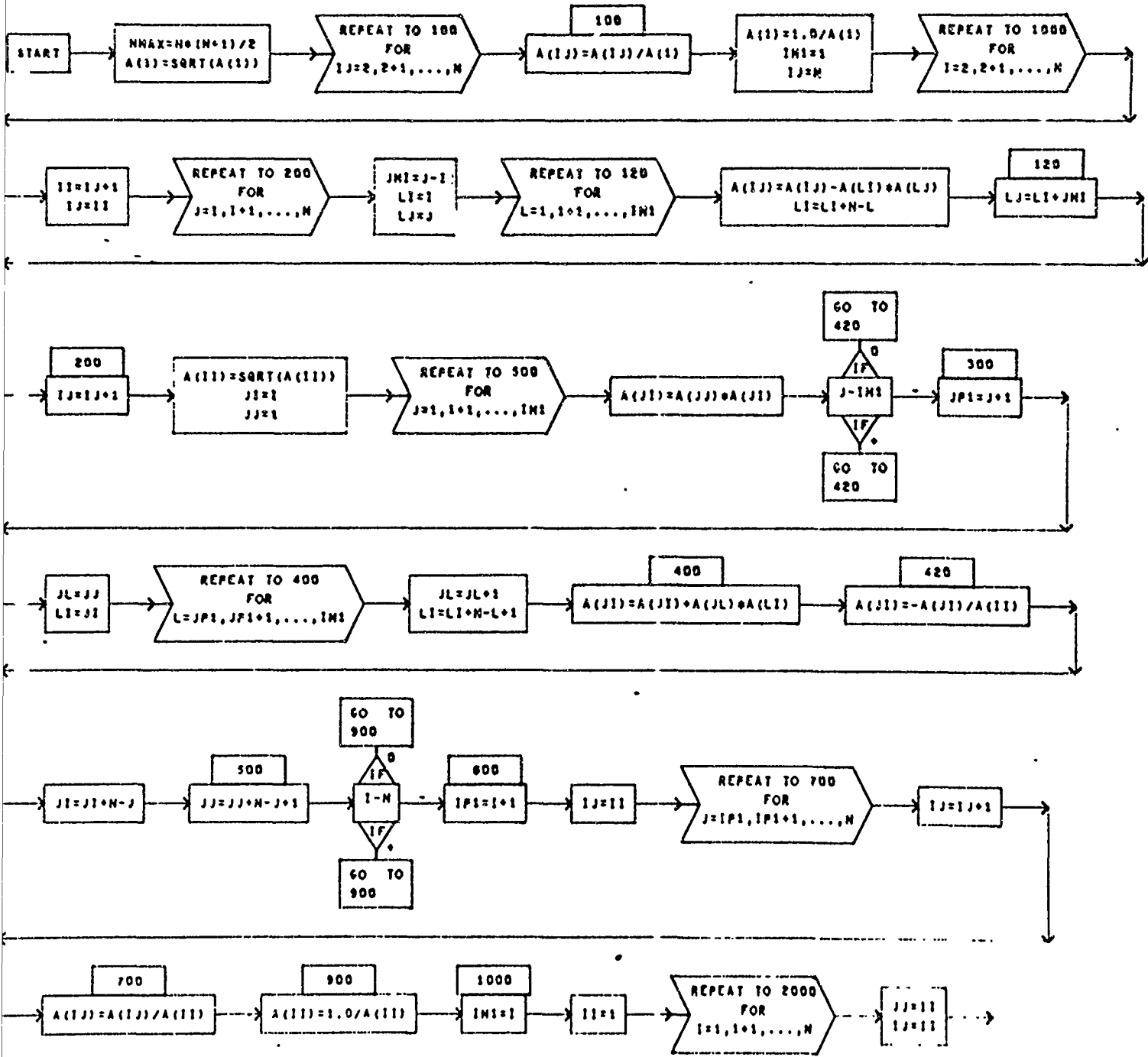
ELEMENTS ARE STORED ROWWISE.

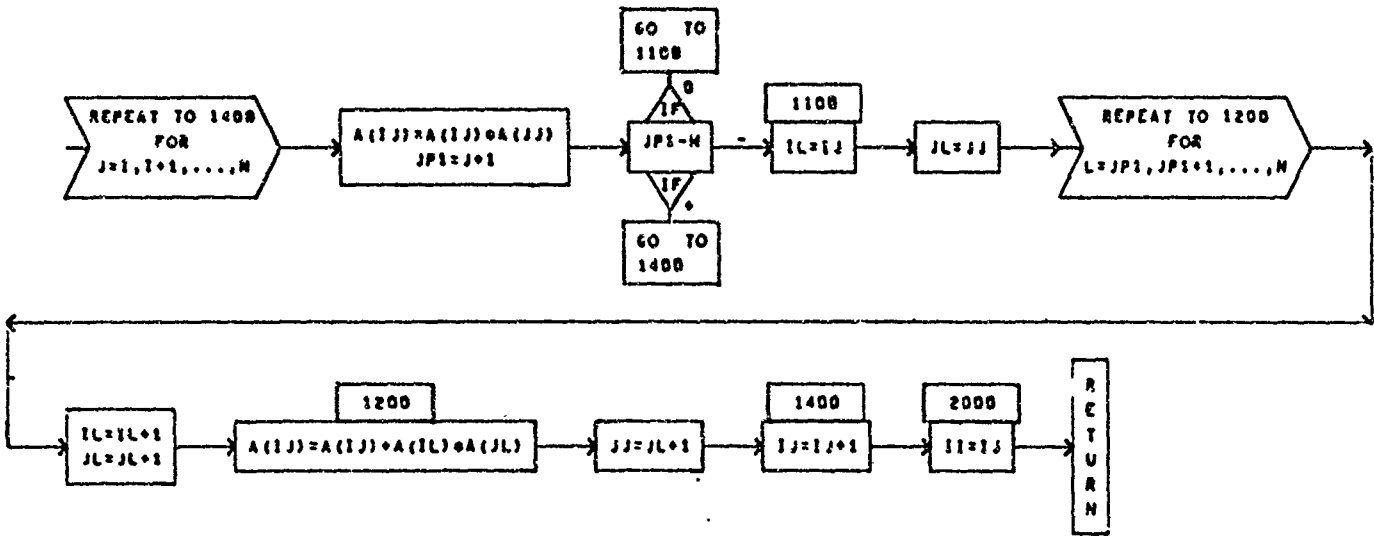
N = ORDER OF MATRIX

PROGRAM INVERTS IN PLACE.

D I M E N S I O N E D V A R I A B L E S

SYMBOL	STORAGES	SYMBOL	STORAGES	SYMBOL	STORAGES	SYMBOL	STORAGES	SYMBOL	STORAGES
A	1								





EIGMAT

THIS SUBROUTINE FINDS THE EIGENVALUES AND EIGENVECTORS FOR
 SYMMETRIC MASS AND STIFFNESS MATRICES.

THE ARGUMENTS ARE--

N- ORDER OF MATRICES.

A- DUMMY VECTOR WITH DIMENSION IN MAIN PROGRAM OF $N*(N+1)/2$
 VALU- STORAGE FOR EIGENVALUES MUST BE DIMENSIONED IN THE MAIN
 PROGRAM AS A VECTOR OF LENGTH NEIG.

TEMP,B,C,D,- DUMMY VECTORS WITH DIMENSION OF N IN MAIN PROGRAM.

E- DUMMY ARRAY WITH DIMENSIONS OF (N,3) IN MAIN PROGRAM.

IDUM- DUMMY INTEGER VECTOR WITH DIMENSION OF N IN MAIN PROGRAM.

MTAPE- TAPE WHERE STIFFNESS MATRIX IS STORED IN COMPACT FORM.

MTAPE- TAPE WHERE MASS MATRIX IS STORED IN COMPACT FORM.

ITAPE,ITAPE- SCRATCH TAPES.

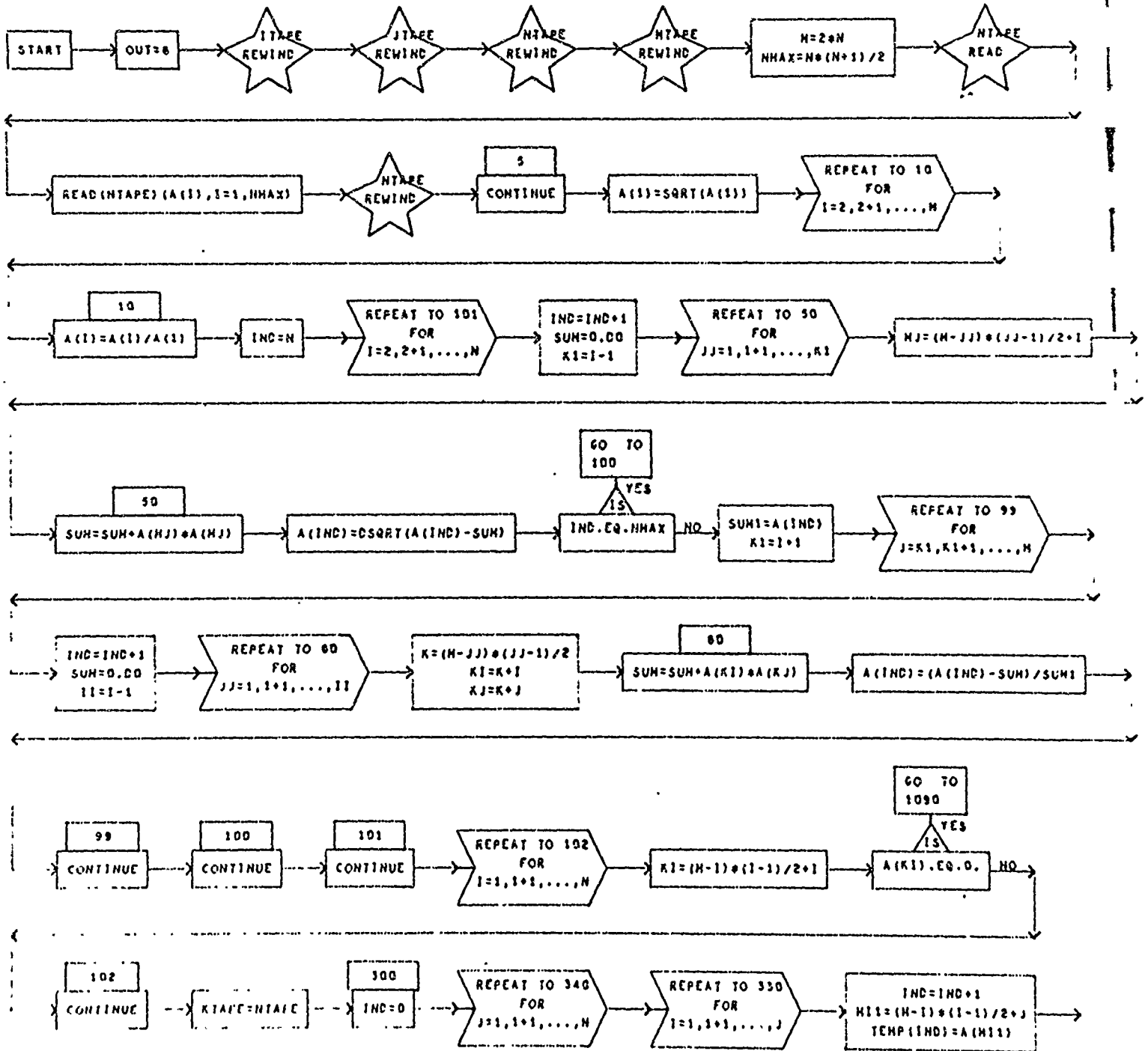
NEIG- NUMBER OF EIGENVALUES DESIRED.

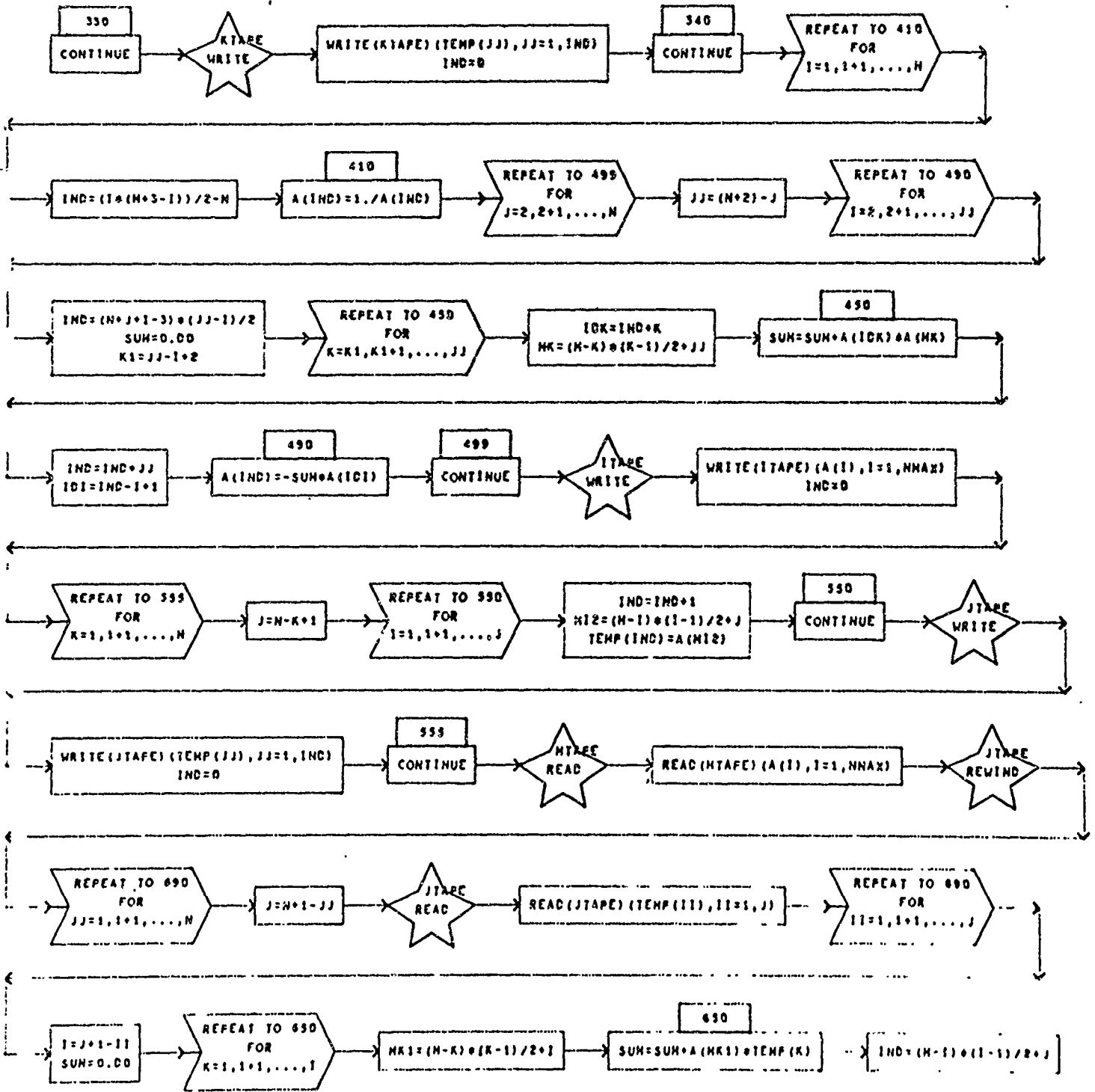
NVEC- NUMBER OF EIGENVECTORS DESIRED. MUST BE EQUAL TO OR LESS
 THAN NEIG.

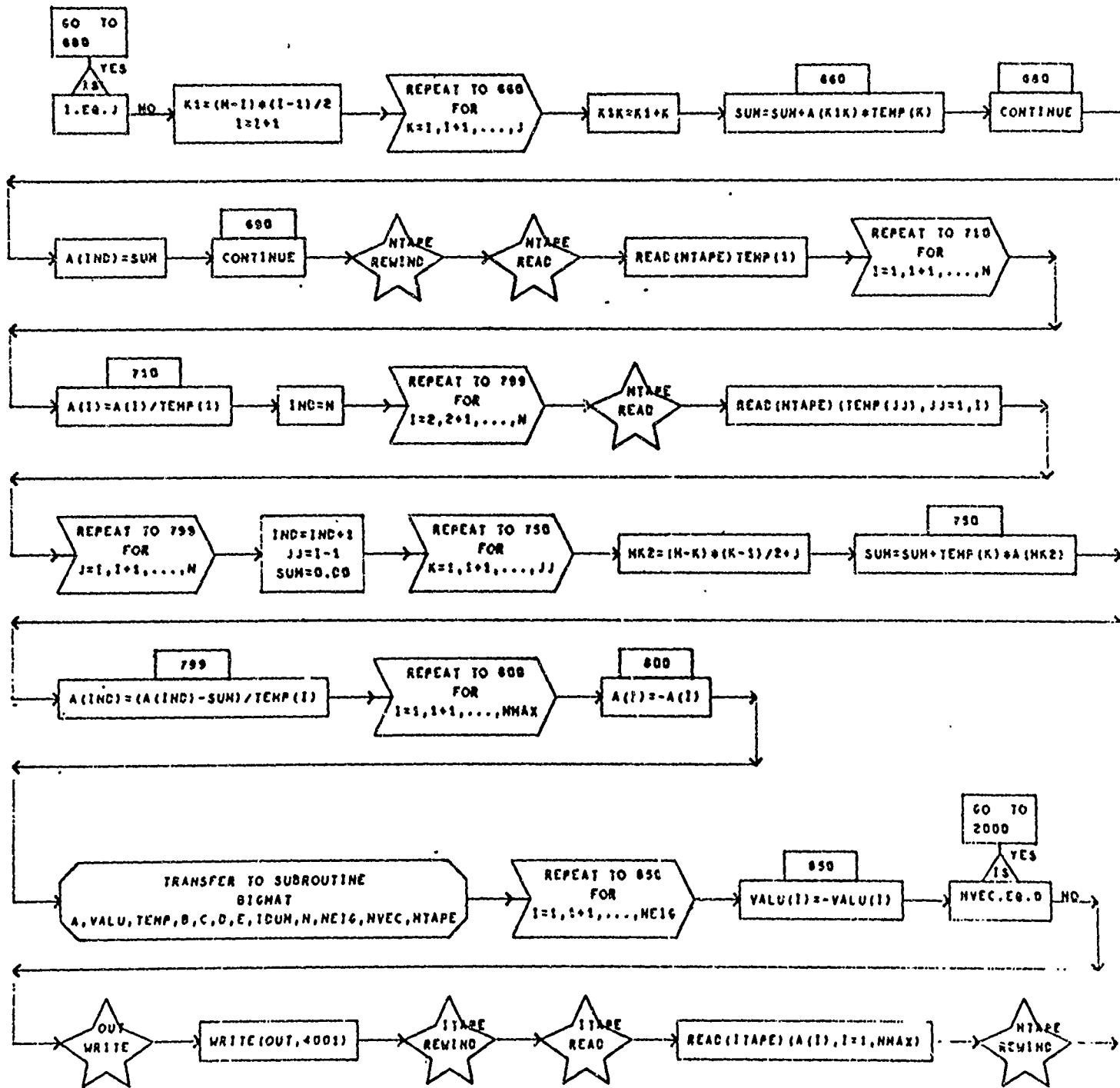
THE MASS AND STIFFNESS MATRICES ARE STORED IN COMPACT FORM AS
 VECTORS. ONLY THE UPPER TRIANGLE OF THESE MATRICES (BY ROWS) IS
 STORED.

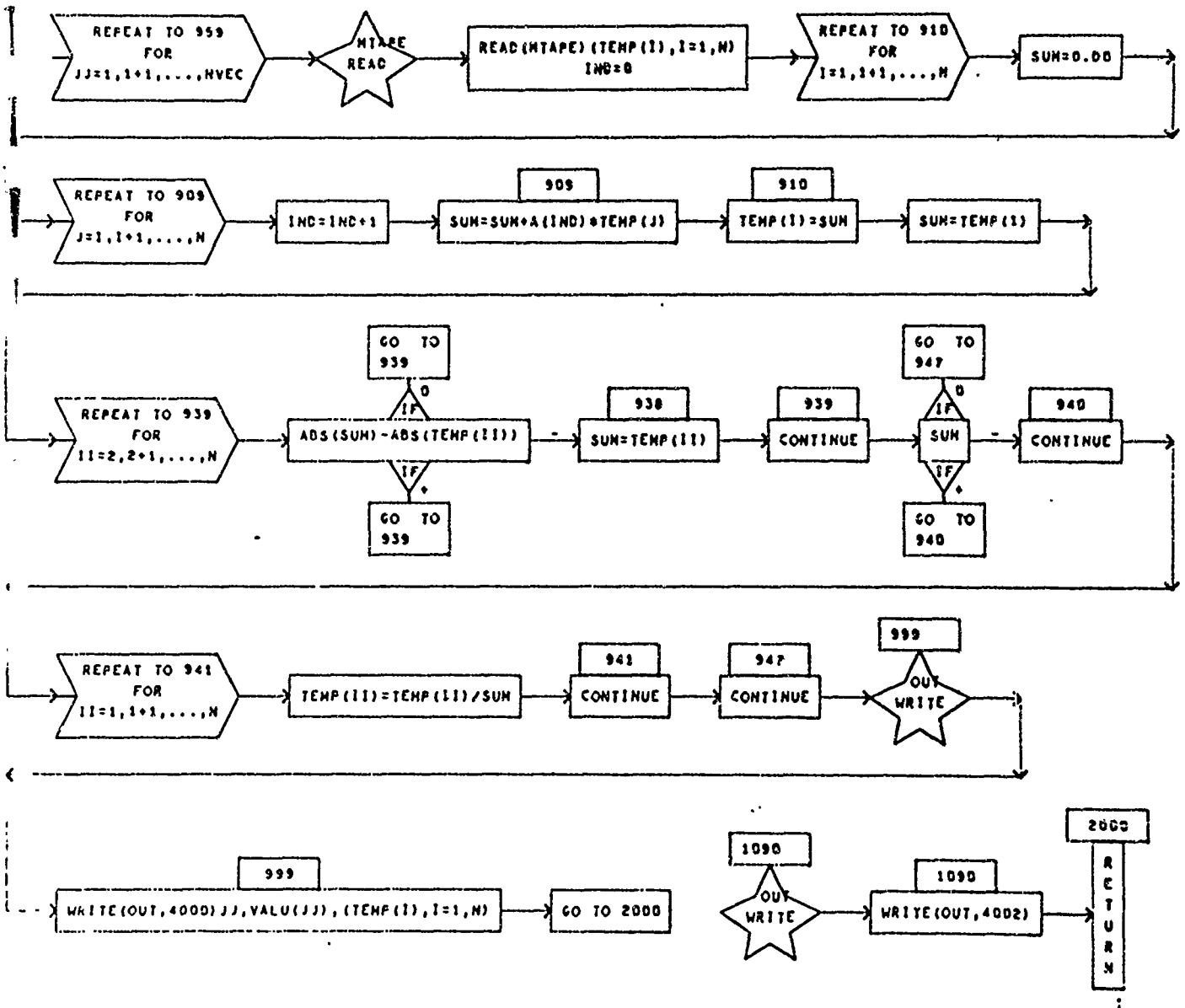
D I M E N S I O N E D V A R I A B L E S

SYMBOL	STORAGES	SYMBOL	STORAGES	SYMBOL	STORAGES	SYMBOL	STORAGES	SYMBOL	STORAGES
A	1	TEMP	1	VALU	1	B	1	C	1
D	1	E	N,3	IDUM	1				









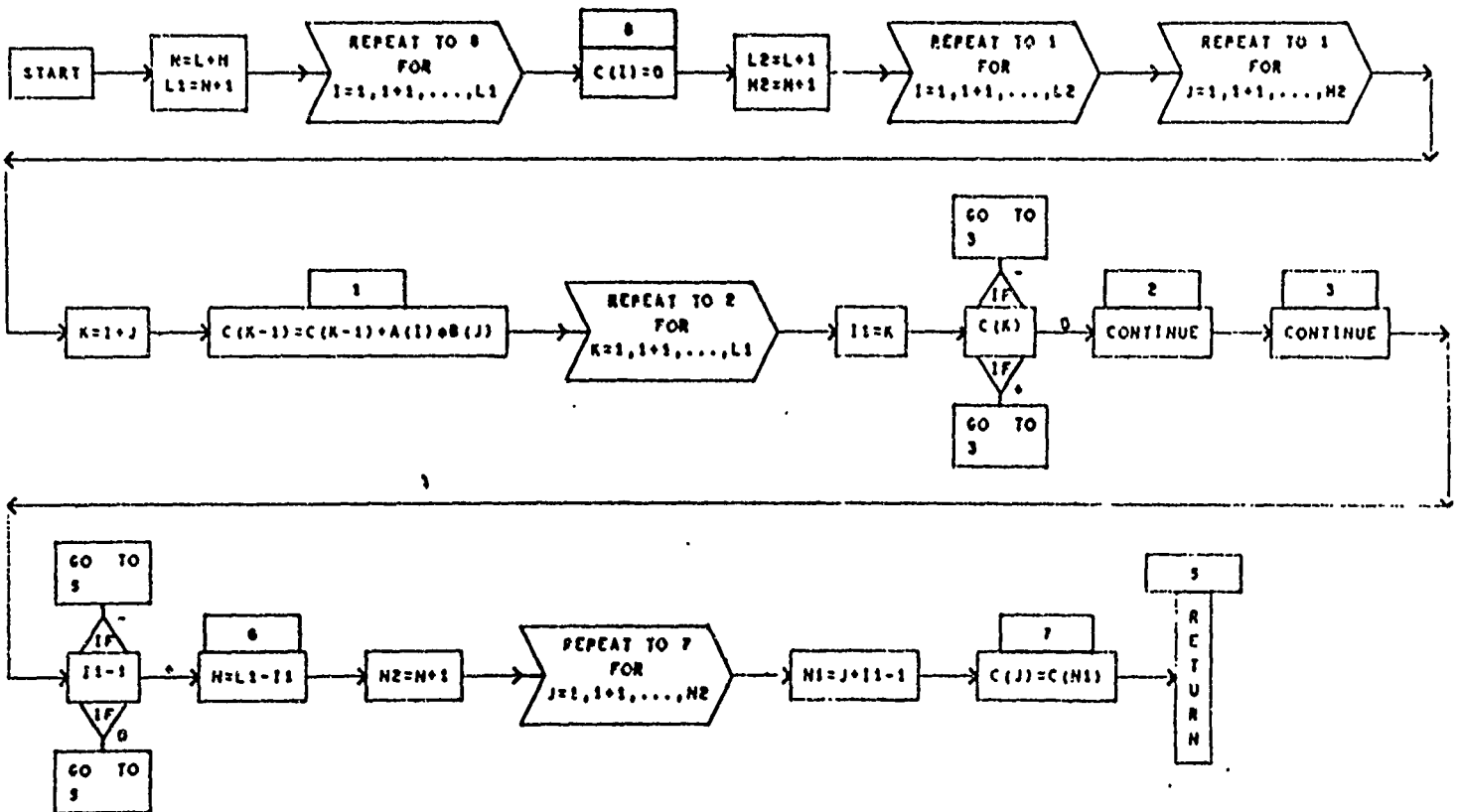
POLYNOMIAL MULTIPLY

D I M E N S I O N E D V A R I A B L E S

SYMBOL	STORAGES	SYMBOL	STORAGES	SYMBOL	STORAGES	SYMBOL	STORAGES	SYMBOL	STORAGES
A	1	B	1	C	1				

SUBROUTINE PLYMP(A,L,B,M,C,N)

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APPENDIX D

Symbol List

Symbol List

Listed below by their FORTRAN names are some of the input quantities to the program and their equivalent names in Section 3.0.

<u>Input Quantity</u>	<u>Symbol in Section 3.0</u>
YM	E
PR	ν
GE	G
DENS	ρ
X	X
Y	Y
RSMASS	M_i
AR	A
XI	I
YJ	J
PTH	t
DX	D_x
DY	D_y
D1	D_1
DXY	D_{xy}
BETA	β

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5. AUTHOR(S) (First name, middle initial, last name)

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13. ABSTRACT

THIS STUDY COVERS THE DEVELOPMENT OF A SET OF COMPUTER PROGRAM TO PERFORM FLUTTER ANALYSIS BY THE COLLOCATION METHOD. WHILE THIS METHOD HAS BEEN KNOWN FOR SOME TIME, ONLY RECENTLY HAVE ADVANCES IN COMPUTER TECHNOLOGY MADE THE METHOD TECHNICALLY AND FINANCIALLY FEASIBLE. THE INGREDIENTS OF A COLLOCATION FLUTTER ANALYSIS ARE 1) A FLEXIBILITY MATRIX, 2) AERODYNAMIC INFLUENCE COEFFICIENT MATRIX, AND 3) AN EIGENVALUE SOLUTION. THIS STUDY IS PRESENTED IN FOUR VOLUMES. VOLUME I CONTAINS A GENERAL PROGRAM DISCUSSION. VOLUME II CONTAINS THE PROGRAM FLUENC WHICH CALCULATES THE FLEXIBILITY MATRIX. VOLUME III CONTAINS A SET OF THREE PROGRAMS TO CALCULATE AERODYNAMIC INFLUENCE COEFFICIENTS FOR SUBSONIC, TRANSONIC, AND SUPERSONIC FLIGHT REGIMES. VOLUME IV CONTAINS THE PROGRAM COMA WHICH SETS UP AND SOLVES THE FLUTTER EIGENVALUE MATRIX.

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